



# Proposed Western Wheelers Motorsport Park | West Wyalong Airport, Bland Shire

Parking & Traffic Impact Assessment Report

P2101

Prepared for  
**Bland Shire Council**

21 April 2025

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# 1 Introduction

## 1.1 Background

Bland Shire Council has engaged Greys Consulting to prepare a Traffic Impact Assessment (TIA) to support a development application for the proposed Western Wheelers Motorsport Park, located adjacent to the West Wyalong Airport in Wyalong, NSW.

The development site is located directly north of the existing airport facility. Access will be via Airport Road, which intersects with the Newell Highway, a classified state road that serves as a vital freight and transport route through the region. The area is strategically positioned to take advantage of the accessibility provided by the surrounding regional road network while supporting tourism, motorsport events, and club activities.

The proposal comprises constructing a new car racing track facility and associated infrastructure over an area of approximately 90,618 m<sup>2</sup>. The development will include a sealed racing surface, return road, supporting facilities for club operations, vehicle access, and parking.

The proposed development is located entirely within Lot A in DP 220617 and is aligned with strategic regional objectives to stimulate economic activity through tourism, motorsports, and regional events.

This TIA assesses existing and future traffic conditions, parking demand, access arrangements, and safety considerations to support informed decision-making and ensure that the development integrates safely and efficiently with the surrounding road network.

The boundaries of the proposed development are highlighted in Figure 1-1.



Source: Sixmaps

**Figure 1-1 Subject Site Location**



## 1.2 Report Objectives

This TIA aims to evaluate the traffic and transport implications of the proposed Western Wheelers Motorsport Park development adjacent to West Wyalong Airport. The assessment has been undertaken to accompany a Development Application (DA) submitted to Bland Shire Council and addresses the following key aims:

- To assess existing traffic conditions on the surrounding road network, particularly along the Newell Highway and Airport Road, based on available traffic data and observed access characteristics.
- To project future traffic generation associated with the proposed motorsport facility, considering expected visitor numbers, event operations, and access modes.
- To assess the adequacy of existing access arrangements and recommend improvements where necessary to accommodate additional traffic volumes safely and efficiently.
- To identify appropriate intersection treatments at the Airport Road / Newell Highway junction to support anticipated traffic movements associated with event and non-event days.
- To evaluate the proposed access driveway location, internal circulation layout, and parking provisions for compliance with relevant Australian Standards (AS2890 series), Austroads guidelines, and Council planning controls.
- To assess active and public transport accessibility and identify whether additional infrastructure or wayfinding improvements are warranted.
- To ensure that access for service and emergency vehicles, including waste collection and delivery movements, is feasible and safe.
- To undertake high-level SIDRA intersection modelling where appropriate to quantify traffic performance under base case, project case, and mitigation scenarios.
- To provide a clear summary of findings and offer evidence-based recommendations to inform Council's decision-making on the proposed development.

## 1.3 Reference Documents

The following documents and guidelines have been reviewed and referenced in the preparation of this report:

- Bland Shire Council Development Control Plan (DCP) 2012
- Bland Local Environmental Plan (LEP) 2011
- Guide to Traffic Impact Assessment (TfNSW, 2024)
- Austroads Guide to Road Design – Part 4A: Unsignalised and Signalised Intersections
- AS 2890.1:2004 – Off-street Car Parking
- AS 2890.2:2018 – Off-street Commercial Vehicle Facilities
- AS 2890.6:2022 – Off-street Parking for People with Disabilities

## 2 Existing Conditions

### 2.1 Existing Development

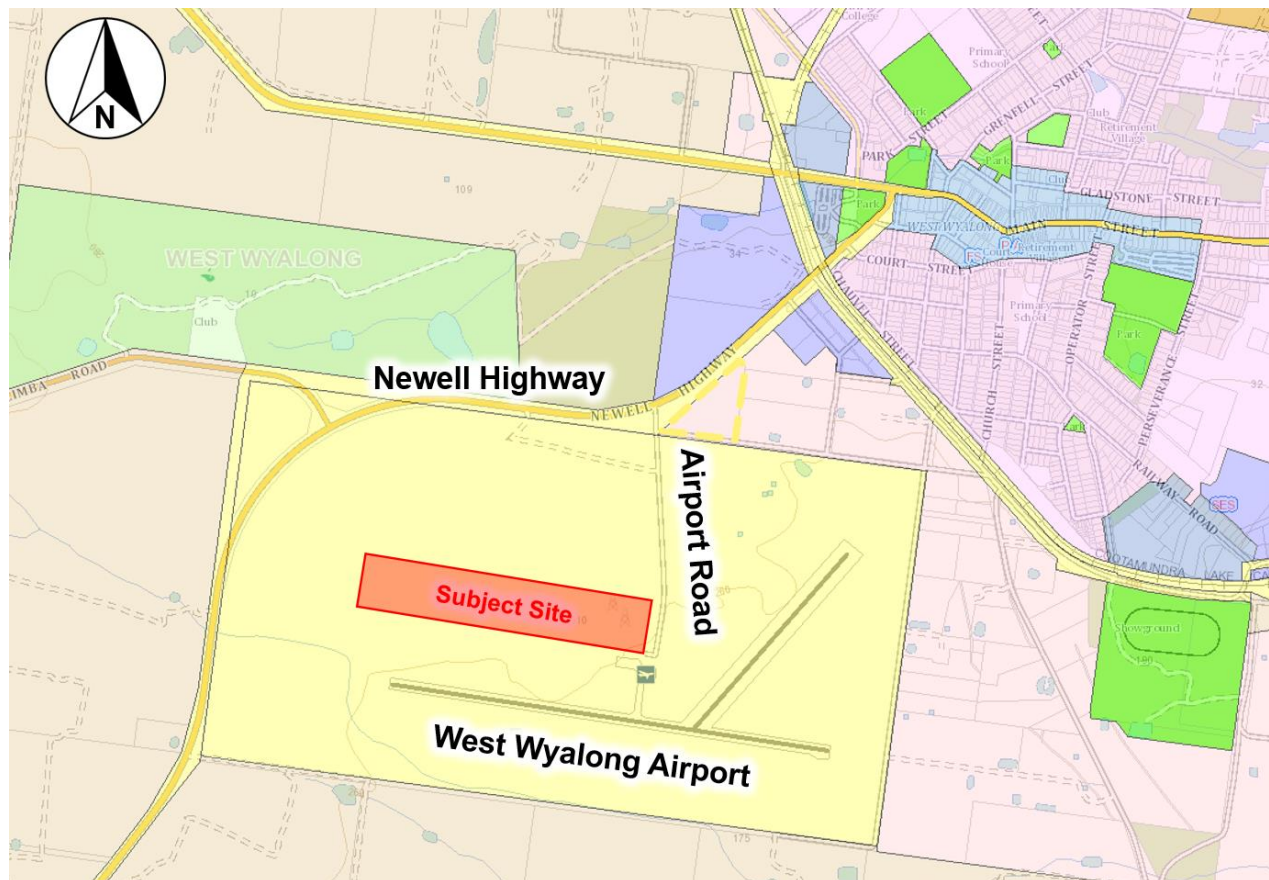
The subject site is immediately east of the West Wyalong Airport and comprises vacant land with no existing buildings or formalised infrastructure. The area is generally flat, cleared, and free from significant vegetation, making it well-suited for development without requiring substantial earthworks. The site directly adjoins Airport Road and is approximately 150 metres south of the Newell Highway, providing efficient regional and state road network access.

The site is zoned SP1 – Special Activities (Airport) under the Bland Local Environmental Plan 2011, which permits uses associated with airport operations and ancillary development, subject to development consent. The proposed motorsport use is consistent with the intent of this zoning, which supports aviation-adjacent land uses that do not conflict with airport safety or operations. Surrounding land uses include:

- SP1 (Special Activities – Airport) to the south (active airport runway and terminal area),
- RU1 (Primary Production) or RU4 (Rural Small Holdings) to the west and north,
- RE1 (Public Recreation) or IN1 (General Industrial) further east along Airport Road.

There are no sensitive residential areas in the immediate vicinity of the proposed development. The nearest residential zones (R5 – Large Lot Residential) are located more than 1 km from the site boundary, which limits the potential for amenity impacts due to traffic or noise.

Vehicle access is available via Airport Road, which intersects with Newell Highway at a priority-controlled T-intersection. This connection forms the key transport link between the site and broader regional centres. The site and its surrounding landuse are depicted in Figure 2-1.



Source: Hawkesbury Council LEP 2012 Maps (last updated 12/02/2025)

**Figure 2-1 Study Area Land Use Plan**

## 2.2 Surrounding Road Network

### 2.2.1 Key Roads

The proposed development is located adjacent to Airport Road, which intersects with the Newell Highway. These roads form the primary access and distribution routes for traffic generated by the proposed motorsport facility. Key characteristics of the road network are summarised in Table 2.1.

**Table 2-1 Bus Services Operating Near the Site**

Road Name	Jurisdiction	Hierarchy	No. Lanes	Divided	Speed Limit	Comments
Newell Highway	TfNSW	Arterial State Road	2	No	110 km/h	<i>Key regional freight corridor</i>
Airport Road	Bland Shire Council	Local Road	2	No	50 km/h	<i>Access to airport and development site</i>

The main traffic corridors in the vicinity of the subject site is Newell Highway, which is classified as state-controlled roads. Transport for NSW has authority over these roads.

### 2.2.2 Newell Highway

Newell Highway (A39) is a principal north–south state highway and forms part of the national highway network. In the vicinity of the site, it operates as a two-lane undivided arterial road with a posted speed limit of 110 km/h adjacent to the site. Newell Highway is under the jurisdiction of Transport for NSW (TfNSW), serving as a key north–south freight and regional travel corridor. It provides regional connectivity across western New South Wales and is one of the state’s primary inland freight routes. It accommodates significant volumes of freight and long-distance traffic and plays a strategic role in regional connectivity between the Riverina, Central West, and Northern Inland regions of NSW.

### 2.2.3 Airport Road

Airport Road is a sealed local road providing direct vehicular access to the West Wyalong Airport and adjacent rural land uses. It connects to the Newell Highway at a T-intersection, which is currently unsignalised and priority-controlled. The road is lightly trafficked under current conditions but is expected to accommodate increased volumes during events at the proposed motorsport park.

Airport Road is a local access road managed by Bland Shire Council. It connects the airport precinct to the West Wyalong township and local industrial areas. It is a two-lane undivided road providing access to the airport terminal and aviation support facilities. The road operates under default urban speed limits and is not currently signalised, and CHR(S) and BAL treatments are at its intersection with Newell Highway.

## 2.3 Existing Traffic Controls

The following traffic controls and infrastructure are relevant to the operation and capacity of the surrounding road network:

- A posted speed limit of 110 km/h applies to Newell Highway at the intersection with Airport Road.
- A sealed, priority-controlled T-intersection is present at the Newell Highway / Airport Road intersection.
- The intersection includes an existing right-turn lane treatment (CHR(S)) and a Basic Left Turn treatment (BAL) on Newell Highway for northbound traffic turning right into Airport Road, providing separation from through vehicles.
- No formal left-turn deceleration lane exists for vehicles turning left into Airport Road from the north.
- These controls inform the assessment of site access safety, intersection performance, and any necessary upgrades to accommodate the proposed development.



## 3 Proposed Development

### 3.1 Development Description

The proposal involves the establishment of a motorsport facility adjacent to the existing West Wyalong Airport, within land under the management of Bland Shire Council. The Western Wheelers Car Club proposes to develop a sealed racing track, associated support infrastructure, and access roads over a site area of approximately 90,618 square metres. The facility is intended to support club-level car racing, driver education programs, community motorsport events, and related activities.

The site is situated directly east of the airport runway and south of the Newell Highway, with vehicular access provided via Airport Road. The existing topography is relatively flat and cleared of significant vegetation, making the site well-suited for development with minimal earthworks. No existing structures are located within the proposed development area.

The key components of the proposed development include:

- A sealed racing track with a return lane
- Dedicated entry and exit driveways connected to Airport Road
- Informal parking and staging areas for vehicles and spectators
- Event control and support areas, to be delivered in future stages

The facility is expected to operate on a flexible schedule with activity concentrated on weekends and during organised events. It is not intended to operate as a daily commercial attraction and will primarily be used by club members and authorised participants. Anticipated traffic volumes will vary depending on the nature of the event, with peak volumes expected during scheduled racing events and community open days.

### 3.2 Access Arrangements

Vehicle access to the site will be facilitated via a new connection to Airport Road. The proposed access location is situated approximately 150 metres south of the intersection with Newell Highway, providing direct connectivity to the regional road network. The access is proposed as a two-way sealed driveway, designed to accommodate light vehicles, trailers, and service vehicles.

The Newell Highway / Airport Road intersection is currently configured as a priority-controlled T-intersection. The major road provides for two-way through traffic movements and includes an existing right-turn treatment for northbound vehicles turning into Airport Road. A basic left-turn treatment exists on the southbound approach. Given the nature and scale of the proposed development, the capacity and functionality of this intersection are reviewed in Section 5 of this report.

No internal roads are formally proposed at this stage; however, the internal layout includes provision for vehicle manoeuvring, informal parking areas, and access to track entry points. The access geometry, internal swept paths, and turning facilities for service vehicles have been reviewed against AS2890.2 and AS2890.1 and are discussed further in Section 6.

### 3.3 Parking Provision

Parking will be provided within the site in an informal format to accommodate spectators, participants, staff, and service vehicles. Given the unique land use and absence of specific parking rates within the TfNSW Guide to Traffic Impact Assessment or the Bland Shire DCP, parking demand has been assessed using first-principles methodology. Parking areas will be delineated using surface markings or temporary fencing during events, with sufficient area allocated for overflow or future formalisation.

The final parking demand and provision strategy is addressed in Section 7 of this report.

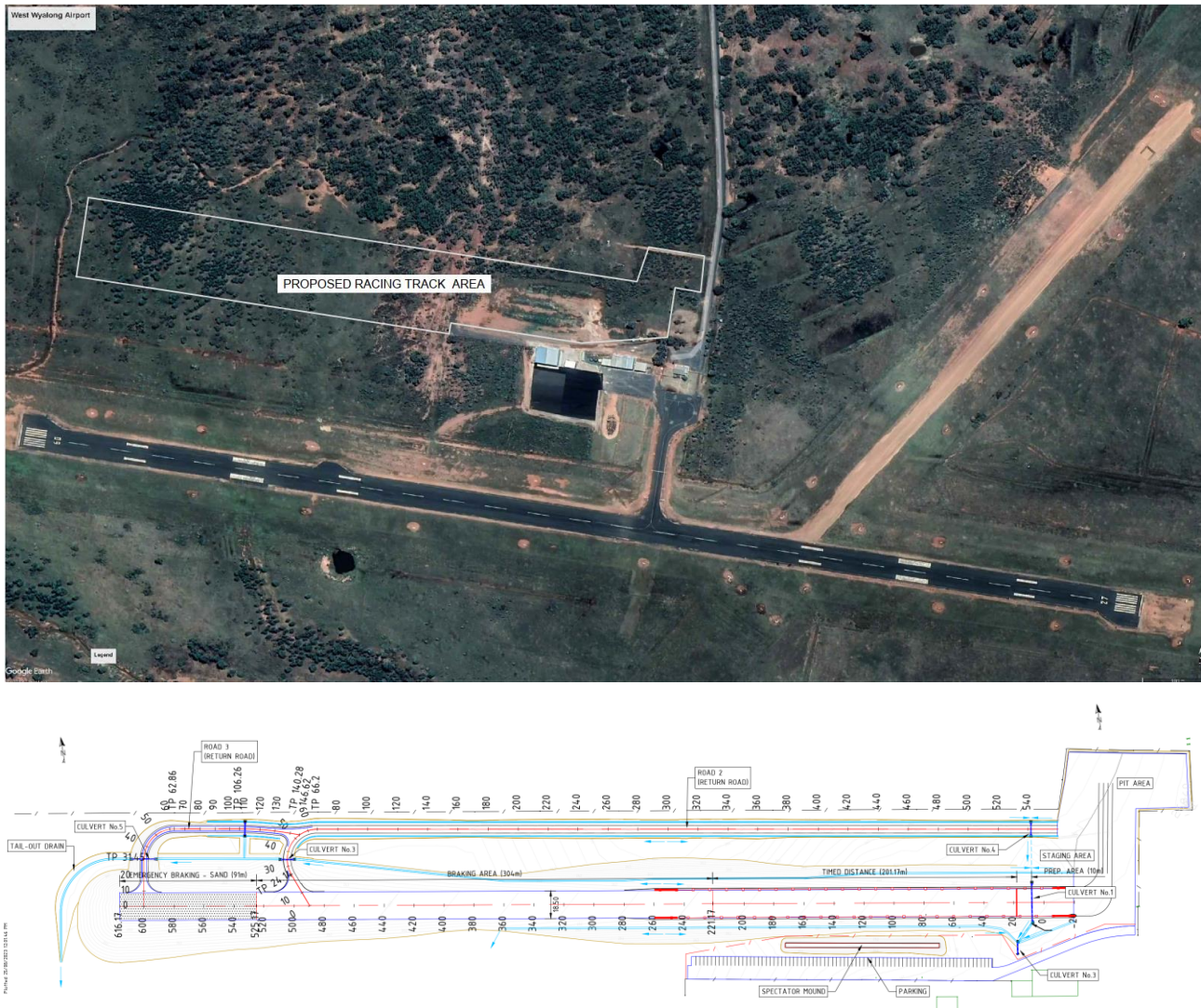
### 3.4 Operational Characteristics

The facility is intended for use during daylight hours, with no lighting infrastructure proposed. Noise management and event scheduling will be addressed through a separate environmental assessment.

Vehicle access will be managed via existing airport gates, with the potential for dedicated entry control during events. On-site traffic movement will be managed to avoid conflicts between users, and sufficient sight distance is available at the site access point to accommodate safe entry and exit under rural speed conditions.

Event management and traffic control measures will be implemented where required during peak usage periods, and these measures are further considered in Section 9.

The concept layout and locality of the proposed motorsport facility is illustrated in Figure 3-1.



Source: PLAN LAYOUT – WESTERN WHEELERS MOTORSPORT PARK

**Figure 3-1 Study Area Land Use Plan**

## 4 Traffic Generation and Distribution

### 4.1 Traffic Generation Methodology

There are no prescribed traffic generation rates for motorsport parks or club-based racing facilities within the Transport for NSW (TfNSW) Guide to Traffic Impact Assessment (2024), the RMS Guide to Traffic Generating Developments (2002), or the Bland Shire Council DCP. In the absence of empirical trip generation benchmarks for this specific land use, a first-principles approach has been adopted. This method draws upon land use characteristics, expected operational conditions, and comparisons with similar facilities located in regional New South Wales and Victoria.

The Western Wheelers Motorsport Park is proposed to function primarily as a club-operated venue, hosting low-frequency events such as race meetings, driver education days, and community motorsport exhibitions. The proponent has not provided a detailed operational statement or event schedule at this stage; therefore, estimates of peak demand have been formulated using analogous land uses and site observations at comparable motorsport facilities.

Based on review of publicly available material, transport assessments, and operator information for similar-scale motorsport sites in regional settings, the following typical event attendance characteristics have been noted:

- The *Gunnedah Speedway* (NSW), a regional circuit with similar access constraints, accommodates approximately 60–80 vehicles during regular race events, based on typical pit usage, participant registration counts, and spectator vehicle volumes.
- The *Wodonga Motorsport Complex* (VIC), which hosts club-level time trials and training days, regularly generates around 90 vehicles during peak weekend events, including participants, trailers, support crew, and local visitors.
- The *Wagga Motorsports 1/8 Mile Strip*, used for grassroots racing, has been observed to attract up to 100 vehicles per event, especially during advertised seasonal meets, based on data published in local event logistics and available photographs of queue lengths and pit occupancy.
- Smaller facilities such as *Queanbeyan's Fairbairn Park* and *Ararat Hillclimb* generate in the order of 40–70 vehicles depending on event type and track capacity.

These figures have been used as a reference to establish the peak trip generation range for the Western Wheelers Motorsport Park. The 60–100 vehicle range adopted in this assessment is considered reasonable and conservative, reflecting the maximum number of vehicles expected during a typical peak weekend event at a regional motorsport facility of this scale and nature. This range includes:

- Participant vehicles (driver and crew transport vehicles)
- Trailers towing competition vehicles
- Spectator vehicles (including families or small groups)
- Event marshals and limited support staff

No formal public grandstand facilities or high-capacity spectator infrastructure are proposed, which places the development well below the scale of commercial racing venues or motorsport precincts with significant public draw.

### 4.2 Assessment Periods

Based on the above operational profile, the following peak periods have been adopted for the purpose of this traffic impact assessment:

- AM Peak Hour: 8:00 am to 9:00 am (event arrival period),
- PM Peak Hour: 4:00 pm to 5:00 pm (event departure period).

These periods align with typical event scheduling and ensure the assessment captures the highest traffic demand associated with the proposed development. They are not expected to coincide with weekday commuter peak periods on the surrounding road network.

### 4.3 Event Arrival and Departure Profile

Without fixed operating hours, it is assumed that events will generally be scheduled between 9:00 am and 4:00 pm on weekends. Based on typical participant arrival behaviour and observed access profiles at similar venues, it is estimated that approximately 60 per cent of all vehicles would arrive during the peak one-hour period prior to event commencement, and a similar proportion would depart during the peak one-hour period following event completion.

Accordingly, the following peak hourly traffic generation rates have been adopted for modelling purposes:

- 100 vehicles (maximum total estimate) x 0.6 = 60 vehicle trips in the AM peak hour
- 100 vehicles x 0.6 = 60 vehicle trips in the PM peak hour

These values represent a conservative worst-case scenario, assuming minimal car-pooling and no staged entry or egress strategies. They have been used as input for SIDRA intersection modelling and network performance assessment, which are presented in Section 5.

### 4.4 Non-Event Conditions

Outside of scheduled events, the facility is expected to generate minimal daily vehicle movements, which are limited to maintenance vehicles, individual practice runs, or administrative access. It is estimated that non-event periods would generate fewer than 10 vehicle trips per day, well below thresholds requiring impact assessment under TfNSW or Austroads guidelines.

The adopted approach ensures that the impact analysis covers both the upper-bound demand scenario and recognises the low-intensity baseline use of the facility on non-event days.

### 4.5 Trip Distribution and Assignment

Trip distribution for the development has been determined based on the regional road hierarchy and the origin–destination patterns of participants travelling from surrounding townships and regional centres. Given the location of the site immediately south of the Newell Highway, it is expected that the majority of development traffic will approach from and return to the highway corridor.

Based on regional access assumptions, the directional distribution of traffic has been estimated as follows:

- 60 per cent of vehicles will travel to/from the north via Newell Highway, including West Wyalong, Forbes, Parkes, and other central NSW towns
- 40 per cent of vehicles will travel to/from the south via Newell Highway, including Narrandera, Wagga Wagga, and southern regions

All development-related trips will enter and exit via Airport Road. Vehicles accessing Newell Highway will do so via the existing priority-controlled T-intersection, including a short auxiliary right-turn lane for northbound traffic entering Airport Road. No left-turn auxiliary treatment exists for southbound traffic.

These assumptions have been used to assign development traffic in the SIDRA modelling scenarios, with distribution applied proportionally to each approach leg of the Newell Highway / Airport Road intersection.



## 5 SIDRA modelling and Intersection Performance

### 5.1 Assessment Approach

The operational performance of the Newell Highway / Airport Road intersection has been assessed under three network scenarios using SIDRA Intersection 10.0:

- **Base Case:** existing background traffic conditions with no development traffic
- **Project Case:** Base Case with traffic generated by the proposed motorsport development
- **Project Case with Mitigation:** Project Case with consideration of auxiliary turn treatments on Newell Highway

The intersection of Newell Highway and Airport Road forms the only vehicular access to the site. The analysis focused on the AM arrival and PM departure peak hours, assessing Degree of Saturation (DoS), average delay, Level of Service (LoS), and 95th percentile queue lengths for all movements.

All models included 30% heavy vehicle proportions for development-generated movements to account for trailers and support vehicles associated with the motorsport park.

### 5.2 Network Geometry and Modelling Inputs

The Newell Highway is configured as a two-lane, two-way rural arterial road with a posted speed limit of 110 km/h. Airport Road intersects from the east at a T-junction, also configured as a two-lane road with a posted speed limit of 60 km/h. The intersection geometry was coded to reflect the following characteristics:

- Two-lane two-way operation on Newell Highway (110 km/h)
- A short auxiliary right-turn lane (CHR(S)) for northbound vehicles on Newell Highway entering Airport Road
- No auxiliary left-turn treatment for southbound vehicles on Newell Highway
- A single approach lane for all movements on Airport Road

In the absence of site-specific survey data, existing traffic volumes were sourced from the Transport for NSW Traffic Volume Viewer (Site 6142 – 460 m east of Nicholson Lane, Wyalong) 2024 data, as the most proximate and reliable available data. The AM and PM peak hour volumes were used in the SIDRA model. The heavy vehicle percentages were extracted from the same dataset. The following Weekends (Saturday/Sunday) through traffic volumes were adopted for the Newell Highway:

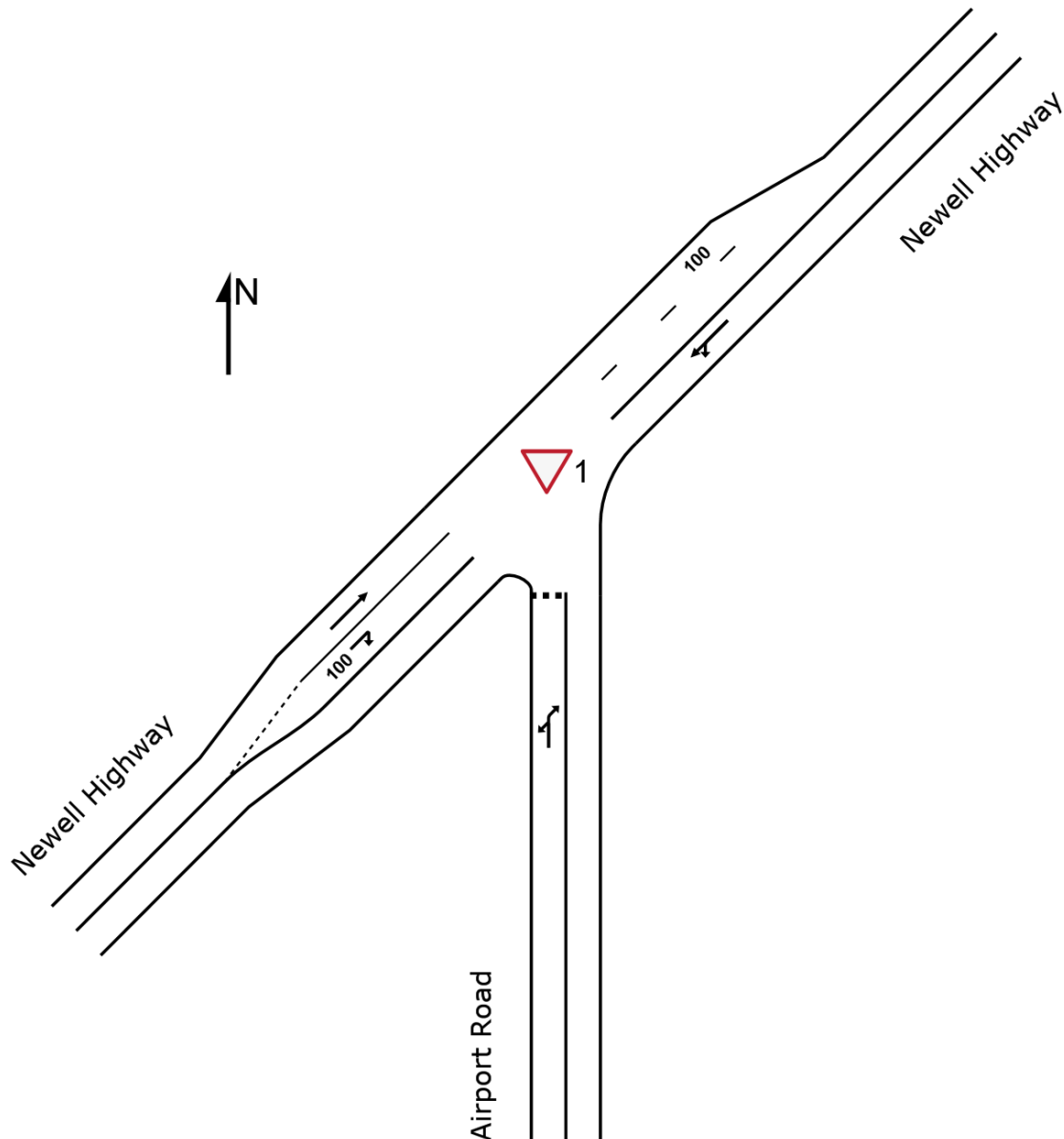
**Table 5-1 Base Case Peak Hour Input Volumes and Vehicle Class Breakdown**

Movement	Total (veh/h)	Light Vehicles	Heavy Vehicles	HV %
Newell Hwy NB Through	48	33	15	31%
Newell Hwy NB Right Turn to Airport Rd	10	7	3	30%
Newell Hwy SB Through	49	34	15	31%
Newell Hwy SB Left Turn to Airport Rd	18	13	5	28%
Airport Rd Left Turn to Newell Hwy NB	15	12	3	20%
Airport Rd Right Turn to Newell Hwy SB	20	15	5	25%



The SIDRA model was configured using these inputs, applying separate vehicle classes per movement and assigning appropriate turning treatments. This configuration accurately reflects the operational context of the Newell Highway / Airport Road intersection during weekday AM and PM peak periods.

These volumes are considered appropriate for assessing an unsignalised rural intersection with limited turning traffic and low background growth. The intersection layout coded in Sidra is depicted in Figure 5-1.



**Figure 5-1 Newell Highway/Airport Road Intersection Layout**

### 5.3 Intersection Geometry and Model Configuration

The Newell Highway is a two-lane rural arterial with a posted speed limit of 110 km/h. Airport Road is a two-lane local access road with a 60 km/h posted speed. The existing intersection is a T-junction with priority control and includes a short auxiliary right-turn lane (CHR(S)) for northbound traffic turning right into Airport Road. No auxiliary left-turn lane is provided for southbound traffic turning left into Airport Road.

The SIDRA model was configured as follows:

- Major road: Newell Highway, with two uninterrupted through lanes
- Minor road: Airport Road, with a single approach lane for both turning movements
- Right-turn treatment: CHR(S) on the northbound approach
- Speed environment: 110 km/h for Newell Highway, 60 km/h for Airport Road

The intersection of Newell Highway/Airport Road layout is depicted in Figure 5-2.



Source: SIXMAPS

**Figure 5-2 Newell Highway/Airport Road Intersection Layout**

## 5.4 Base Case Performance Outputs

SIDRA Intersection outputs for the base case indicate that the Newell Highway / Airport Road intersection performs well below capacity for both AM and PM peak periods. Detailed performance results for each approach and movement are summarised below.

### 5.4.1 AM Peak (Base Case)

- Highest Degree of Saturation (DoS): 0.042 (Newell Hwy NB through and left-turn)
- Average delay: 5.7 seconds for minor road movements
- Level of Service (LoS): A across all approaches
- 95th percentile queue length:
  - Airport Rd Right Turn: 1.1 m
  - Airport Rd Left Turn: 1.1 m

### 5.4.2 PM Peak (Base Case)

- Highest Degree of Saturation (DoS): 0.055 (Newell Hwy NB through and left-turn)
- Average delay: 6.0 seconds for minor road movements
- Level of Service (LoS): A across all approaches
- 95th percentile queue length:
  - Airport Rd Right Turn: 1.2 m
  - Airport Rd Left Turn: 1.1 m

All performance indicators confirm that the intersection operates with significant spare capacity and minimal queuing under existing traffic conditions.

## 5.5 Project Case Performance and Comparison with the Base Case

SIDRA modelling results for the Project Case, which includes development-generated traffic during the AM and PM event peaks (60 vehicles per hour), indicate a marginal increase in overall traffic demand at the intersection. This additional volume was distributed with 60 per cent assigned to/from the north and 40 per cent to/from the south, consistent with expected regional travel patterns.

### 5.5.1 AM Peak (Project Case)

In the AM peak, the dominant development traffic movements are right turns from Newell Highway northbound into Airport Road and left turns from Newell Highway southbound into Airport Road. These account for the arrival phase of the event.

- The Degree of Saturation (DoS) increased from 0.042 in the base case to 0.082.
- Average delays remained consistent with the base case, ranging from 6.5 to 7.0 seconds across critical movements.
- 95th percentile queues increased marginally to 1.4 m, remaining within available storage.
- Level of Service remained at LoS A for all movements.

### 5.5.2 PM Peak (Project Case)

In the PM peak, development traffic is concentrated in outbound movements, notably left turns from Airport Road onto Newell Highway northbound and right turns onto Newell Highway southbound.

- The highest DoS was observed on the Airport Road approach at 0.096.
- Average delays for the right and left turn movements on Airport Road were 6.4 to 7.1 seconds.
- 95th percentile queue length increased to 3.4 m for the right turn from Airport Road, which is still well within available lane capacity.
- All movements retained Level of Service A.

### 5.5.3 Comparison with Base Case

When compared to the base case, the Project Case shows the following performance changes:

- DoS increased by approximately 0.04 on the critical approaches.
- Queue lengths increased modestly (e.g. 1.2 m to 3.4 m in the PM peak on the minor road approach).
- Delays increased slightly but remained well below 10 seconds per vehicle.
- No movements dropped below Level of Service A.

The Project Case confirms that the intersection has adequate capacity to accommodate the proposed development without requiring any changes to the existing priority control or intersection geometry.

## 5.6 Project Case with Mitigation

SIDRA modelling was extended to include a scenario with an auxiliary left-turn lane (AUL) added to the Newell Highway southbound approach. This configuration was tested to determine whether enhanced geometry would improve intersection performance.

### 5.6.1 AM Peak – With Mitigation

With the left-turn treatment in place:

- The highest Degree of Saturation (DoS) observed was 0.052 on the Newell Highway north-east approach.
- The south-west right-turn movement showed a DoS of 0.046.

- Delays ranged from 5.7 to 7.2 seconds across all movements.
- The 95th percentile queue lengths remained modest, at a maximum of 1.6 m.
- All movements operated at Level of Service A.

#### 5.6.2 **PM Peak – With Mitigation**

During the PM peak:

- The highest DoS was observed on the Airport Road approach at 0.112.
- Right and left turns from Airport Road experienced delays of 6.7 to 7.2 seconds.
- The 95th percentile queue for the Airport Road approach reached 3.8 m.
- All other movements remained under 0.045 DoS.
- Level of Service remained at A for all vehicle movements.

#### 5.6.3 **Comparison with Project Case (No Mitigation)**

Compared with the standard project case:

- The AUL treatment slightly reduced delays and queues on the Airport Road approach.
- The left-turn movement benefits from reduced interaction with through traffic on Newell Highway.
- Although marginal, the operational improvements enhance safety by separating decelerating vehicles from high-speed through traffic.

The mitigation scenario confirms that the auxiliary left-turn treatment is not operationally essential under current forecasts. Still, it provides minor improvements and may be justified on safety grounds in the context of future growth or event-specific peak surges.

**Table 5-2 SIDRA Summary Results – Newell Highway / Airport Road Intersection**

Scenario	Peak	Highest DoS	Max Avg Delay (s)	Max 95% Queue (m)	Overall LoS
Base Case	AM	0.042	5.7	1.1	A
	PM	0.055	6.0	1.2	A
Project Case	AM	0.082	6.4	1.4	A
	PM	0.096	6.9	3.4	A
Project + Mitigation	AM	0.052	6.1	1.6	A
	PM	0.112	7.2	3.8	A

## 5.7 Modelling Interpretation & Conclusion

The SIDRA analysis across the Base Case, Project Case, and Project Case with Mitigation confirms that the give-way controlled intersection of Newell Highway and Airport Road operates well within capacity in all assessed scenarios. The intersection functions as a priority T-junction, with through and turning movements on Newell Highway holding right of way and Airport Road approaches operating under give-way control.

In the Base Case, the intersection performs at a high level of service, with Degree of Saturation (DoS) values well below 0.06, average delays under 7 seconds, and minimal queuing on the minor approach. These results are consistent with the site's rural context and the low-volume nature of background traffic.

Performance remains satisfactory in the Project Case, which includes an additional 60 vehicles per hour during each peak period (based on development-generated traffic). The majority of new traffic affects the minor leg (Airport Road), particularly outbound movements during the PM peak. The right-turn movement from Airport Road, which must yield to two-way through traffic on Newell Highway, experiences a DoS increase to 0.096 and 95th percentile queue lengths up to 3.4 m. Nonetheless, all movements retain Level of Service A and delays remain under 8 seconds. The increased traffic loading does not result in any significant deterioration in intersection performance.

In the Project Case with Mitigation, an Auxiliary Left Turn (AUL) treatment is added to the Newell Highway southbound approach. While this does not alter intersection control, it does assist with traffic flow and safety by removing decelerating vehicles from the through lane. This modification reduces DoS and delays marginally, with queue lengths falling slightly for affected movements. However, the intersection still operates at Level of Service A across all movements without the mitigation, indicating that the upgrade is not operationally required.

The modelling confirms that while the give-way controlled Airport Road approach experiences the highest sensitivity to added traffic, performance remains well within acceptable thresholds. The mitigation measure offers a minor operational benefit and a modest improvement to safety through vehicle separation.

Overall, the intersection can accommodate development traffic without upgrade, and any proposed mitigation may be considered for safety enhancement rather than operational necessity.



## 6 Access and Internal Layout Assessment

### 6.1 Access Driveway Configuration

The development will be accessed via a sealed driveway connection from Airport Road, approximately 150 metres from the Newell Highway intersection. The driveway provides a direct entry point into the primary internal access corridor, allowing for efficient movement of inbound vehicles during event peaks.

The access point is aligned to support safe turning movements with minimal conflict and sufficient width to accommodate two-way traffic, including vehicle-trailer combinations. The driveway intersects with the internal loop road that distributes traffic toward the parking area, spectator mound, and track facilities. Entry geometry complies with AS2890.1 (Clause 3.2), ensuring adequate turning paths for vehicles approaching from either direction on Airport Road.

### 6.2 Swept Path and Turning Templates

Swept path assessments have confirmed that the proposed driveway and circulation layout can accommodate 12.5 m Heavy Rigid Vehicles (HRVs) and emergency vehicles. Vehicle turning paths allow for entry and exit without the need for reversing or manoeuvring over unsealed verges. Turning templates are compliant with Austroads Design Vehicle guidelines, and transitions are suitable for dual-axle trailers often associated with motorsport logistics.

No internal conflict is expected under normal or event conditions, and the provision of large-radius curves at entry and egress points supports uninterrupted vehicle flow.

### 6.3 Internal Circulation

The internal road network follows a loop configuration, allowing circulation from the driveway entrance through the event precinct and back out toward Airport Road. The loop road connects directly to designated parking bays and staging zones, with an intuitive one-way flow supported by natural geometry.

Circulation design supports the segregation of spectator vehicles and service vehicle paths, reducing the likelihood of conflict near the spectator mound and track edge. Culverts No. 1 and 3 are integrated beneath internal access points to manage stormwater flows and maintain continuous vehicle access even under wet conditions.

Pavement treatments are anticipated to be sealed or compacted gravel, sufficient for rural event use. Road widths and clearances allow for simultaneous bidirectional movements, and verge zones have been retained to accommodate turning error margins and emergency stops.

### 6.4 Pedestrian Access and Spectator Movement

Pedestrian access within the site is facilitated via connections between the parking area and the spectator mound, which is elevated for safe viewing above the circuit level. The internal layout ensures that pedestrian pathways remain clear of vehicle access lanes during events.

Although dedicated footpaths are not proposed, movement corridors are sufficiently wide to support informal pedestrian flows. Temporary barriers or marshal-managed crossings may be used during high-demand periods to maintain safe pedestrian circulation.

No cycling infrastructure is proposed due to the remote, event-based nature of the development and lack of connection to surrounding active transport networks.

### 6.5 Compliance with AS2890 Series

The proposed access and internal layout have been reviewed against relevant Australian Standards and are deemed compliant with the following:

- **AS2890.1–2004** – General vehicle access and parking dimensions (applies to all standard bays and circulation paths).

- **AS2890.2–2018** – Service vehicle access (confirmed through turning templates for HRV entry and exit).
- **AS2890.6–2022** – Accessible parking requirements (2 accessible bays to be positioned adjacent to spectator entry).

The proposed layout provides sufficient width, turning radius, and delineation to safely accommodate the expected vehicle mix and volume during event operations, including spectators, trailers, service vehicles, and emergency response units.

## 7 Parking Assessment

### 7.1 Estimated Parking Demand

The parking demand for the proposed motorsport park was estimated based on projected attendance during typical event operations and comparable facility benchmarks. The facility is expected to cater to local and regional-level motorsport events, generally attended by:

- Drivers and race teams
- Spectators
- Event staff, officials, and marshals
- Support vehicles including trailers and transport Utes

A peak event was assumed to attract approximately 200 persons on-site during the busiest hour. Vehicle-based arrival assumptions were developed based on the following parameters:

**Table 7-1 Estimated Vehicle Arrival Rate During Events**

User Group	Estimated Persons	Occupancy Ratio	Estimated Vehicles	Notes
Drivers & Crew	60	1.5	40	Includes drivers and at least one pit crew
Spectators	120	2.0	60	Primarily local arrivals in private vehicles
Officials & Volunteers	20	1.0	20	Arriving individually or in pairs
<b>Total</b>	<b>200</b>	—	<b>120</b>	Inclusive of support vehicles

Of the total estimated 120 vehicles:

- Approximately 20 to 25 may be towing trailers or be light trucks/Utes with support equipment.
- The remainder are expected to be standard passenger vehicles.

Thus, the estimated peak parking demand is approximately 120 spaces, noting that this figure includes informal requirements for larger parking footprints due to trailers.

However, not all attendees arrive at the same time. Based on event phasing and staggered arrivals/departures (e.g. staff arriving early, spectators arriving closer to start), a coincident peak parking occupancy of approximately 80–90 vehicles is considered a reasonable design allowance.

### 7.2 Provision in Proposed Layout

As shown in the concept design, the proposed parking arrangement includes approximately 80 delineated spaces located immediately south of the racetrack and adjacent to the spectator mound. The layout is supported by a looped internal access road and is designed to accommodate:

- Standard parking bays (2.5 m x 5.4 m)
- Informal trailer-compatible spaces at the periphery and oversize bays
- Parallel or angled bays along internal roads, consistent with rural site practice

The layout allows for direct pedestrian access to the spectator mound while maintaining spatial separation from the track and driveway circulation. Sightlines and vehicle manoeuvring have been reviewed and found to be sufficient for both cars and trailers.

### **7.3 Accessible Parking Provision**

Two accessible parking spaces will be provided in accordance with AS2890.6–2022, located close to the primary pedestrian route leading to the spectator area. These will be line-marked and signed to ensure usability and compliance, and surface treatments will be level and suitable for wheelchair access.

### **7.4 Overflow and Contingency Parking**

Beyond the 80 formalised bays, the site features grassed and unsealed shoulders that provide overflow or temporary event-specific parking flexibility, particularly during larger regional events. These areas can accommodate an additional 30 to 40 vehicles under managed conditions.

Event marshals or temporary signage may be used to guide overflow parking, with defined entry and exit paths to ensure safety and preserve emergency access routes.

### **7.5 Compliance and Functional Assessment**

The overall parking supply:

- Meets the calculated coincident demand of 80–90 vehicles.
- Is expandable through flexible overflow areas.
- Accounts for the unique mix of vehicles, including Utes, trailers, and spectator cars.
- Provides sufficient circulation and access width in compliance with AS2890.1 and AS2890.2.

The proposed layout is therefore considered sufficient and appropriate for the scale and nature of the development, with adequate flexibility for operational peak conditions and future event growth.

### **7.6 Service and Refuse Vehicle Access**

Service and waste collection operations for the proposed motorsport facility will be accommodated via the internal looped access road, which is designed to support Heavy Rigid Vehicles (HRVs) and similar commercial service vehicles.

The internal circulation layout allows for direct and uninterrupted access by waste collection trucks and delivery/service vehicles without requiring reversing movements or complex manoeuvres. Refuse collection points are to be positioned in proximity to the spectator and pit zones, with clear access provided from the driveway and return route to Airport Road.

Swept path assessments confirm that HRVs can navigate the entry driveway, circulate through the site, and exit without the need for off-tracking or encroachment into pedestrian areas. Turning radii, verge clearances, and entry/exit widths are consistent with the design requirements of AS2890.2–2018 (Off-Street Commercial Vehicle Facilities).

In the context of event operations, waste collection and supply deliveries are expected to occur outside of peak spectator arrival and departure times, minimising potential conflict. If servicing is required during event hours, temporary traffic management (e.g. marshal control or restricted access zones) can be implemented to maintain separation between pedestrians and large vehicles.

Service vehicle access is considered practical, safe, and compliant with industry standards, supporting ongoing operations and maintenance of the motorsport facility without compromising the performance of the internal road network or parking areas.

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## 8 Safety Considerations

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### 8.1 Access Sight Distance

The proposed access driveway onto Airport Road provides sufficient sight distance in both directions for vehicles entering and exiting the site. Based on the 60 km/h speed limit on Airport Road, the minimum Safe Intersection Sight Distance (SISD) of 105 metres is achieved, in accordance with Austroads Guide to Road Design Part 4A. The access location is positioned away from any horizontal or vertical curves that would otherwise constrain visibility.

### 8.2 Conflict Points

Given the single point of access and the low frequency of vehicle movement outside of events, the potential for traffic conflicts is low. On event days, temporary traffic management may be implemented to manage peak arrival and departure flows. The internal layout separates spectator parking from service and operational areas, which assists in reducing internal circulation conflict.

No significant pedestrian activity is anticipated on the external road network due to the rural context of the site. Internal pedestrian movement will be managed within designated areas during events, with options for temporary fencing, marshals, or barriers where necessary.

### 8.3 Emergency Access

The site layout allows for emergency vehicle access from Airport Road through the primary driveway. Sufficient swept path clearances are provided for standard fire appliances and ambulances in accordance with AS 2890.2:2018. Internal areas have been designed to allow for turning movements and through access, enabling emergency vehicles to reach key operational areas on-site without obstruction.

### 8.4 Traffic Management for Events

A Traffic Management Plan (TMP) may be prepared and implemented for larger events or those attracting regional attendance. The TMP would address ingress/egress arrangements, marshal locations, pedestrian management, temporary signage, and communication with emergency services. This plan can be prepared in consultation with Council and local police for significant events.



## 9 Mitigation Measures and Recommendations

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Based on the findings of this assessment, the following mitigation measures are recommended to ensure safe and efficient operation of the development:

- The existing CHR(S) treatment on Newell Highway for northbound right-turn movements into Airport Road is considered adequate for the assessed development traffic. No immediate upgrade to a full CHR is required under current volumes.
- Consideration may be given to providing an auxiliary left-turn lane (AUL) on the southbound approach of Newell Highway, particularly if traffic volumes increase, or larger events are planned. This would provide improved deceleration and safety benefits.
- Clear advance signage should be installed on Newell Highway to indicate the turn-off to the Motorsport Park, particularly during events.
- Internal directional signage should be used to manage parking and service vehicle movements on event days.
- Accessible parking spaces should be designated and signposted in accordance with AS 2890.6.
- Temporary traffic marshals are recommended during peak periods to assist with traffic flow, parking coordination, and pedestrian safety.
- A site-specific Traffic Management Plan should be developed for major events, including risk mitigation measures, staging details, and coordination with emergency services.

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## 10 Conclusion

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This Traffic Impact Assessment has been prepared to accompany a development application for the proposed Western Wheelers Motorsport Park adjacent to West Wyalong Airport. The assessment has considered the proposed development's traffic generation, parking demand, access arrangements, and interaction with the surrounding road network.

The traffic generated by the development is expected to be concentrated during periodic events and is not anticipated to adversely impact the surrounding road network. The existing intersection of Newell Highway and Airport Road operates within capacity under both existing and project scenarios. No immediate upgrade works are considered necessary; however, future treatments may be implemented if growth or higher-than-forecast usage are warranted.

The internal site layout can accommodate the forecast parking demand and supports the safe movement of vehicles, including emergency and service access. No road safety issues were identified that would preclude the development from proceeding, and the access geometry complies with relevant Austroads and Australian Standard design requirements.

Subject to implementation of the recommendations in this report, the proposed development is considered to have an acceptable traffic and parking outcome and is supported from a traffic engineering perspective.

## **APPENDIX A**

### **SIDRA LAYOUTS & MODEL RESULTS**

## SITE LAYOUT

 Site: [1] 101 Newell Hwy-Airport Rd AM Base (Base Case)

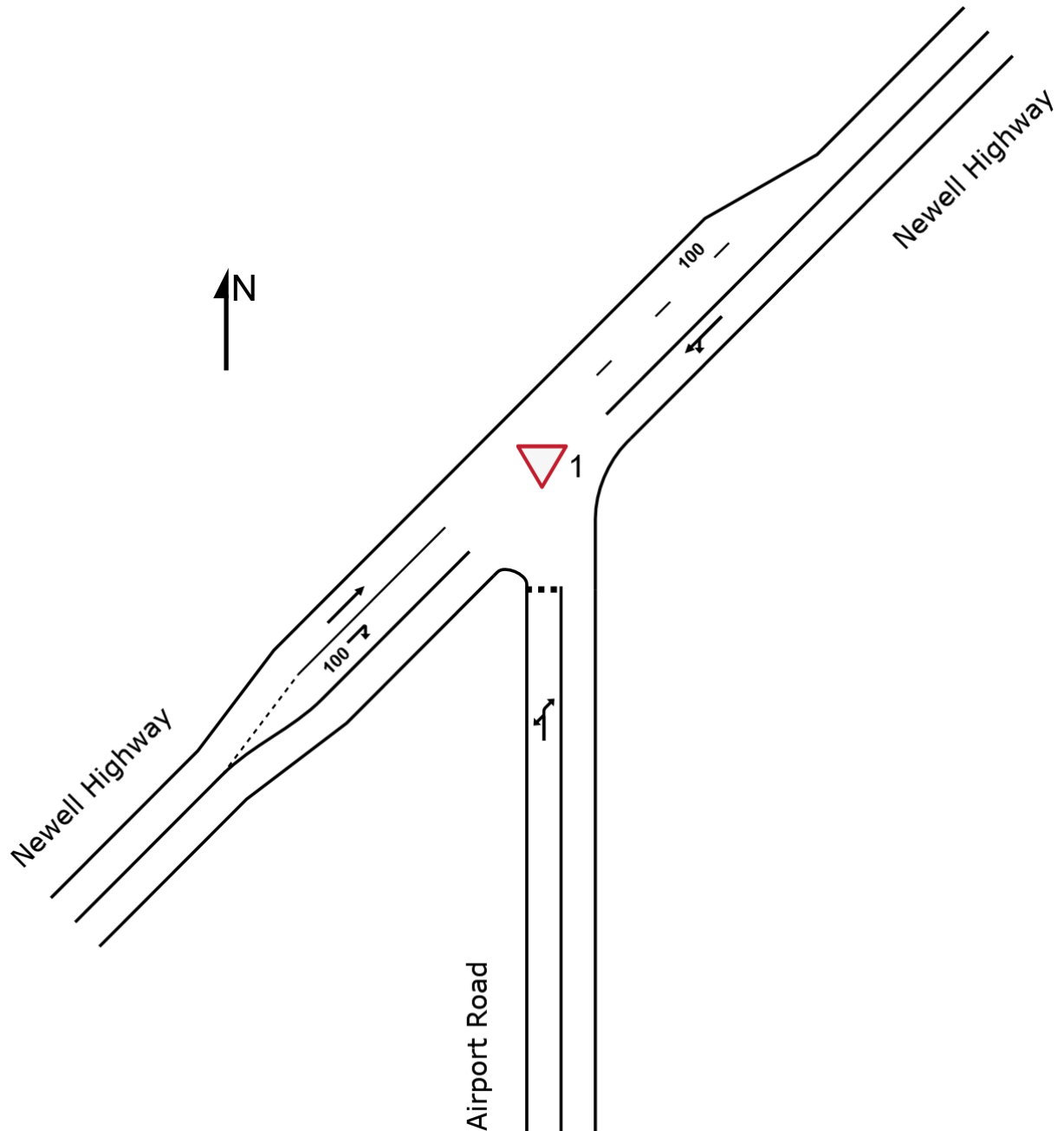
New Site

Site Category: (None)

Give-Way (Two-Way)

Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Organisation: GREYS CONSULTING | Licence: NETWORK / 1PC | Created: Monday, 21 April 2025 1:07:11 PM

Project: C:\Users\Alex\OneDrive - Greys Australia Pty Ltd\2025 Jobs\Projects\P2101 Western Wheelers Motorsport Park Wyalong Airport TIS  
Sidra Outputs\P2101.001M Western Wheelers Motorsport Park Wyalong Airport TIS Base & Project Cases.sipx

# MOVEMENT SUMMARY

Site: [1] 101 Newell Hwy-Airport Rd AM Base (Base Case)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Demand Flows		Arrival Flows		Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue		Prop. Qued	Eff. Stop of Cycles	Number of Cycles	Aver. Speed
			[ Total HV ]	[ Total HV ]						[ Veh.	Dist ]		Rate to Depart		
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Airport Road															
1b	L3	All MCs	16	20.0	16	20.0	0.035	6.9	LOS A	0.1	1.1	0.21	0.54	0.21	51.5
3a	R1	All MCs	21	25.0	21	25.0	0.035	6.0	LOS A	0.1	1.1	0.21	0.54	0.21	51.5
Approach			37	22.9	37	22.9	0.035	6.4	LOS A	0.1	1.1	0.21	0.54	0.21	51.5
NorthEast: Newell Highway															
24a	L1	All MCs	19	27.8	19	27.8	0.042	7.3	LOS A	0.0	0.0	0.00	0.19	0.00	62.7
25	T1	All MCs	47	35.6	47	35.6	0.042	0.0	LOS A	0.0	0.0	0.00	0.19	0.00	80.1
Approach			66	33.3	66	33.3	0.042	2.1	NA	0.0	0.0	0.00	0.19	0.00	74.2
SouthWest: Newell Highway															
31	T1	All MCs	60	29.8	60	29.8	0.037	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	11	30.0	11	30.0	0.007	6.7	LOS A	0.0	0.3	0.18	0.57	0.18	50.3
Approach			71	29.9	71	29.9	0.037	1.0	NA	0.0	0.3	0.03	0.09	0.03	87.1
All Vehicles			174	29.7	174	29.7	0.042	2.6	NA	0.1	1.1	0.06	0.22	0.06	71.8

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).  
Vehicle movement LOS values are based on average delay per movement.  
Minor Road Approach LOS values are based on average delay for all vehicle movements.  
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).  
Two-Way Sign Control Capacity Model: SIDRA Standard.  
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.  
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.



# QUEUE ANALYSIS

Site: [1] 101 Newell Hwy-Airport Rd AM Base (Base Case)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Lane Queues (Distance)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (m)	Back of Queue (m)		Queue at Start of Gap (m)		Cycle-Average Queue (m)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
v/c					Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.035	1.000	0.0	0.5	1.1	0.4	1.1	0.1	0.1	0.00	0.00	0.0	NA	NA
Approach		0.035			0.5	1.1	0.4	1.1	0.1	0.1	0.00	0.00			
NorthEast: Newell Highway															
Lane 1	Y	0.042	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.042			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.037			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection		0.042			0.5	1.1	0.4	1.1	0.1	0.1	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Lane Queues (Vehicles)																
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (veh)	Back of Queue (veh)		Queue at Start of Gap (veh)		Cycle-Average Queue (veh)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.	
v/c				Av.		95%	Av.		95%	Av.		95%	Av.	95%	%	%
South: Airport Road																
Lane 1		0.035	1.000	0.0	0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00	0.0	NA	NA	
Approach		0.035			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00				
NorthEast: Newell Highway																
Lane 1	Y	0.042	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA	
Approach		0.042			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00				
SouthWest: Newell Highway																
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA	
Lane 2		0.007	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1	
Approach		0.037			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00				
Intersection		0.042			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00				

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Continuous Lane Performance													
Lane Number	Deg. Satn	Unint. Speed	Unint. Travel Delay	Hdwy	Spacing	Aver. Vehicle Length	Occup. Time	Space Time	Space Occup. Ratio	Time Occup. Ratio	Density	LOS	
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	Method
NorthEast: Newell Highway													
Lane 1	0.042	84.0	0.0	54.29	1266.3	6.3	0.36	53.93	0.5	0.7	0.8	1.0	LOS A
SouthWest: Newell Highway													
Lane 1	0.037	100.0	0.0	60.00	1666.4	6.1	0.29	59.71	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

# MOVEMENT SUMMARY

▽ Site: [1 (3)] 101 Newell Hwy-Airport Rd AM Project Case  
(Project Case)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	Demand Flows		Arrival Flows		Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue		Prop. Qued	Eff. Stop Rate	Number of Cycles to Depart	Aver. Speed
			[ Total HV ]	[ Total HV ]						[ Veh. ]	Dist ]				
			veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Airport Road															
1b	L3	All MCs	16	20.0	16	20.0	0.038	6.9	LOS A	0.1	1.2	0.23	0.55	0.23	51.4
3a	R1	All MCs	21	25.0	21	25.0	0.038	6.6	LOS A	0.1	1.2	0.23	0.55	0.23	51.3
Approach			37	22.9	37	22.9	0.038	6.7	LOS A	0.1	1.2	0.23	0.55	0.23	51.3
NorthEast: Newell Highway															
24a	L1	All MCs	82	29.5	82	29.5	0.082	6.5	LOS A	0.0	0.0	0.00	0.40	0.00	52.9
25	T1	All MCs	47	35.6	47	35.6	0.082	0.0	LOS A	0.0	0.0	0.00	0.40	0.00	65.3
Approach			129	31.7	129	31.7	0.082	4.1	NA	0.0	0.0	0.00	0.40	0.00	56.9
SouthWest: Newell Highway															
31	T1	All MCs	60	29.8	60	29.8	0.037	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	48	30.4	48	30.4	0.036	7.0	LOS A	0.2	1.4	0.28	0.59	0.28	50.1
Approach			108	30.1	108	30.1	0.037	3.1	NA	0.2	1.4	0.12	0.26	0.12	69.2
All Vehicles			275	29.9	275	29.9	0.082	4.1	NA	0.2	1.4	0.08	0.37	0.08	60.2

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).  
Vehicle movement LOS values are based on average delay per movement.  
Minor Road Approach LOS values are based on average delay for all vehicle movements.  
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).  
Two-Way Sign Control Capacity Model: SIDRA Standard.  
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.  
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

# QUEUE ANALYSIS

▼ Site: [1 (3)] 101 Newell Hwy-Airport Rd AM Project Case  
(Project Case)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Lane Queues (Distance)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (m)	Back of Queue (m)		Queue at Start of Gap (m)		Cycle-Average Queue (m)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
v/c				Av.		95%	Av.		95%	Av.		95%	%		%
South: Airport Road															
Lane 1		0.038	1.000	0.0	0.5	1.2	0.5	1.2	0.1	0.2	0.00	0.00	0.0	NA	NA
Approach		0.038			0.5	1.2	0.5	1.2	0.1	0.2	0.00	0.00			
NorthEast: Newell Highway															
Lane 1	Y	0.082	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.082			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.036	1.000	0.0	0.6	1.4	0.6	1.4	0.1	0.1	0.01	0.01	NA	0.0	1
Approach		0.037			0.6	1.4	0.6	1.4	0.1	0.1	0.00	0.00			
Intersection		0.082			0.6	1.4	0.6	1.4	0.1	0.2	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Lane Queues (Vehicles)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (veh)	Back of Queue (veh)		Queue at Start of Gap (veh)		Cycle-Average Queue (veh)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
v/c					Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.038	1.000	0.0	0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.038			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			
NorthEast: Newell Highway															
Lane 1	Y	0.082	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.082			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.036	1.000	0.0	0.1	0.2	0.1	0.2	0.0	0.0	0.01	0.01	NA	0.0	1
Approach		0.037			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			
Intersection		0.082			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Continuous Lane Performance													
Lane Number	Deg. Satn	Unint. Speed	Unint. Travel Delay	Hdwy Spacing	Aver. Vehicle Length	Occup. Time	Space Time	Space Occup. Ratio	Time Occup. Ratio	Density	LOS (Density Method)		
	v/c	km/h	sec	sec	m	sec	sec	%	%	veh/km	pc/km		
NorthEast: Newell Highway													
Lane 1	0.082	70.2	0.0	27.80	542.5	6.2	0.42	27.38	1.2	1.5	1.8	2.3	LOS A
SouthWest: Newell Highway													
Lane 1	0.037	100.0	0.0	60.00	1666.4	6.1	0.29	59.71	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

# MOVEMENT SUMMARY

Site: [1 (2)] 101 Newell Hwy-Airport Rd PM Base (Base Case)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Demand Flows [ Total HV ] veh/h %	Arrival Flows [ Total HV ] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back Of Queue [ Veh. Dist ] veh m		Prop. Qued	Eff. Stop Rate	Number of Cycles to Depart	Aver. Speed km/h	
South: Airport Road														
1b	L3	All MCs	16 20.0	16 20.0	0.036	7.0	LOS A	0.1	1.2	0.25	0.55	0.25	51.4	
3a	R1	All MCs	21 25.0	21 25.0	0.036	6.2	LOS A	0.1	1.2	0.25	0.55	0.25	51.3	
Approach			37 22.9	37 22.9	0.036	6.5	LOS A	0.1	1.2	0.25	0.55	0.25	51.4	
NorthEast: Newell Highway														
24a	L1	All MCs	19 27.8	19 27.8	0.055	7.5	LOS A	0.0	0.0	0.00	0.14	0.00	65.4	
25	T1	All MCs	71 31.3	71 31.3	0.055	0.0	LOS A	0.0	0.0	0.00	0.14	0.00	84.5	
Approach			89 30.6	89 30.6	0.055	1.6	NA	0.0	0.0	0.00	0.14	0.00	79.6	
SouthWest: Newell Highway														
31	T1	All MCs	61 32.8	61 32.8	0.038	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0	
32b	R3	All MCs	11 30.0	11 30.0	0.007	6.8	LOS A	0.0	0.3	0.22	0.57	0.22	50.2	
Approach			72 32.4	72 32.4	0.038	1.0	NA	0.0	0.3	0.03	0.08	0.03	87.3	
All Vehicles			198 29.8	198 29.8	0.055	2.3	NA	0.1	1.2	0.06	0.20	0.06	74.3	

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).  
Vehicle movement LOS values are based on average delay per movement.  
Minor Road Approach LOS values are based on average delay for all vehicle movements.  
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).  
Two-Way Sign Control Capacity Model: SIDRA Standard.  
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.  
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.



# QUEUE ANALYSIS

Site: [1 (2)] 101 Newell Hwy-Airport Rd PM Base (Base Case)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Lane Queues (Distance)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (m)	Back of Queue (m)		Queue at Start of Gap (m)		Cycle-Average Queue (m)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.036	1.000	0.0	0.5	1.2	0.5	1.1	0.1	0.1	0.00	0.00	0.0	NA	NA
Approach		0.036			0.5	1.2	0.5	1.1	0.1	0.1	0.00	0.00			
NorthEast: Newell Highway															
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection		0.055			0.5	1.2	0.5	1.1	0.1	0.1	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Lane Queues (Vehicles)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (veh)	Back of Queue (veh)		Queue at Start of Gap (veh)		Cycle-Average Queue (veh)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
v/c					Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.036	1.000	0.0	0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.036			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			
NorthEast: Newell Highway															
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
Intersection		0.055			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Continuous Lane Performance													
Lane Number	Deg. Satn	Unint. Speed	Unint. Travel Delay	Hdwy	Spacing	Aver. Vehicle Length	Occup. Time	Space Time	Space Occup. Ratio	Time Occup. Ratio	Density	LOS	
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	Method
NorthEast: Newell Highway													
Lane 1	0.055	87.6	0.0	40.24	979.0	6.2	0.34	39.90	0.6	0.8	1.0	1.3	LOS A
SouthWest: Newell Highway													
Lane 1	0.038	100.0	0.0	58.97	1637.6	6.3	0.30	58.67	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

# MOVEMENT SUMMARY

Site: [1 (4)] 101 Newell Hwy-Airport Rd PM Project Case  
(Project Case)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Vehicle Movement Performance													
Mov ID	Turn	Mov Class	Demand Flows [ Total HV ] veh/h %	Arrival Flows [ Total HV ] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back Of Queue [ Veh. Dist ] veh m		Prop. Qued	Eff. Stop Rate	Number of Cycles to Depart	Aver. Speed km/h
South: Airport Road													
1b	L3	All MCs	54 27.5	54 27.5	0.096	7.1	LOS A	0.4	3.4	0.25	0.56	0.25	51.0
3a	R1	All MCs	46 27.3	46 27.3	0.096	6.4	LOS A	0.4	3.4	0.25	0.56	0.25	51.1
Approach			100 27.4	100 27.4	0.096	6.8	LOS A	0.4	3.4	0.25	0.56	0.25	51.1
NorthEast: Newell Highway													
24a	L1	All MCs	19 27.8	19 27.8	0.055	7.5	LOS A	0.0	0.0	0.00	0.14	0.00	65.4
25	T1	All MCs	71 31.3	71 31.3	0.055	0.0	LOS A	0.0	0.0	0.00	0.14	0.00	84.5
Approach			89 30.6	89 30.6	0.055	1.6	NA	0.0	0.0	0.00	0.14	0.00	79.6
SouthWest: Newell Highway													
31	T1	All MCs	61 32.8	61 32.8	0.038	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	11 30.0	11 30.0	0.007	6.8	LOS A	0.0	0.3	0.22	0.57	0.22	50.2
Approach			72 32.4	72 32.4	0.038	1.0	NA	0.0	0.3	0.03	0.08	0.03	87.3
All Vehicles			261 29.8	261 29.8	0.096	3.4	NA	0.4	3.4	0.10	0.29	0.10	66.8

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).  
Vehicle movement LOS values are based on average delay per movement.  
Minor Road Approach LOS values are based on average delay for all vehicle movements.  
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).  
Two-Way Sign Control Capacity Model: SIDRA Standard.  
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).  
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.  
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

# QUEUE ANALYSIS

▼ Site: [1 (4)] 101 Newell Hwy-Airport Rd PM Project Case  
(Project Case)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Lane Queues (Distance)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (m)	Back of Queue (m)		Queue at Start of Gap (m)		Cycle-Average Queue (m)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.096	1.000	0.0	1.4	3.4	1.2	3.1	0.2	0.4	0.00	0.01	0.0	NA	NA
Approach		0.096			1.4	3.4	1.2	3.1	0.2	0.4	0.00	0.01			
NorthEast: Newell Highway															
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection		0.096			1.4	3.4	1.2	3.1	0.2	0.4	0.00	0.01			

Two-Way Sign Control Capacity Model: SIDRA Standard.  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Lane Queues (Vehicles)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (veh)	Back of Queue (veh)		Queue at Start of Gap (veh)		Cycle-Average Queue (veh)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
v/c					Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.096	1.000	0.0	0.2	0.4	0.1	0.4	0.0	0.0	0.00	0.01	0.0	NA	NA
Approach		0.096			0.2	0.4	0.1	0.4	0.0	0.0	0.00	0.01			
NorthEast: Newell Highway															
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
Intersection		0.096			0.2	0.4	0.1	0.4	0.0	0.0	0.00	0.01			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Continuous Lane Performance													
Lane Number	Deg. Satn	Unint. Speed	Unint. Travel Delay	Hdwy	Spacing	Aver. Vehicle Length	Occup. Time	Space Time	Space Occup. Ratio	Time Occup. Ratio	Density	LOS	
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	Method
NorthEast: Newell Highway													
Lane 1	0.055	87.6	0.0	40.24	979.0	6.2	0.34	39.90	0.6	0.8	1.0	1.3	LOS A
SouthWest: Newell Highway													
Lane 1	0.038	100.0	0.0	58.97	1637.6	6.3	0.30	58.67	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

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Project: C:\Users\Alex\OneDrive - Greys Australia Pty Ltd\2025 Jobs\Projects\P2101 Western Wheelers Motorsport Park Wyalong Airport TIS  
\Sidra Outputs\P2101.001M Western Wheelers Motorsport Park Wyalong Airport TIS Base & Project Cases.sipx

## SITE LAYOUT

▽ Site: [1 (5)] 101 Newell Hwy-Airport Rd AM Project Case with Mitigation (Project Case wit Mitigation)

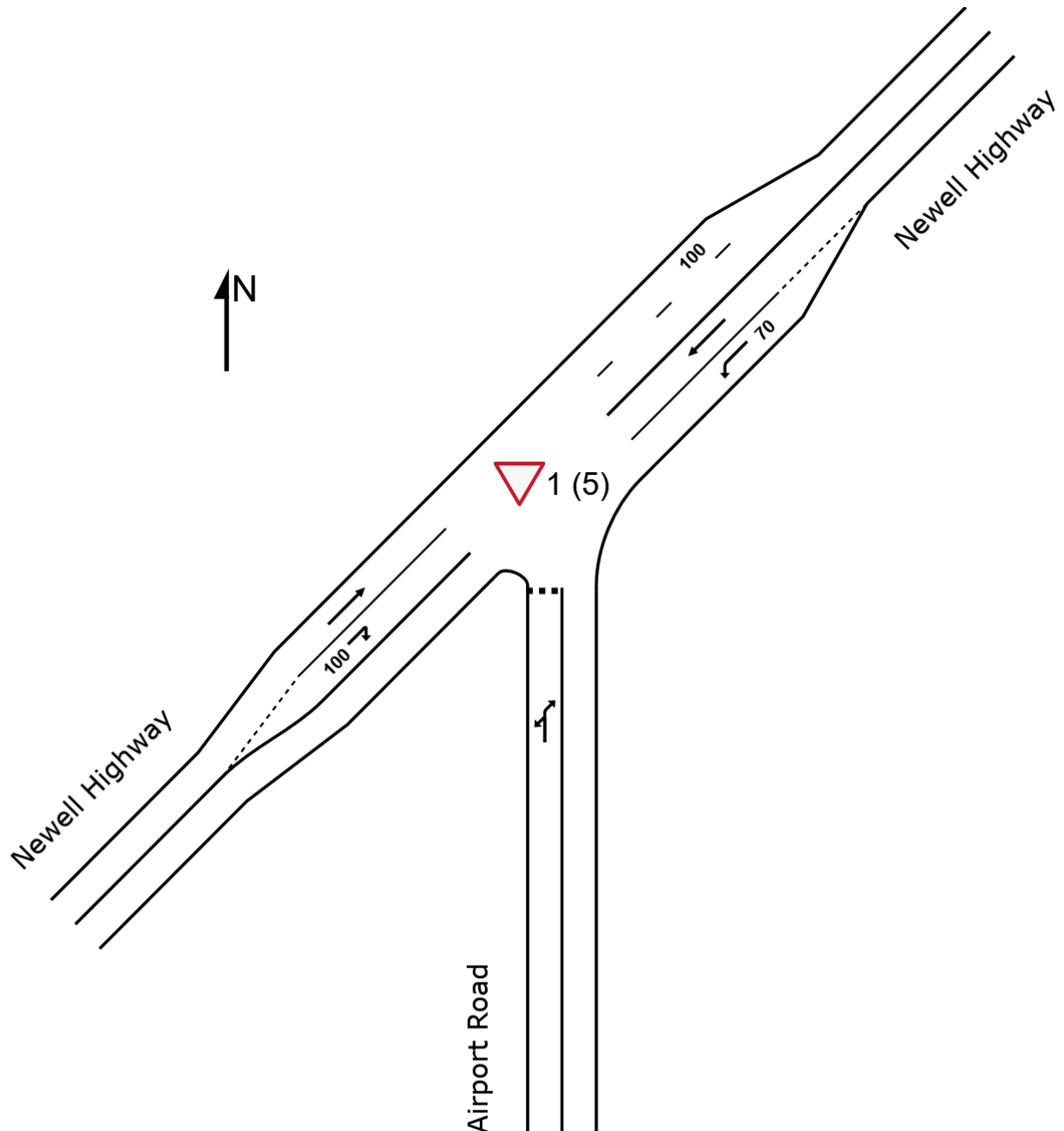
New Site

Site Category: (None)

Give-Way (Two-Way)

Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.





# MOVEMENT SUMMARY

Site: [1 (5)] 101 Newell Hwy-Airport Rd AM Project Case with Mitigation (Project Case wit Mitigation)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Demand Flows		Arrival Flows		Deg. Satn	Aver. Delay	Level of Service	95% Back Of Queue		Prop. Qued	Eff. Number Stop of Cycles Rate to Depart	Aver. Speed
			[ Total HV ]	[ Total HV ]										
			veh/h	%	veh/h	%	v/c	sec		[ Veh. veh	Dist ] m			km/h
South: Airport Road														
1b	L3	All MCs	16	20.0	16	20.0	0.044	6.9	LOS A	0.2	1.4	0.25	0.55	51.3
3a	R1	All MCs	21	25.0	21	25.0	0.044	6.9	LOS A	0.2	1.4	0.25	0.55	51.3
Approach			37	22.9	37	22.9	0.044	6.9	LOS A	0.2	1.4	0.25	0.55	51.3
NorthEast: Newell Highway														
24a	L1	All MCs	82	29.5	82	29.5	0.052	5.7	LOS A	0.0	0.0	0.00	0.60	51.5
25	T1	All MCs	47	35.6	47	35.6	0.030	0.0	LOS A	0.0	0.0	0.00	0.00	100.0
Approach			129	31.7	129	31.7	0.052	3.6	NA	0.0	0.0	0.00	0.38	62.6
SouthWest: Newell Highway														
31	T1	All MCs	60	29.8	60	29.8	0.037	0.0	LOS A	0.0	0.0	0.00	0.00	100.0
32b	R3	All MCs	48	30.4	48	30.4	0.046	7.2	LOS A	0.2	1.6	0.28	0.60	50.1
Approach			108	30.1	108	30.1	0.046	3.2	NA	0.2	1.6	0.12	0.27	69.2
All Vehicles			275	29.9	275	29.9	0.052	3.9	NA	0.2	1.6	0.08	0.36	63.1

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).  
Vehicle movement LOS values are based on average delay per movement.  
Minor Road Approach LOS values are based on average delay for all vehicle movements.  
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).  
Two-Way Sign Control Capacity Model: SIDRA Standard.  
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).  
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.  
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

# QUEUE ANALYSIS

Site: [1 (5)] 101 Newell Hwy-Airport Rd AM Project Case with Mitigation (Project Case wit Mitigation)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Lane Queues (Distance)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (m)	Back of Queue (m)		Queue at Start of Gap (m)		Cycle-Average Queue (m)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.044	1.000	0.0	0.5	1.4	0.5	1.3	0.1	0.2	0.00	0.00	0.0	NA	NA
Approach		0.044			0.5	1.4	0.5	1.3	0.1	0.2	0.00	0.00			
NorthEast: Newell Highway															
Lane 1	Y	0.052	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.030	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.052			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.046	1.000	0.0	0.6	1.6	0.6	1.5	0.1	0.2	0.01	0.02	NA	0.0	1
Approach		0.046			0.6	1.6	0.6	1.5	0.1	0.2	0.00	0.00			
Intersection		0.052			0.6	1.6	0.6	1.5	0.1	0.2	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Lane Queues (Vehicles)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor	Overflow Queue	Back of Queue (veh)		Queue at Start of Gap (veh)		Cycle-Average Queue (veh)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
		v/c				Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%
South: Airport Road															
Lane 1		0.044	1.000	0.0	0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.044			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			
NorthEast: Newell Highway															
Lane 1	Y	0.052	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.030	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.052			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.046	1.000	0.0	0.1	0.2	0.1	0.2	0.0	0.0	0.01	0.02	NA	0.0	1
Approach		0.046			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			

Intersection	0.052	0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00
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Two-Way Sign Control Capacity Model: SIDRA Standard.  
 Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
 Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
 Short Lanes are not included in determining Queue Storage Ratios.

Continuous Lane Performance													
Lane Number	Deg. Satn	Unint. Speed	Unint. Travel Delay	Hdwy	Spacing	Aver. Vehicle Length	Occup. Time	Space Time	Space Occup. Ratio	Time Occup. Ratio	Density	LOS	
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	Method
NorthEast: Newell Highway													
Lane 1	0.052	60.0	0.0	43.85	730.4	6.1	0.49	43.36	0.8	1.1	1.4	1.7	LOS A
Lane 2	0.030	100.0	0.0	76.00	2110.8	6.5	0.30	75.70	0.3	0.4	0.5	0.6	LOS A
SouthWest: Newell Highway													
Lane 1	0.037	100.0	0.0	60.00	1666.4	6.1	0.29	59.71	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

# MOVEMENT SUMMARY

Site: [1 (6)] 101 Newell Hwy-Airport Rd PM Project Case with Mitigation (Project Case wit Mitigation)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Vehicle Movement Performance														
Mov ID	Turn	Mov Class	Demand Flows [ Total HV ] veh/h %	Arrival Flows [ Total HV ] veh/h %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Back Of Queue [ Veh. veh     Dist ] veh     m		Prop. Qued	Eff. Stop Rate	Number of Cycles to Depart	Aver. Speed km/h	
South: Airport Road														
1b	L3	All MCs	54 27.5	54 27.5	0.112	7.2	LOS A	0.4	3.8	0.26	0.57	0.26	51.0	
3a	R1	All MCs	46 27.3	46 27.3	0.112	6.6	LOS A	0.4	3.8	0.26	0.57	0.26	51.3	
Approach			100 27.4	100 27.4	0.112	6.9	LOS A	0.4	3.8	0.26	0.57	0.26	51.1	
NorthEast: Newell Highway														
24a	L1	All MCs	19 27.8	19 27.8	0.012	5.7	LOS A	0.0	0.0	0.00	0.60	0.00	51.6	
25	T1	All MCs	71 31.3	71 31.3	0.044	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0	
Approach			89 30.6	89 30.6	0.044	1.2	NA	0.0	0.0	0.00	0.13	0.00	83.4	
SouthWest: Newell Highway														
31	T1	All MCs	61 32.8	61 32.8	0.038	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0	
32b	R3	All MCs	11 30.0	11 30.0	0.009	6.9	LOS A	0.0	0.3	0.22	0.58	0.22	50.2	
Approach			72 32.4	72 32.4	0.038	1.0	NA	0.0	0.3	0.03	0.09	0.03	87.3	
All Vehicles			261 29.8	261 29.8	0.112	3.3	NA	0.4	3.8	0.11	0.28	0.11	67.8	

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).  
Vehicle movement LOS values are based on average delay per movement.  
Minor Road Approach LOS values are based on average delay for all vehicle movements.  
NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).  
Two-Way Sign Control Capacity Model: SIDRA Standard.  
Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.  
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

# QUEUE ANALYSIS

▼ Site: [1 (6)] 101 Newell Hwy-Airport Rd PM Project Case with Mitigation (Project Case wit Mitigation)  
Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site  
Site Category: (None)  
Give-Way (Two-Way)  
Site Scenario: 1 | Local Volumes

Lane Queues (Distance)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (m)	Back of Queue (m)		Queue at Start of Gap (m)		Cycle-Average Queue (m)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airport Road															
Lane 1		0.112	1.000	0.0	1.5	3.8	1.4	3.4	0.2	0.4	0.00	0.01	0.0	NA	NA
Approach		0.112			1.5	3.8	1.4	3.4	0.2	0.4	0.00	0.01			
NorthEast: Newell Highway															
Lane 1	Y	0.012	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.044	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.044			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.009	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection		0.112			1.5	3.8	1.4	3.4	0.2	0.4	0.00	0.01			

Two-Way Sign Control Capacity Model: SIDRA Standard.  
Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
Short Lanes are not included in determining Queue Storage Ratios.

Lane Queues (Vehicles)															
Lane Number	Contin. Lane	Deg. Satn	Prog. Factor (Queue)	Overflow Queue (veh)	Back of Queue (veh)		Queue at Start of Gap (veh)		Cycle-Average Queue (veh)		Queue Storage Ratio		Prob. Block.	Prob. SL Ov.	Ov. Lane No.
		v/c				Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%
South: Airport Road															
Lane 1		0.112	1.000	0.0	0.2	0.4	0.2	0.4	0.0	0.1	0.00	0.01	0.0	NA	NA
Approach		0.112			0.2	0.4	0.2	0.4	0.0	0.1	0.00	0.01			
NorthEast: Newell Highway															
Lane 1	Y	0.012	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.044	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.044			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest: Newell Highway															
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.009	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			

Intersection	0.112	0.2	0.4	0.2	0.4	0.0	0.1	0.00	0.01
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Two-Way Sign Control Capacity Model: SIDRA Standard.  
 Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.  
 Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).  
 Short Lanes are not included in determining Queue Storage Ratios.

Continuous Lane Performance													
Lane Number	Deg. Satn	Unint. Speed	Unint. Travel Delay	Hdwy	Spacing	Aver. Vehicle Length	Occup. Time	Space Time	Space Occup. Ratio	Time Occup. Ratio	Density	LOS	
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	Method
NorthEast: Newell Highway													
Lane 1	0.012	60.0	0.0	190.00	3166.4	6.0	0.48	189.52	0.2	0.3	0.3	0.4	LOS A
Lane 2	0.044	100.0	0.0	51.04	1417.6	6.2	0.30	50.75	0.4	0.6	0.7	0.9	LOS A
SouthWest: Newell Highway													
Lane 1	0.038	100.0	0.0	58.97	1637.6	6.3	0.30	58.67	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m



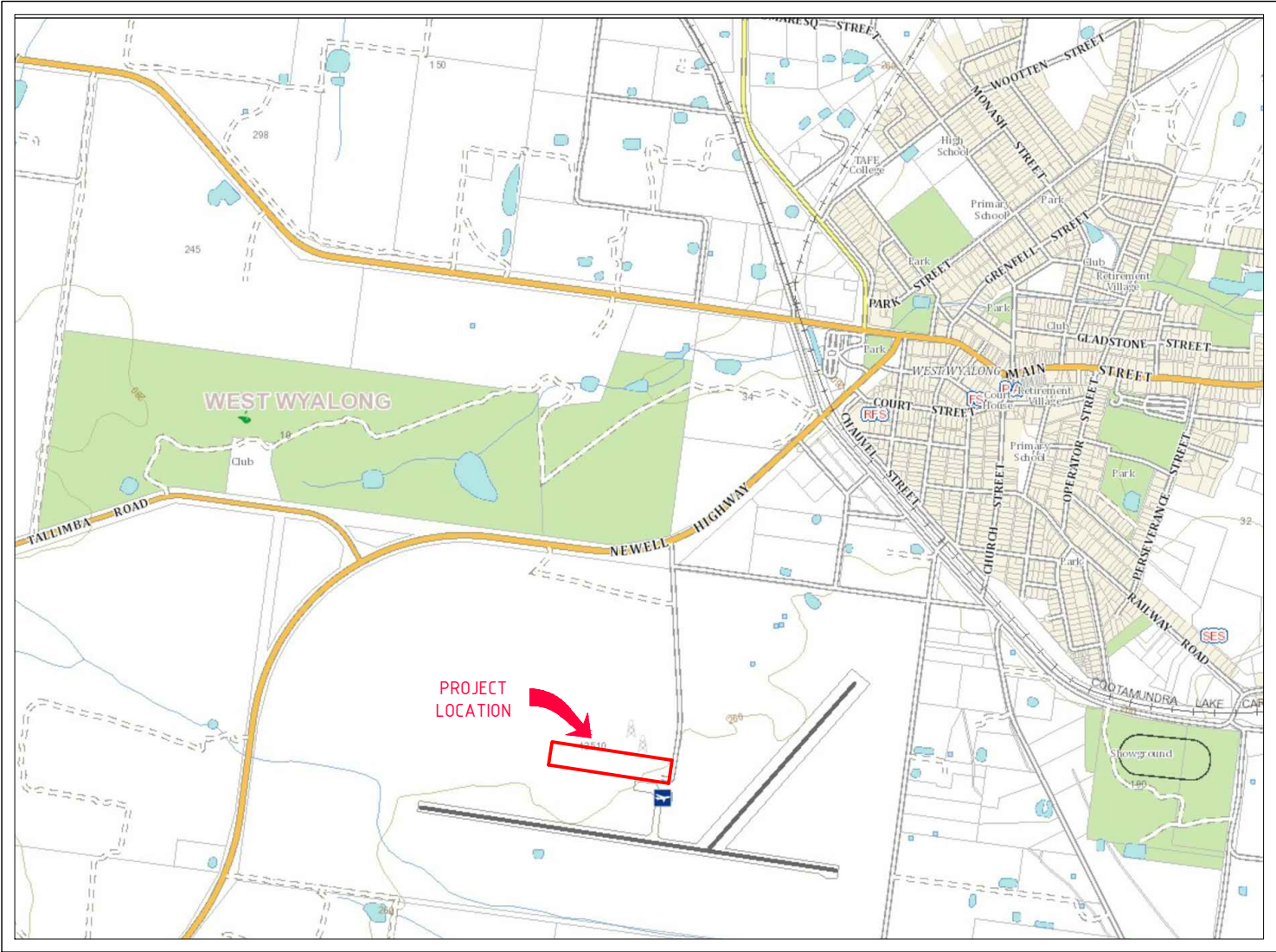
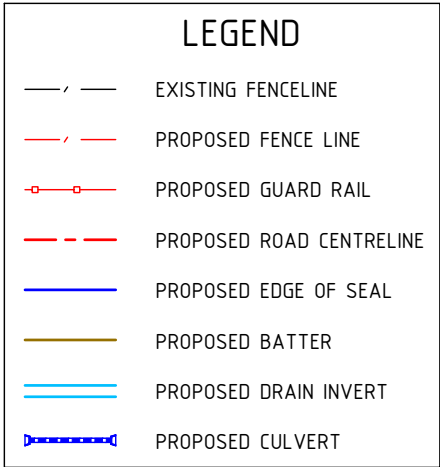
## **APPENDIX B**

### **DEVELOPMENT PLANS**

# WESTERN WHEELERS MOTORSPORT PARK

REVISION D  
AMENDED FOR COUNCIL APPROVAL  
25-08-2023

DRAWING SCHEDULE	
SHEET	DRAWING TITLE
0	COVER SHEET
1	NOTES & BILL OF QUANTITIES
2	TYPICAL CROSS SECTIONS
3	GENERAL LAYOUT PLAN
4	DRAG STRIP PLAN & LONG SECTION (-20 - 340)
5	DRAG STRIP PLAN & LONG SECTION (340 - 620)
6	DRAG STRIP CROSS SECTIONS (-20 - 40)
7	DRAG STRIP CROSS SECTIONS (60 - 120)
8	DRAG STRIP CROSS SECTIONS (140 - 200)
9	DRAG STRIP CROSS SECTIONS (221.17 - 280)
10	DRAG STRIP CROSS SECTIONS (300 - 360)
11	DRAG STRIP CROSS SECTIONS (380 - 440)
12	DRAG STRIP CROSS SECTIONS (460 - 520)
13	DRAG STRIP CROSS SECTIONS (525.17 - 580)
14	DRAG STRIP CROSS SECTIONS (600 - 616.17)
15	RETURN ROAD PLAN & LONG SECTION (0 - 360)
16	RETURN ROAD PLAN & LONG SECTION (360 - 560)
17	ROAD 2 CROSS SECTIONS (24.14 - 42.73)
18	ROAD 2 CROSS SECTIONS (50 - 62.87)
19	ROAD 2 CROSS SECTIONS (66.2 - 120)
20	ROAD 2 CROSS SECTIONS (140 - 200)
21	ROAD 2 CROSS SECTIONS 220 - 280)
22	ROAD 2 CROSS SECTIONS (300 - 360)
23	ROAD 2 CROSS SECTIONS (380 - 440)
24	ROAD 2 CROSS SECTIONS (460 - 520)
25	ROAD 2 CROSS SECTIONS (540 - 560)
26	ROAD 3 PLAN & LONG SECTION
27	ROAD 3 CROSS SECTIONS (19.3 - 50)
28	ROAD 3 CROSS SECTIONS (60 - 106.26)
29	ROAD 3 CROSS SECTIONS (110 - 127.44)
30	CULVERTS 1, 2 & 4 PROFILES
31	CULVERTS 5 & 6 PROFILES
32	EROSION & SEDIMENT CONTROL PLAN (WEST)
33	EROSION & SEDIMENT CONTROL PLAN (EAST)
34	EROSION & SEDIMENT CONTROL NOTES
35	EROSION & SEDIMENT CONTROL DETAILS



PROJECT LOCATION


## GENERAL

- ## STORMWATER

- ## ROADS

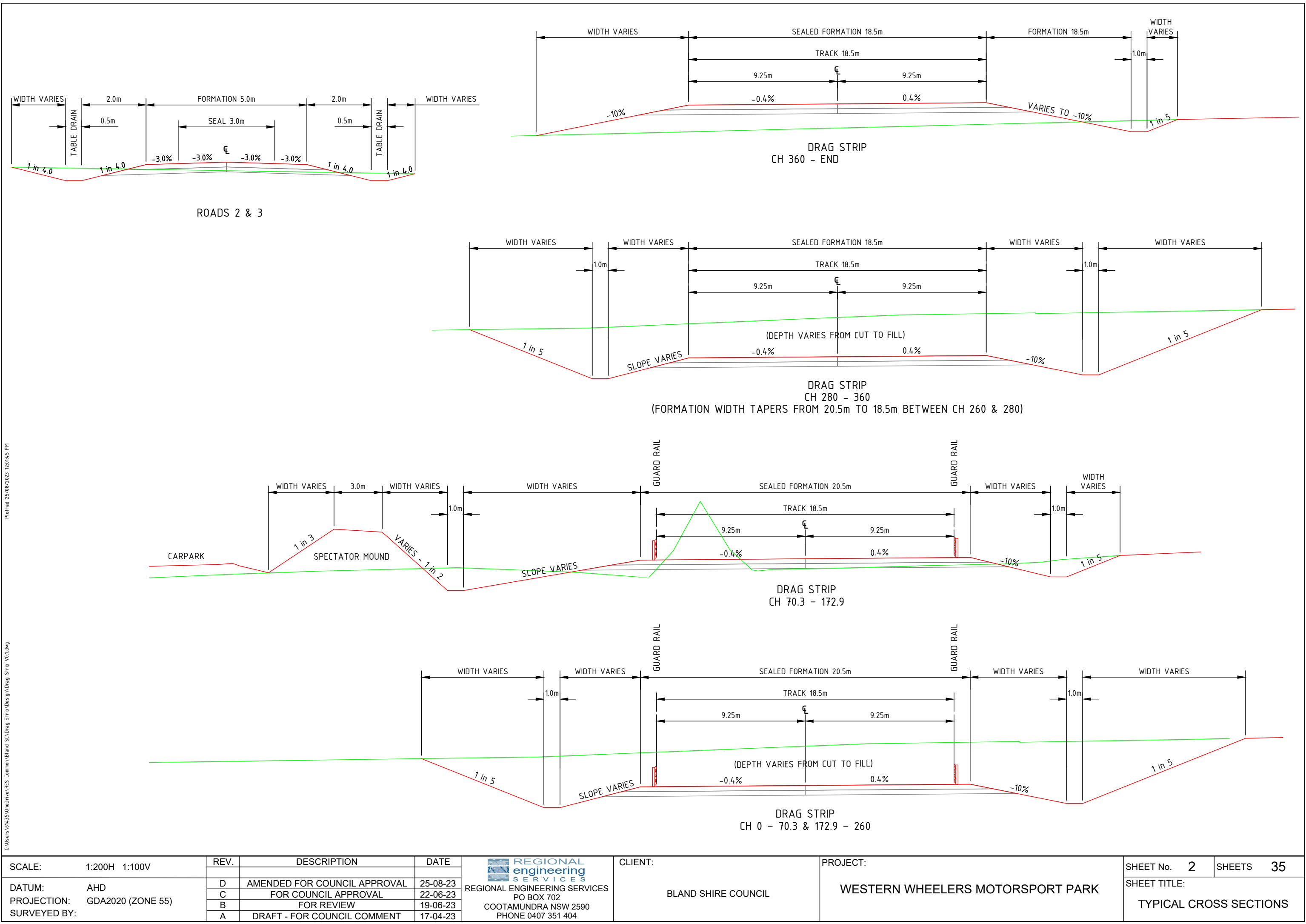
4. PROVIDE TWO COAT BITUMEN FLUSH SEAL WITH 14mm AND 7mm AGGREGATE LAYERS. BOTH COATS OF BITUMEN REQUIRED TO BE PROPERLY HEATED AND THE FINISHED SURFACE TO BE COMPACTED TO THE BLAND SHIRE COUNCIL'S REQUIREMENTS.
5. ROADWORKS TO BE CARRIED OUT IN ACCORDANCE WITH BLAND SHIRE COUNCIL.
6. CONTRACTOR MUST HAVE ALL SERVICES ACCURATELY LOCATED BY THE RELEVANT SERVICE AUTHORITY PRIOR TO COMMENCEMENT OF CONSTRUCTION.
7. CONTRACTOR MUST TAKE CARE NOT TO DISTURB TREES THAT ARE TO REMAIN.
8. ANY TREES TO BE REMOVED FROM THE PROPOSED ROAD FORMATION MUST HAVE ALL ROOTS GRUBBED FOR A MINIMUM DEPTH OF 500mm WITH ALL MAJOR ROOTS COMPLETELY REMOVED. REMAINING HOLES SHALL BE FULLY BACKFILLED WITH SELECT FILL MATERIAL AND THOROUGHLY COMPACTED TO 95% STANDARD DENSITY.
9. TABLE DRAINS SHALL BE SHAPED TO DIRECT WATER INTO CULVERTS.

Description	Unit	Quantity
<b>Earthworks</b>		
Cut to fill	m3	21200
Cut to spoil	m3	200
<b>Pipe Drainage</b>		
Single cell 450mm Dia. RRJ CL4 RCP	m	68.32
Concrete Headwalls 450mm RCP	each	11
Rock Riprap (d50 = 200mm, min. depth = 300mm)	m3	8
<b>Flexible Pavements</b>		
Drag Strip		
Base Course (150mm)	m3	1,760
Subbase Course (150mm)	m3	1,980
Return Roads		
Base Course (150mm)	m3	622
Subbase Course (150mm)	m3	745
Pit Area		
Base Course (150mm)	m3	1,000
Subbase Course (150mm)	m3	1,100
Parking Area		
Base Course (150mm)	m3	553
Subbase Course (150mm)	m3	608
<b>Two coat 14/7mm spray seal</b>		
Drag Strip	m2	10,666
Return Roads	m2	2,206
Pit Area	m2	6,665
Parking Area	m2	3,684
<b>Stopping Bed</b>		
Sand (91m x 18.5m x 0.3m)	m3	505
<b>Spectator Protection</b>		
Guardrail (total length including 4x terminals)	m	560
Fencing	m	331
<b>Erosion &amp; Sediment Control</b>		
Earth bund (1522m)	m3	380
Level spreader	ea.	2
Straw bales / coconut fibre logs (group)	locations	64

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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE:  NOTES & BILL OF QUANTITIES					
	C	FOR COUNCIL APPROVAL	22-06-23						
	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						


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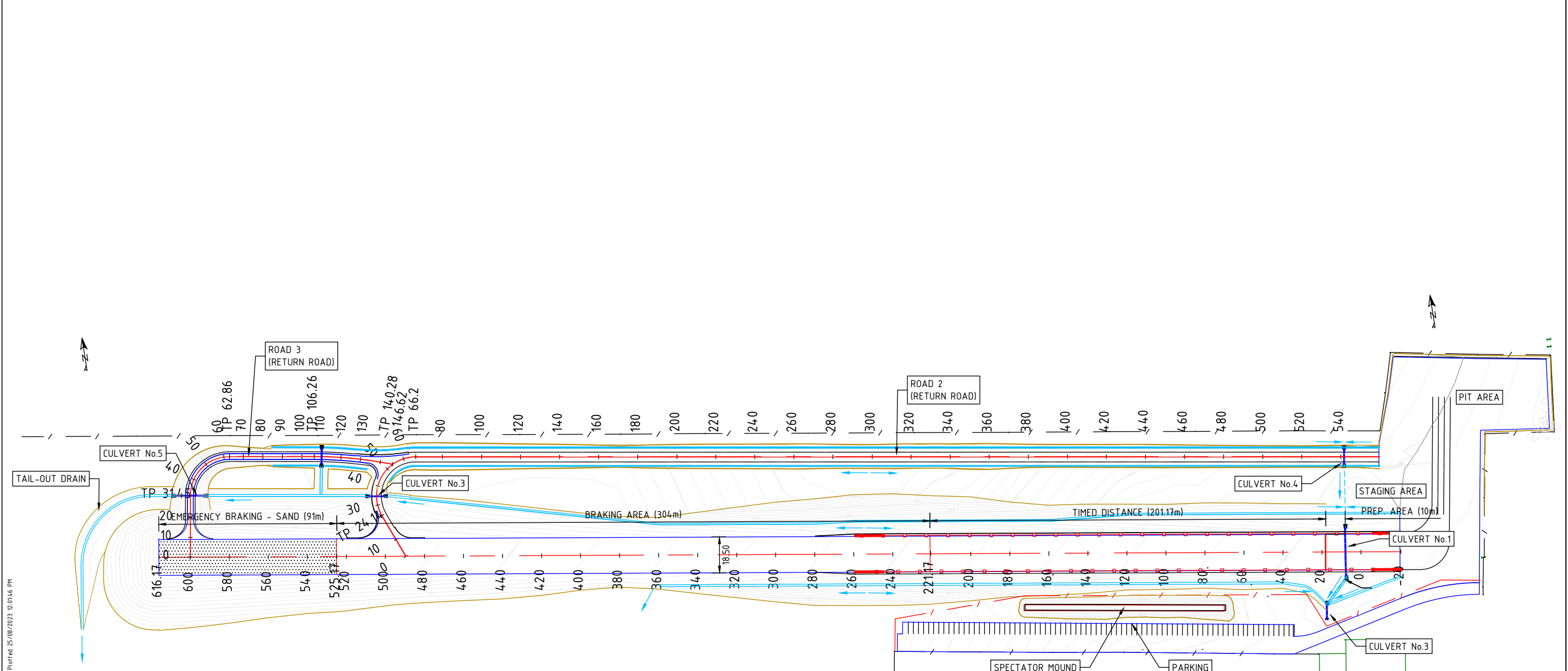
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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE:  TYPICAL CROSS SECTIONS	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					
DATUM: AHD								
PROJECTION: GDA2020 (ZONE 55)								
SURVEYED BY:								



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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE:  GENERAL LAYOUT PLAN					
	C	FOR COUNCIL APPROVAL	22-06-23						
	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						



EXISTING 450Ø RCP  
GRADE UNKNOWN (FALL STH)  
INLET INV. 259.705, OUTLET INV. 259.63  
EXTEND (CL4 RRJ) 7.32m AT 1% GRADE

GRADE BETWEEN  
CULVERTS 4%

450Ø RCP CL4 RRJ  
LENGTH 21.96m, GRADE 0.5% (FALL STH)  
INLET INV. 259.97, OUTLET INV. 259.85  
NEW CULVERT

GUARD RAIL TERMINAL (TYPICAL)


Class 4 RRJ450 Inv 259.905

0.500%

H GEOM	
EXIST. CL	

$$\frac{L=640.00}{B=278^{\circ}44'17''}$$

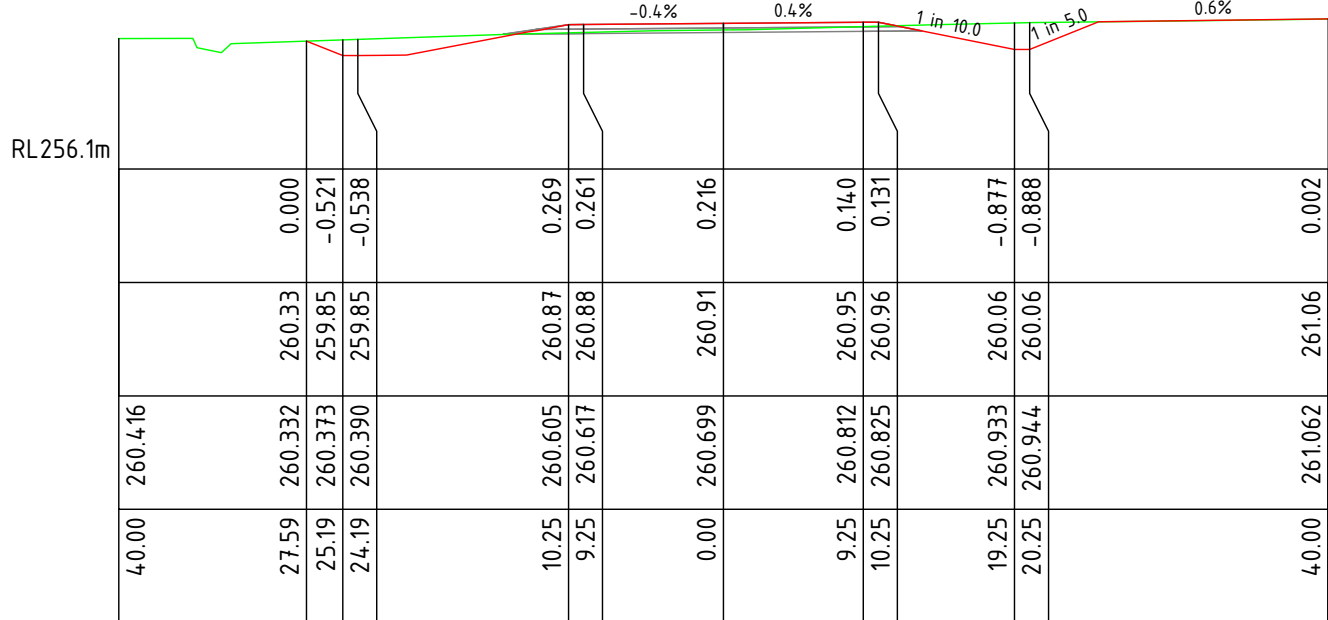
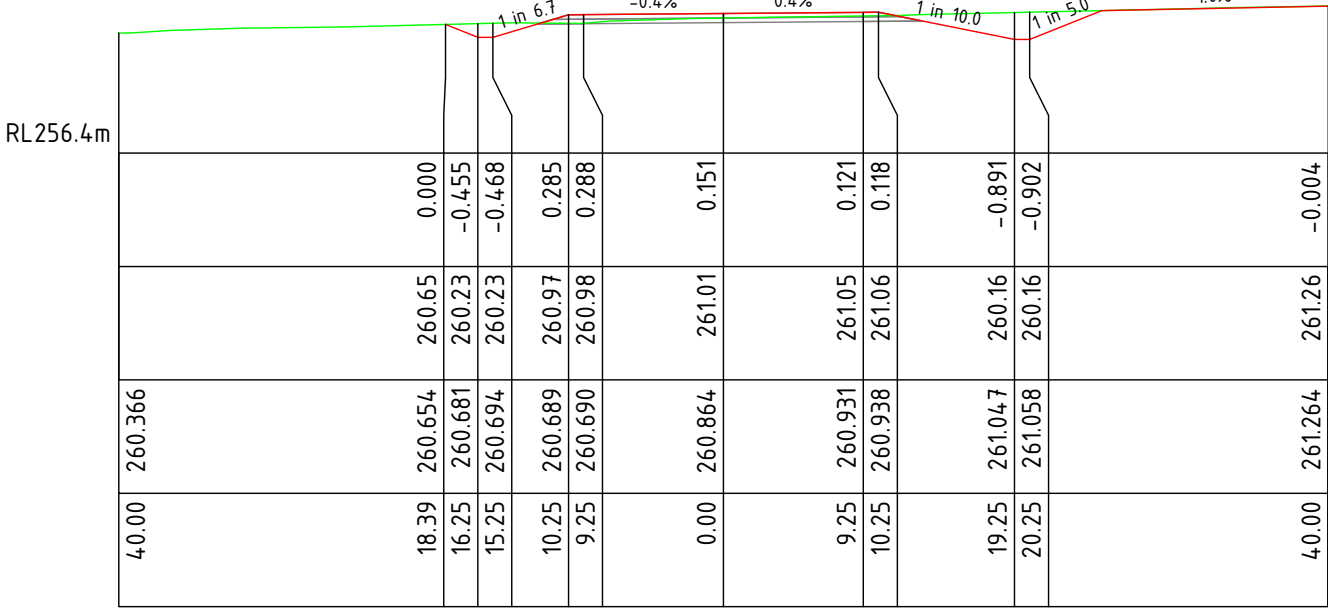
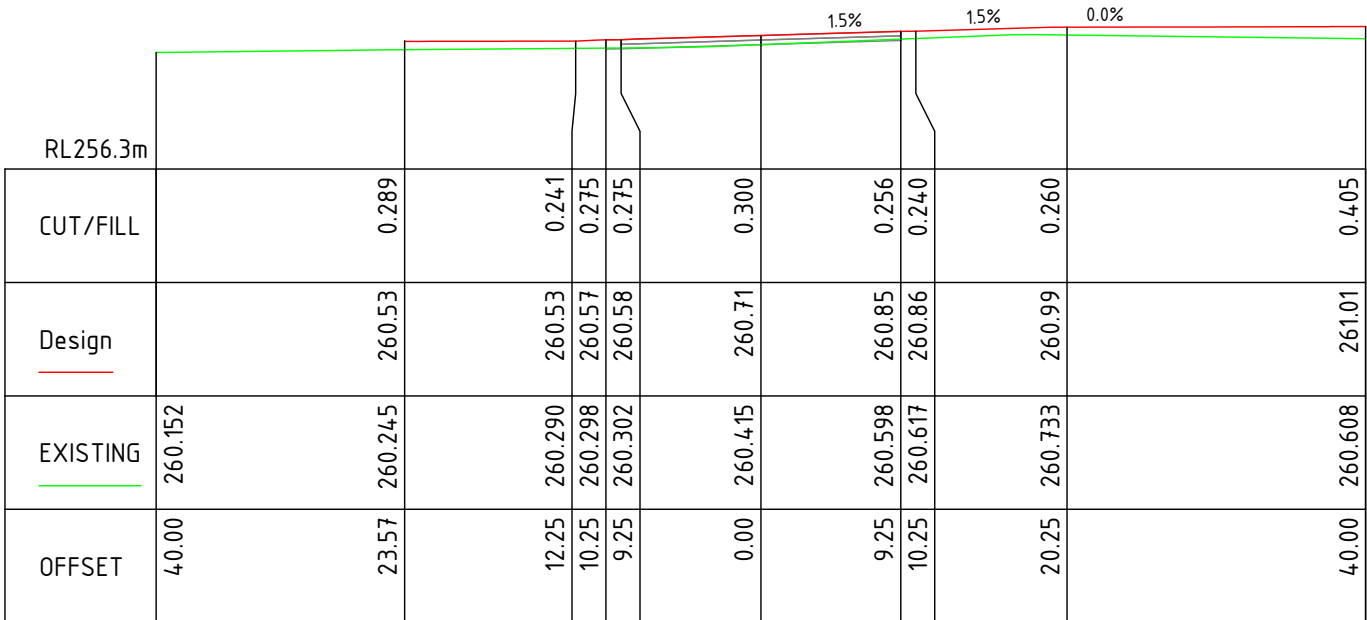
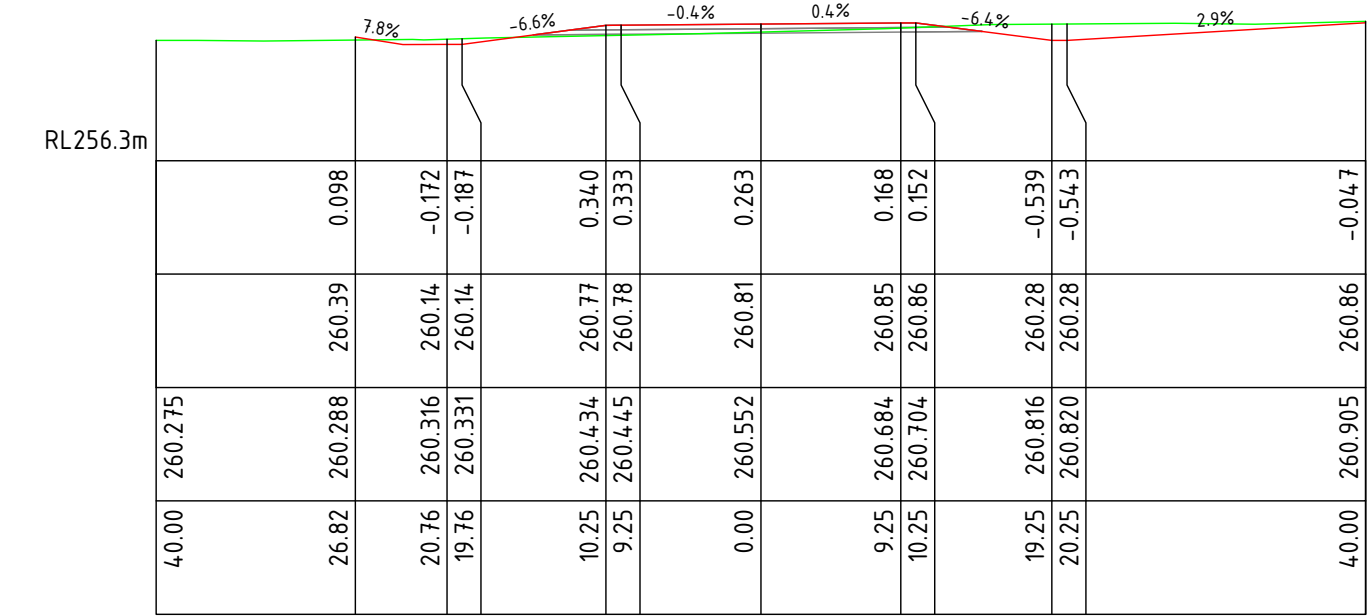
CHAINAGE	EXISTING CENTRELINE	DESIGN CENTRELINE	CUT / FILL
0.00	260.552	260.815	+0.263
20.00	260.699	260.915	+0.216
40.00	260.864	261.015	+0.151
60.00	260.871	261.115	+0.244
80.00	260.938	261.215	+0.277
100.00	261.012	261.315	+0.303
120.00	261.174	261.415	+0.241
140.00	261.399	261.515	+0.116
160.00	261.966	261.615	-0.351
180.00	262.591	261.715	-0.876
200.00	263.068	261.815	-1.253
220.00	263.577	261.915	-1.662
240.00	263.735	262.015	-1.720
260.00	263.945	262.115	-1.830
280.00	263.851	262.215	-1.636
300.00	263.602	262.315	-1.287
320.00	263.253	262.415	-0.838
340.00	262.874	262.515	-0.359


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DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: <b>DRAG STRIP PLAN &amp; LONG SECTION (-20 - 340)</b>	
PROJECTION:	GDA2020 (ZONE 55)	C	FOR COUNCIL APPROVAL	22-06-23					
SURVEYED BY:		B	FOR REVIEW	19-06-23					
		A	DRAFT - FOR COUNCIL COMMENT	17-04-23					





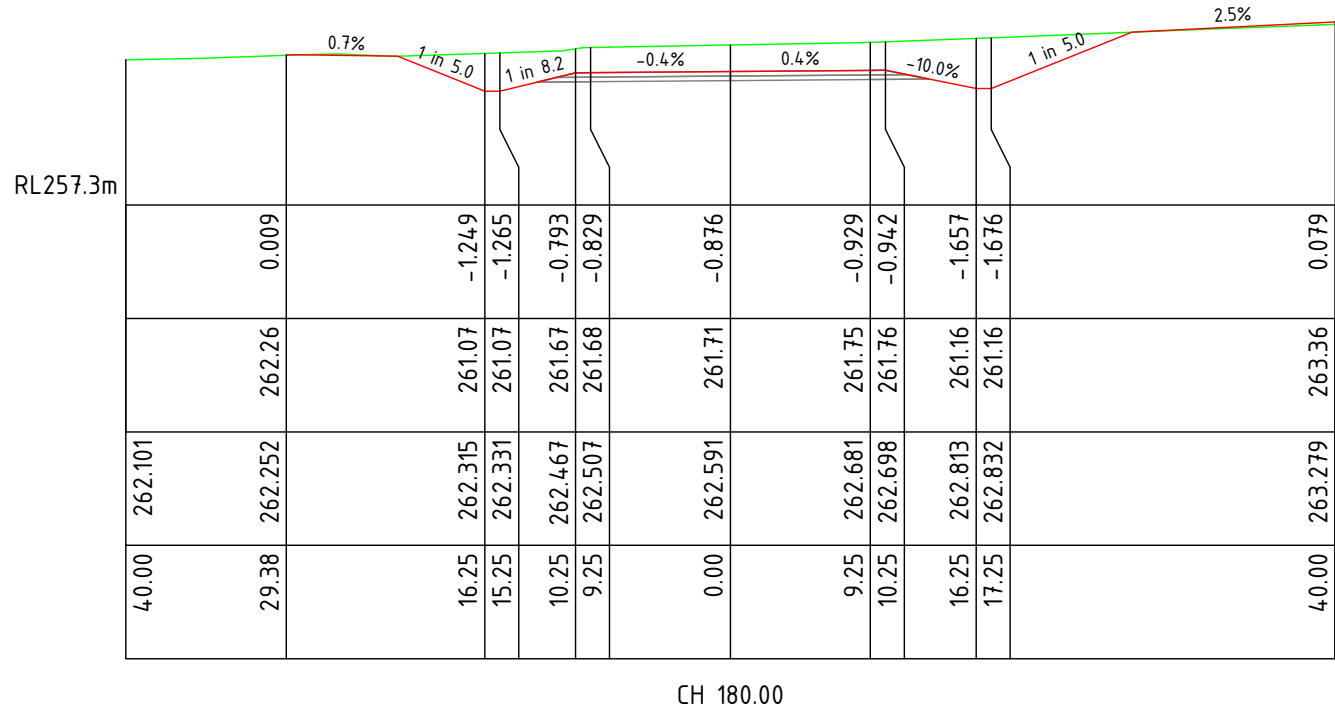
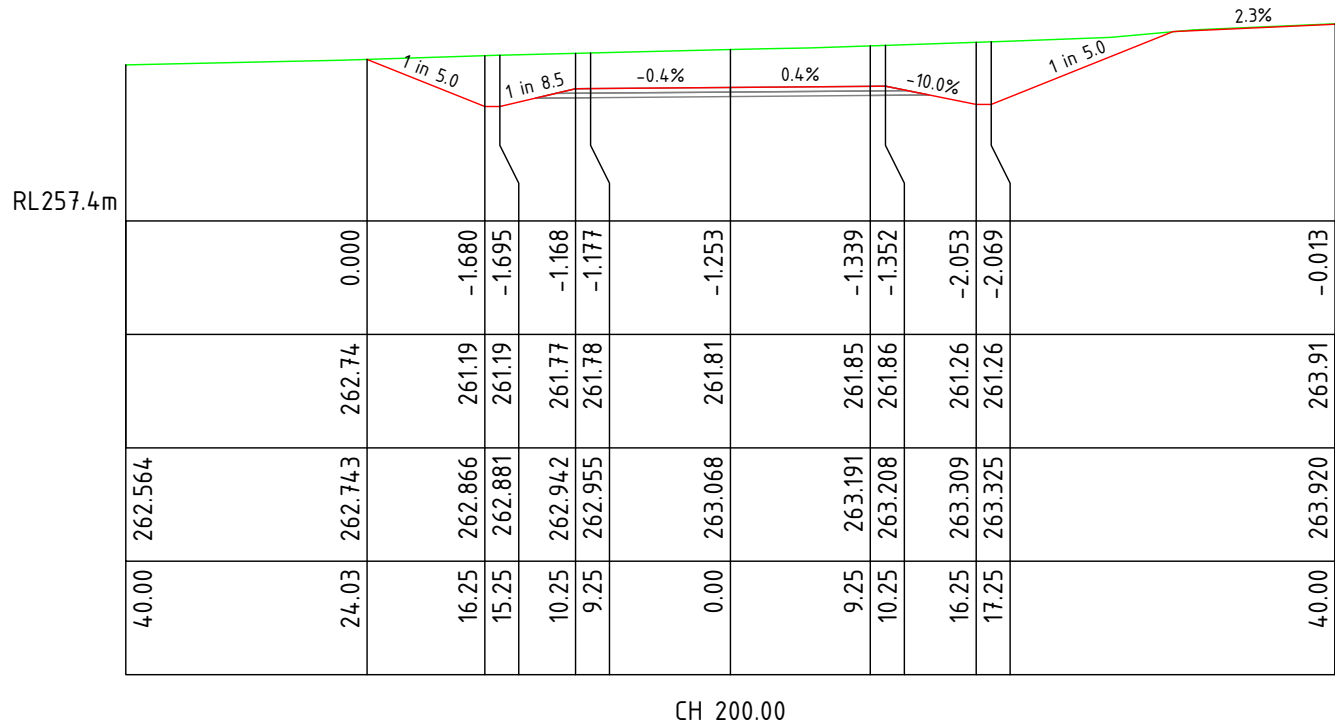
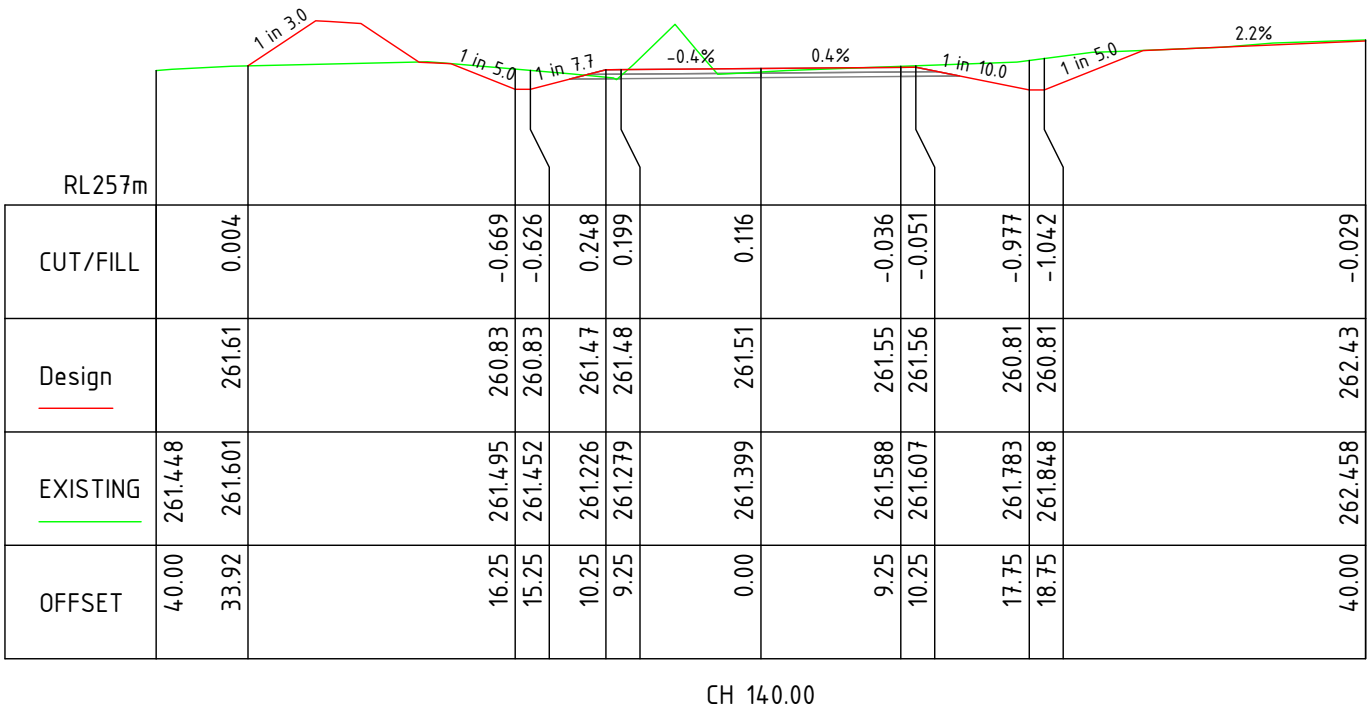
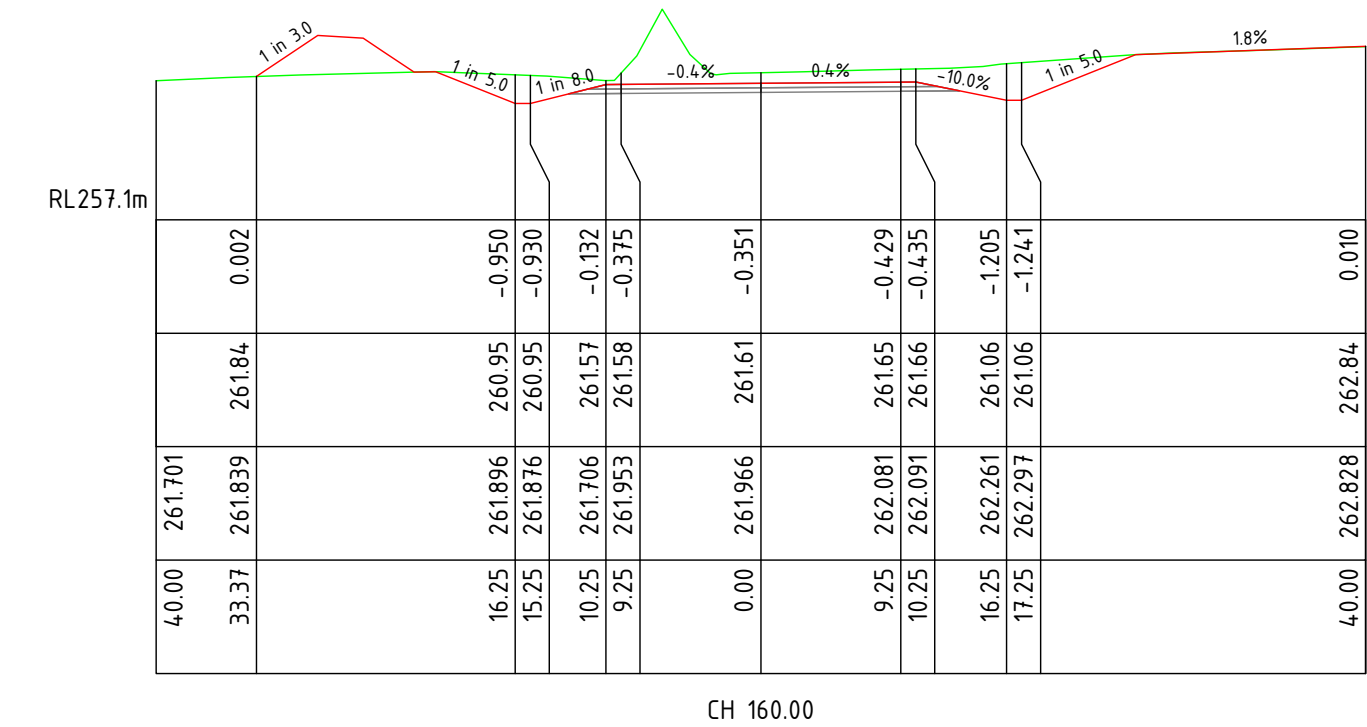
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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: DRAG STRIP CROSS SECTIONS (-20 - 40)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

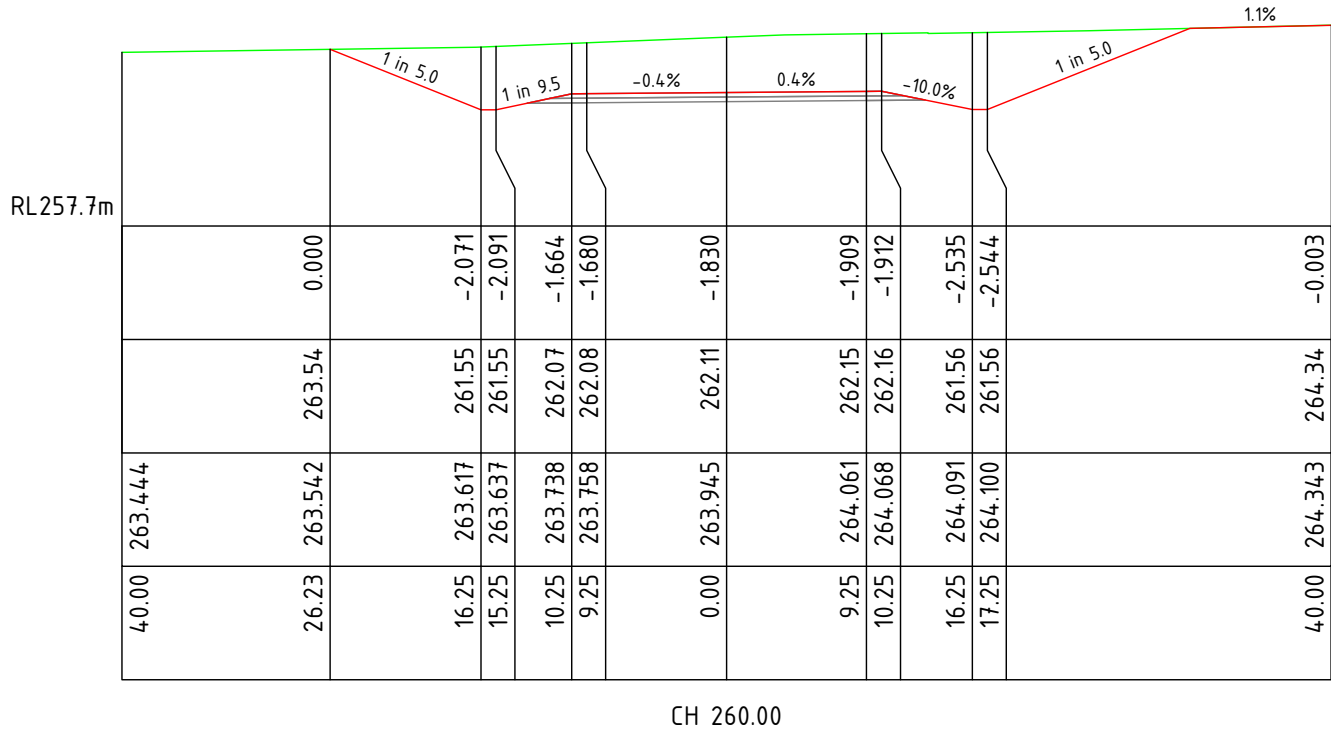
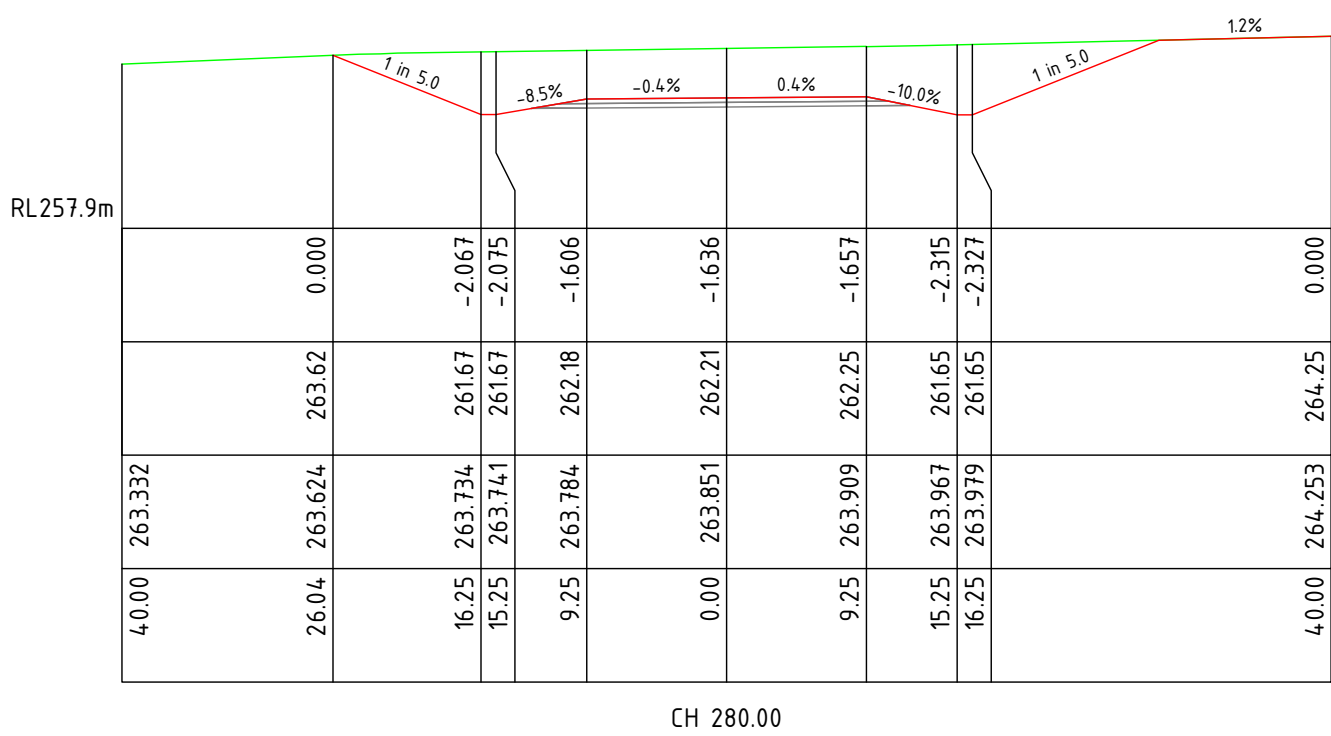
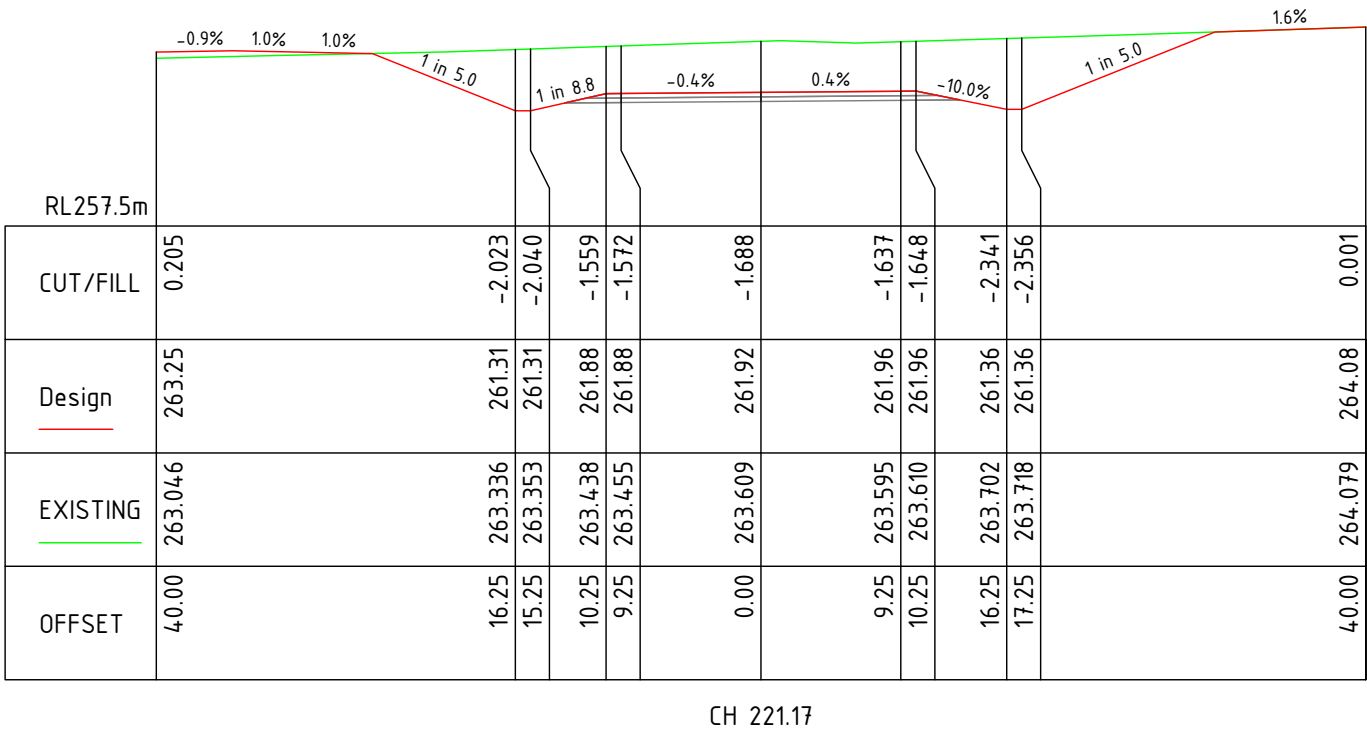
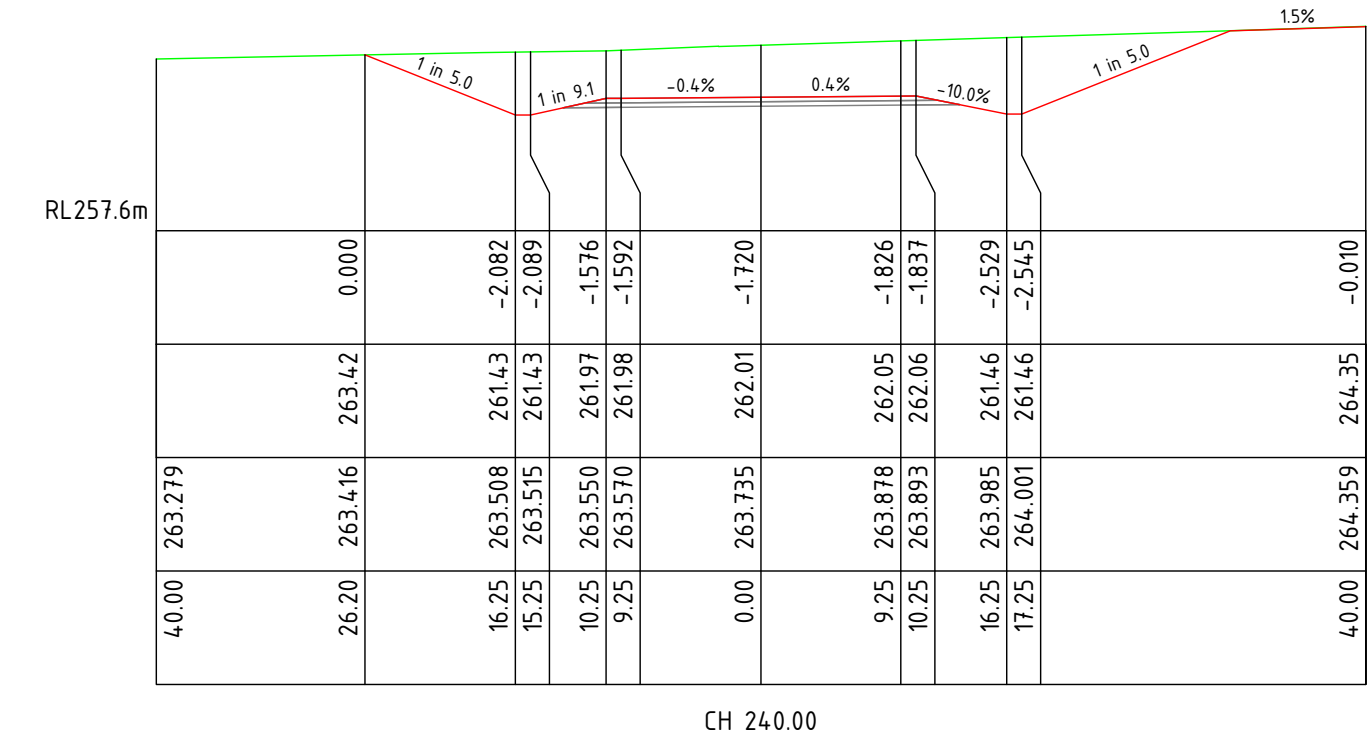



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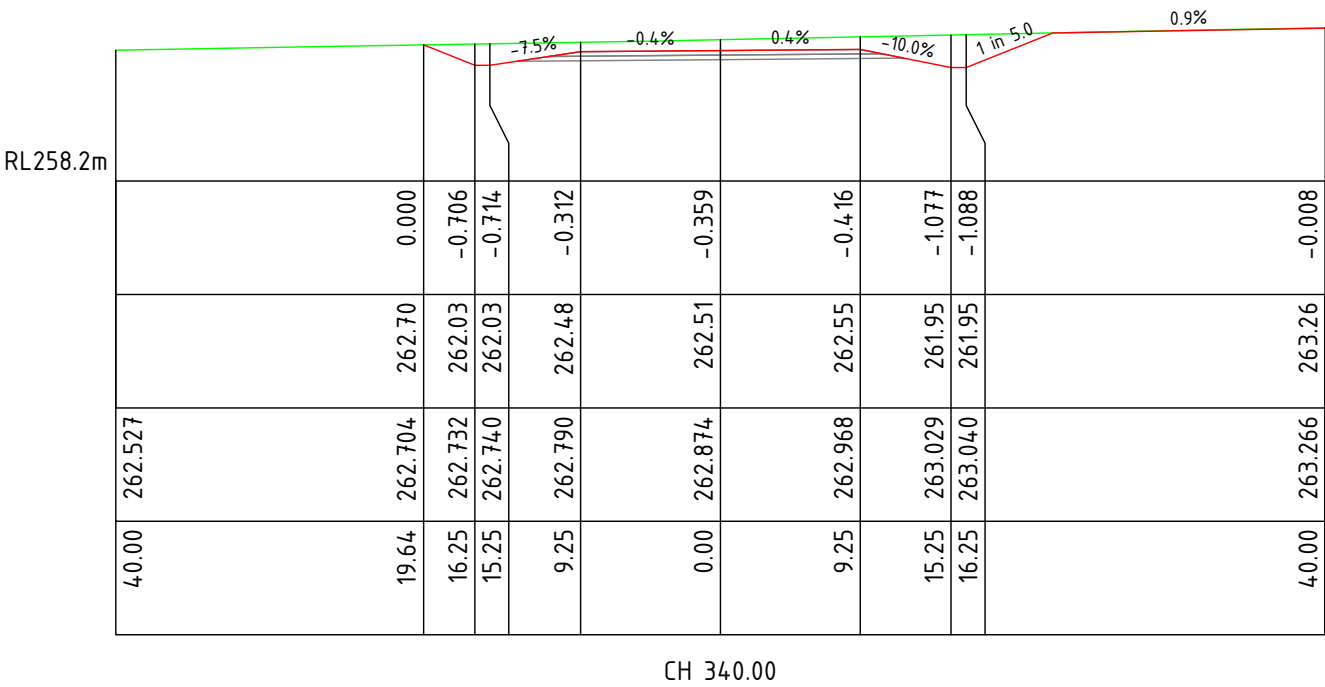
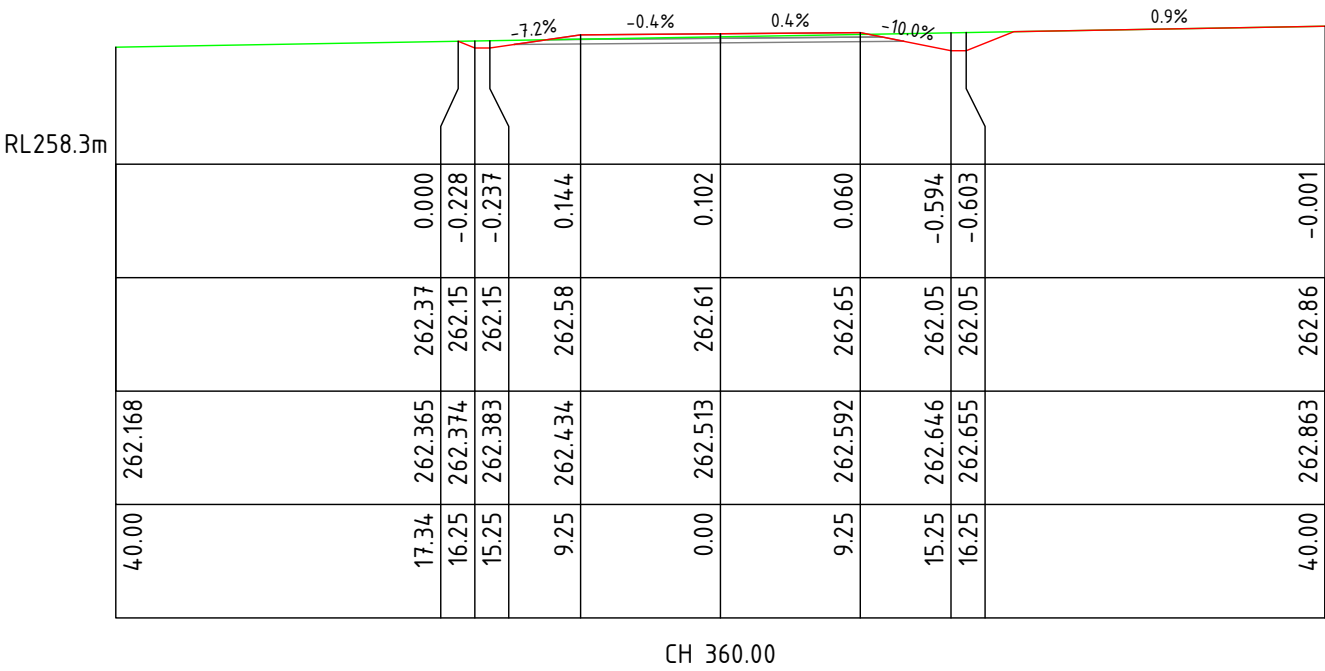
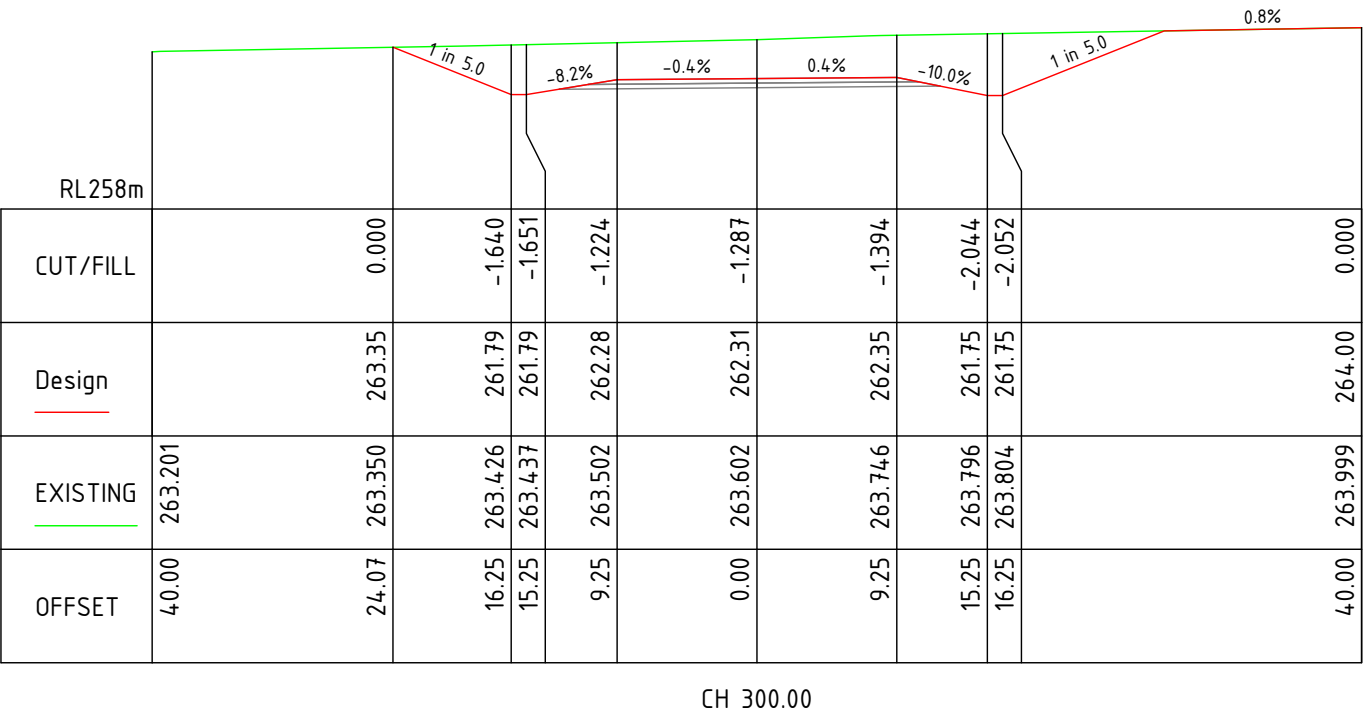
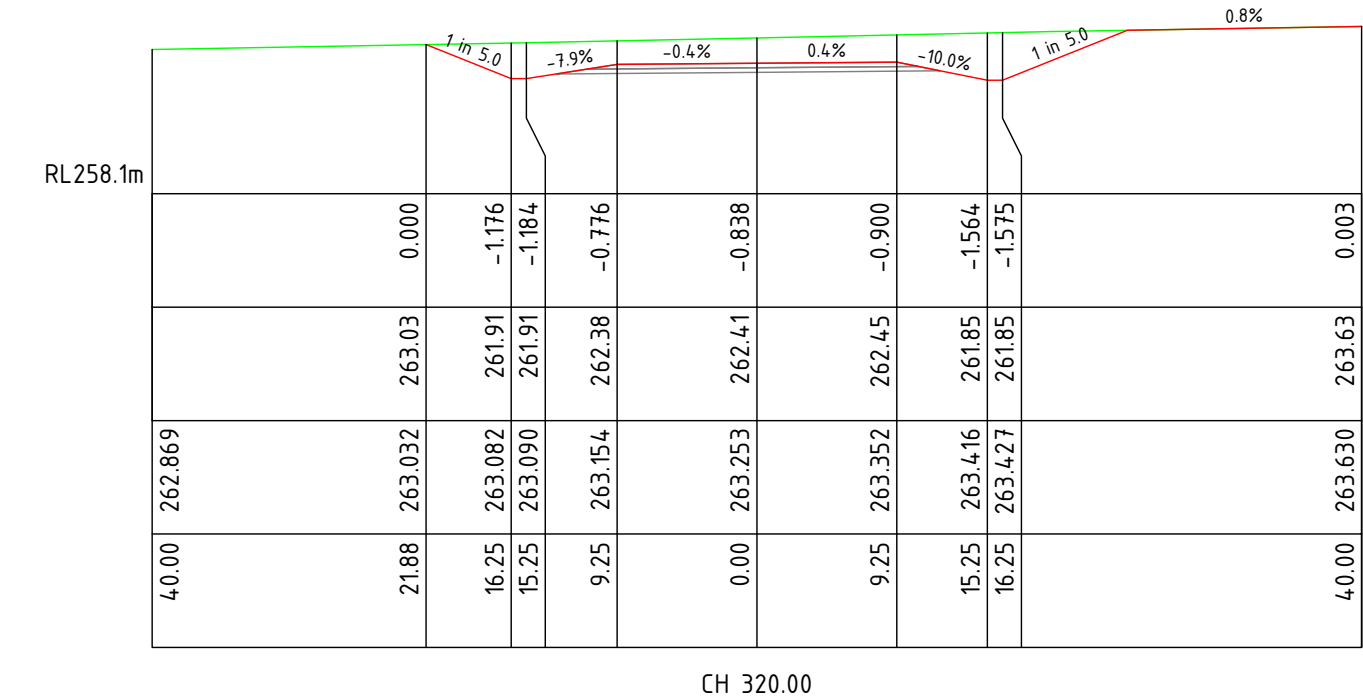
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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: DRAG STRIP CROSS SECTIONS (140 - 200)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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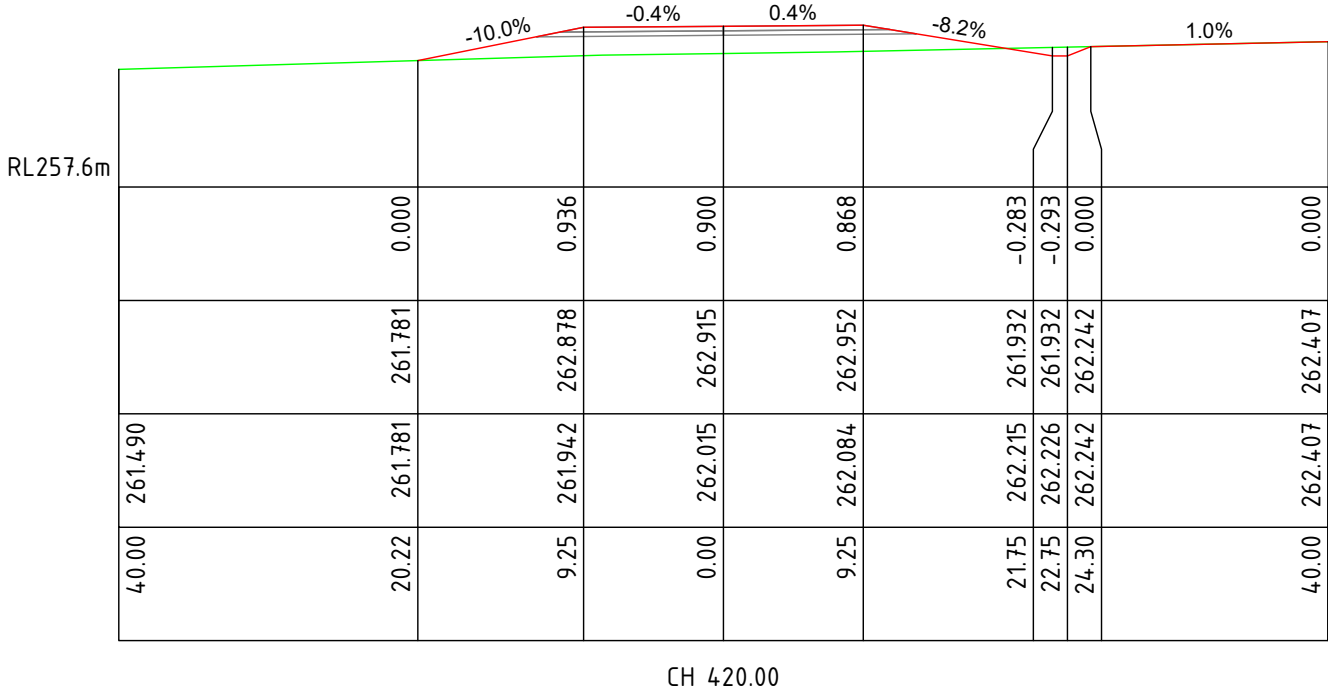
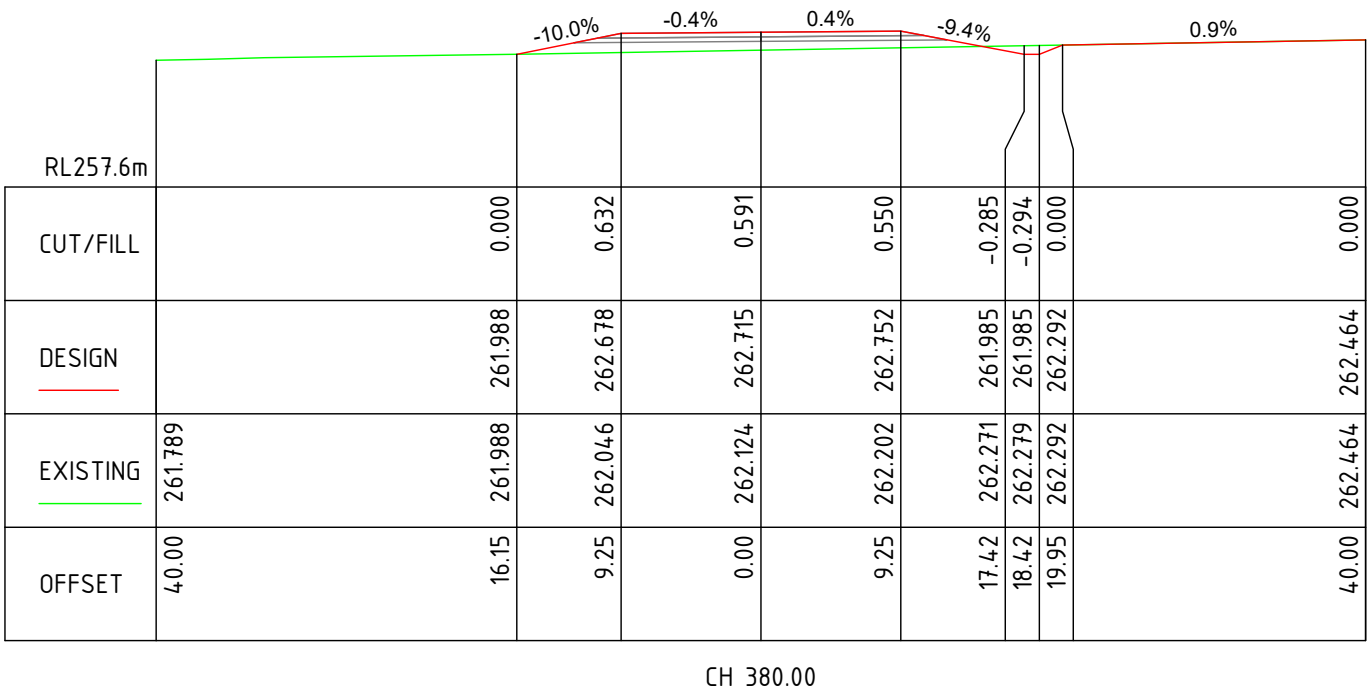
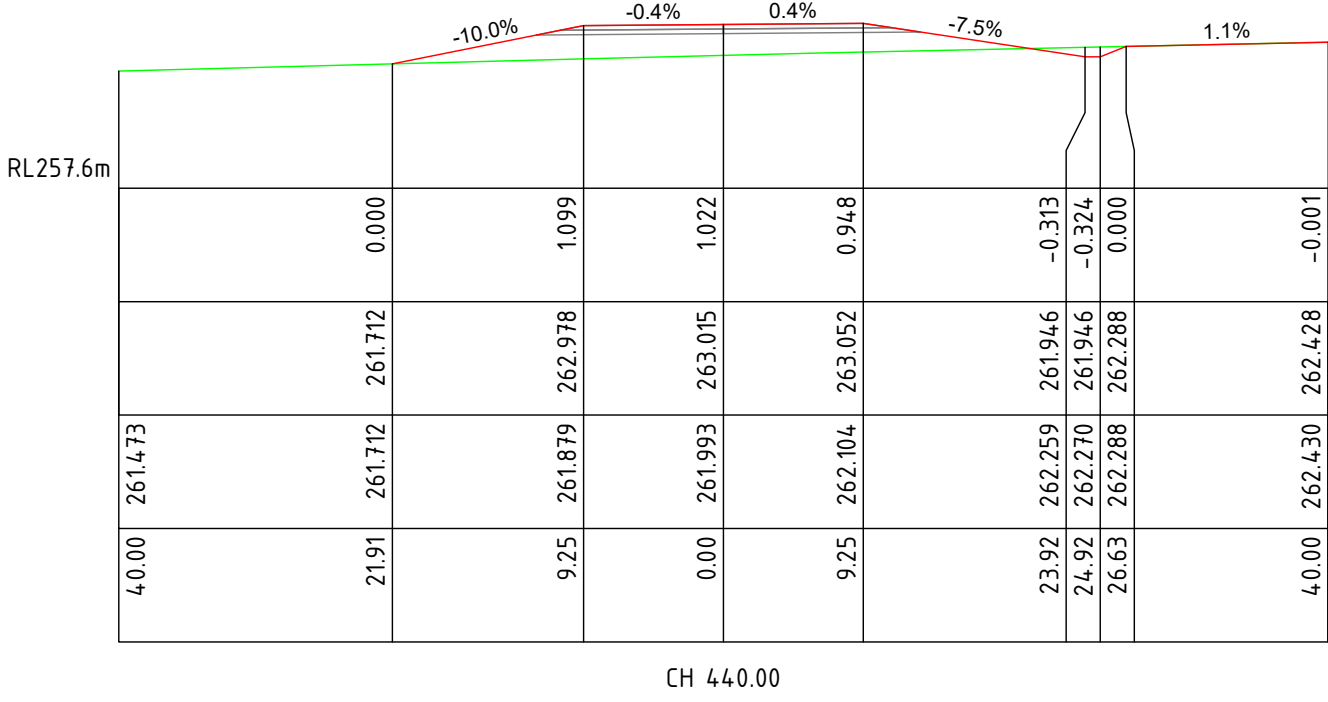
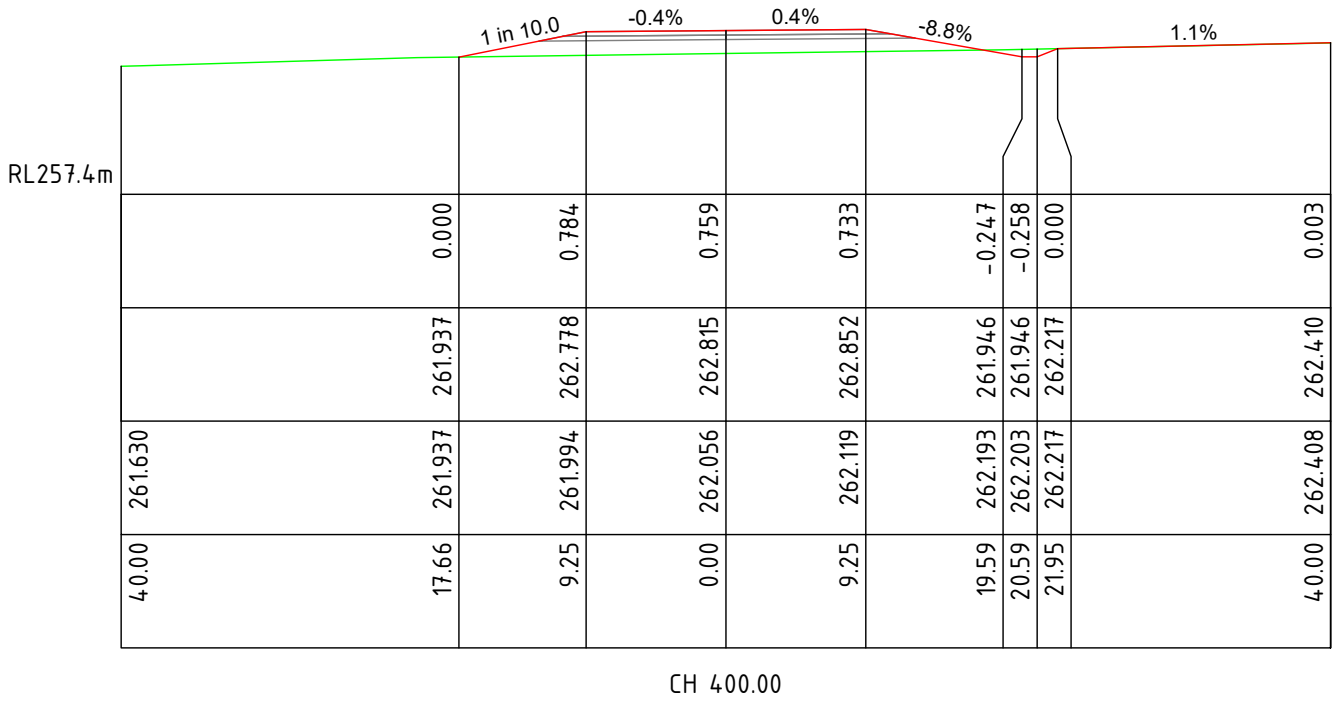


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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: DRAG STRIP CROSS SECTIONS (221.17 - 280)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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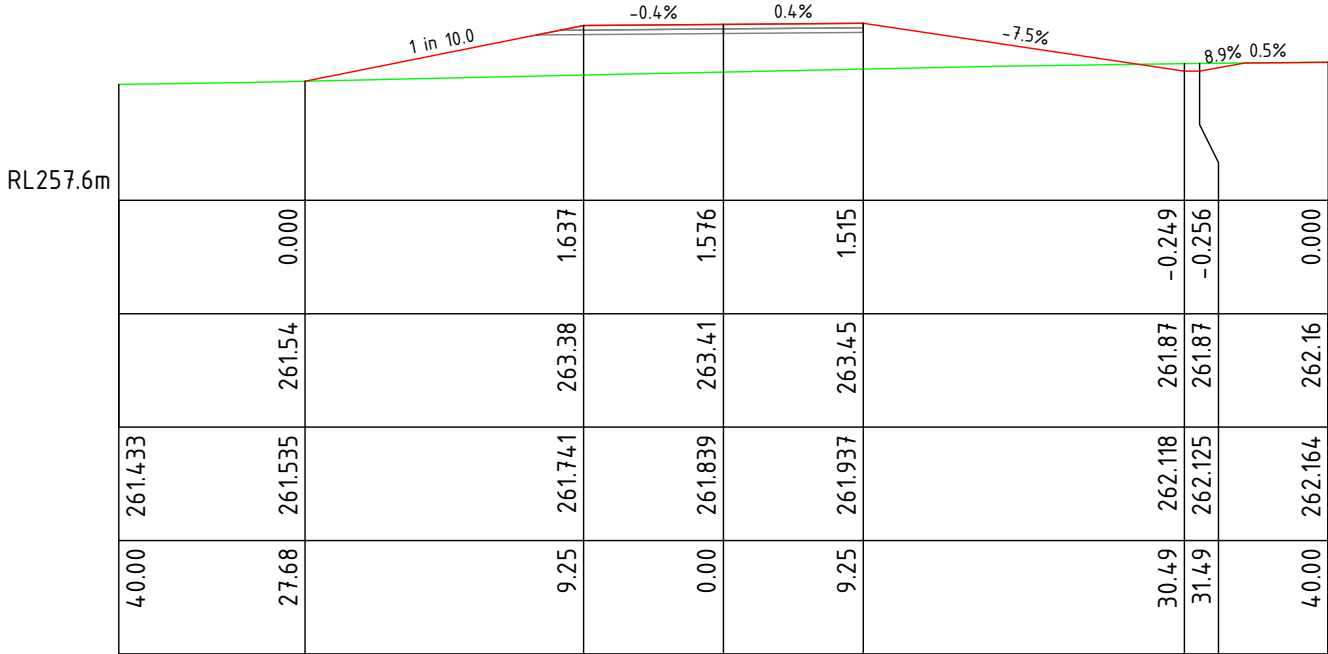




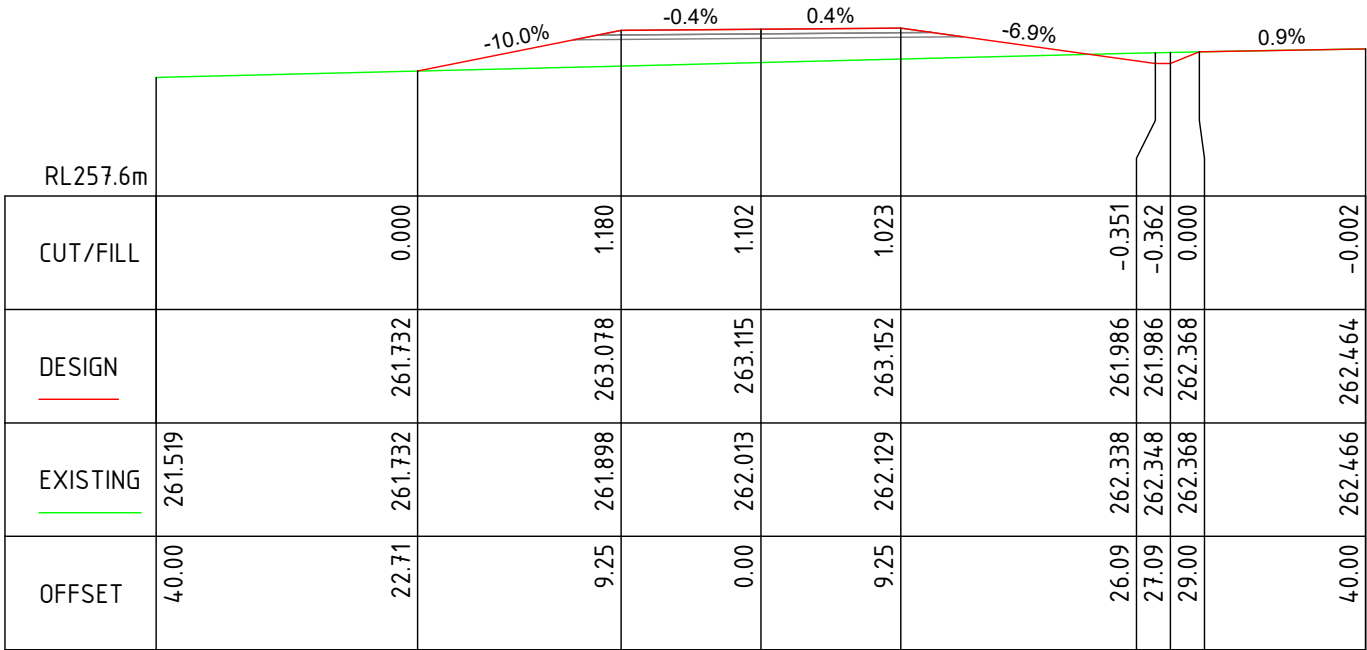
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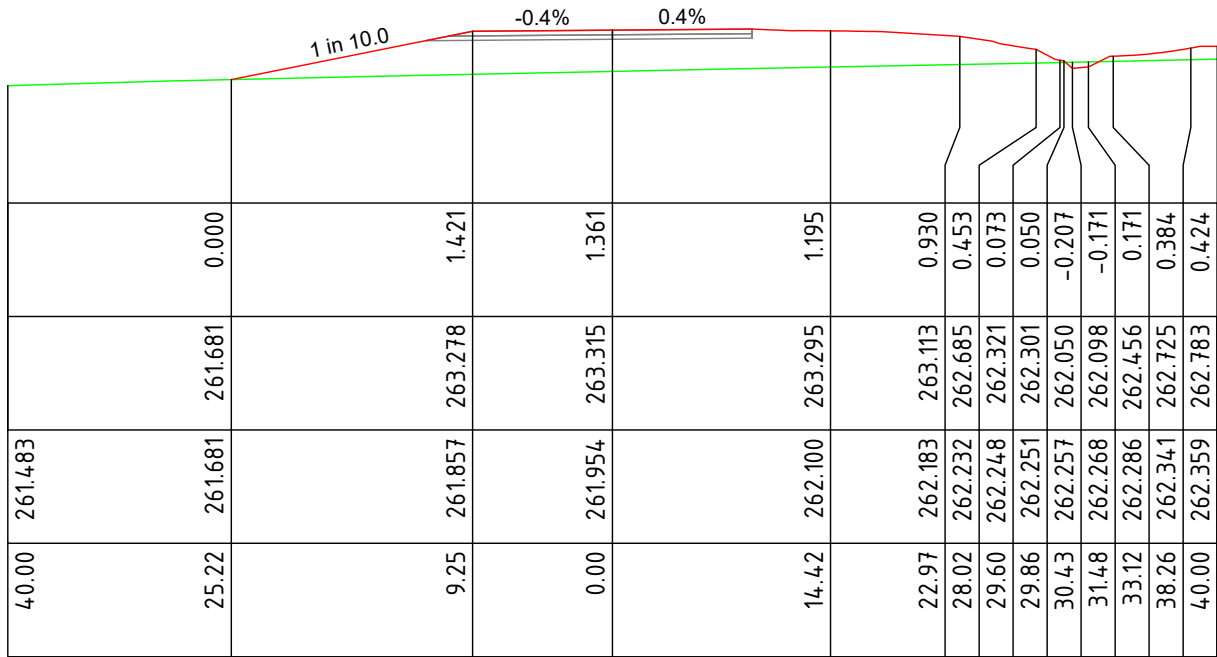
CH 480.00




CH 520.00



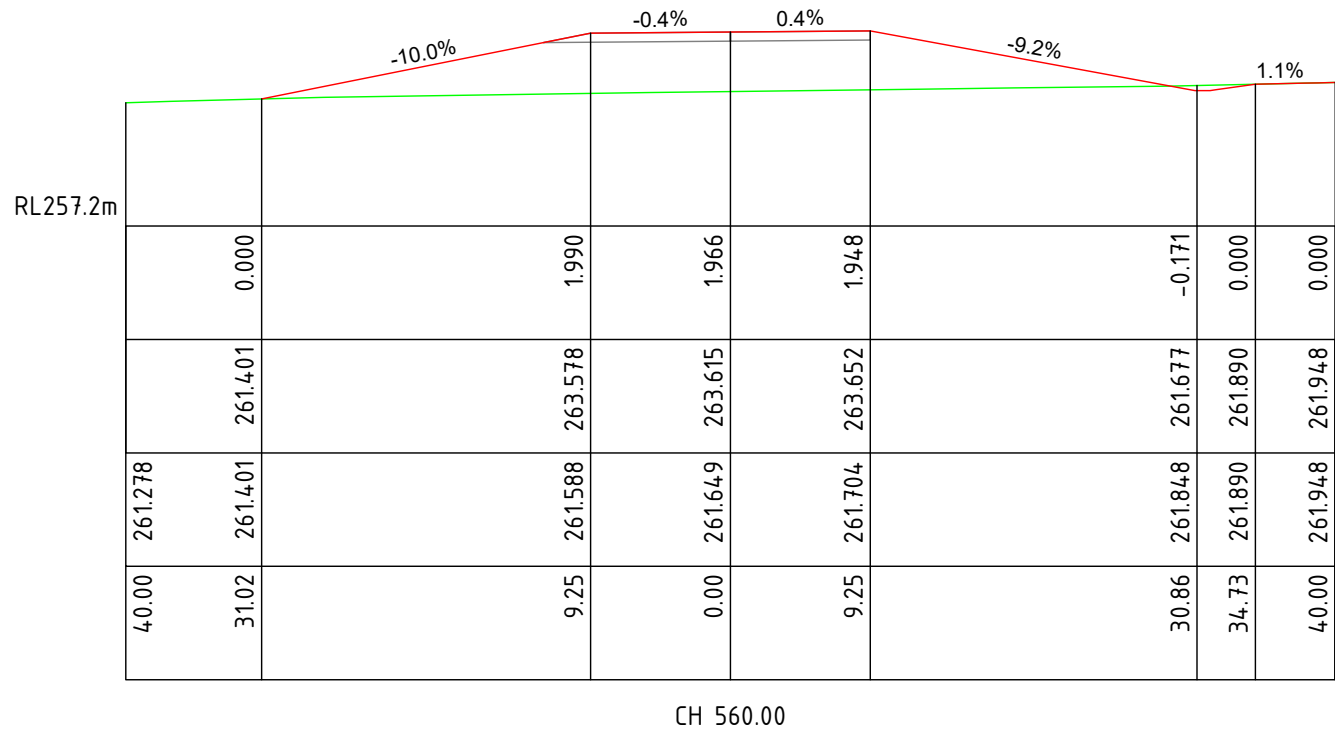
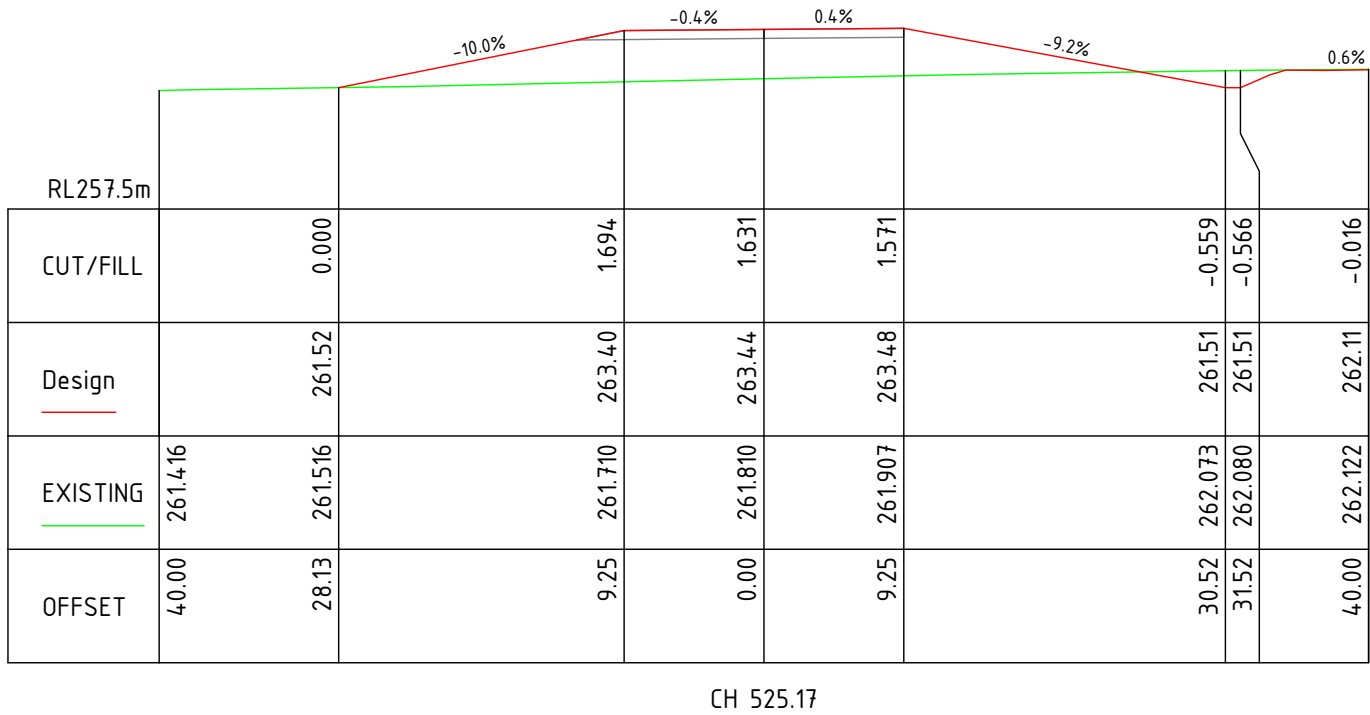
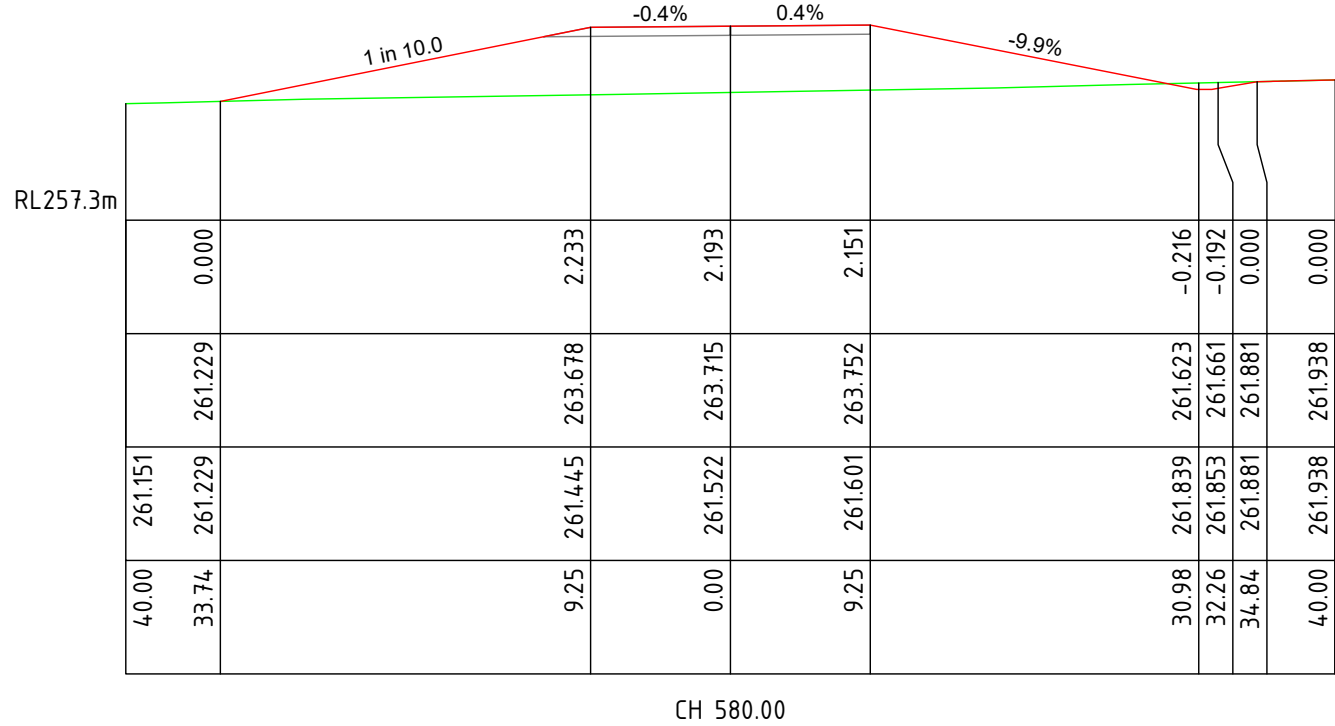
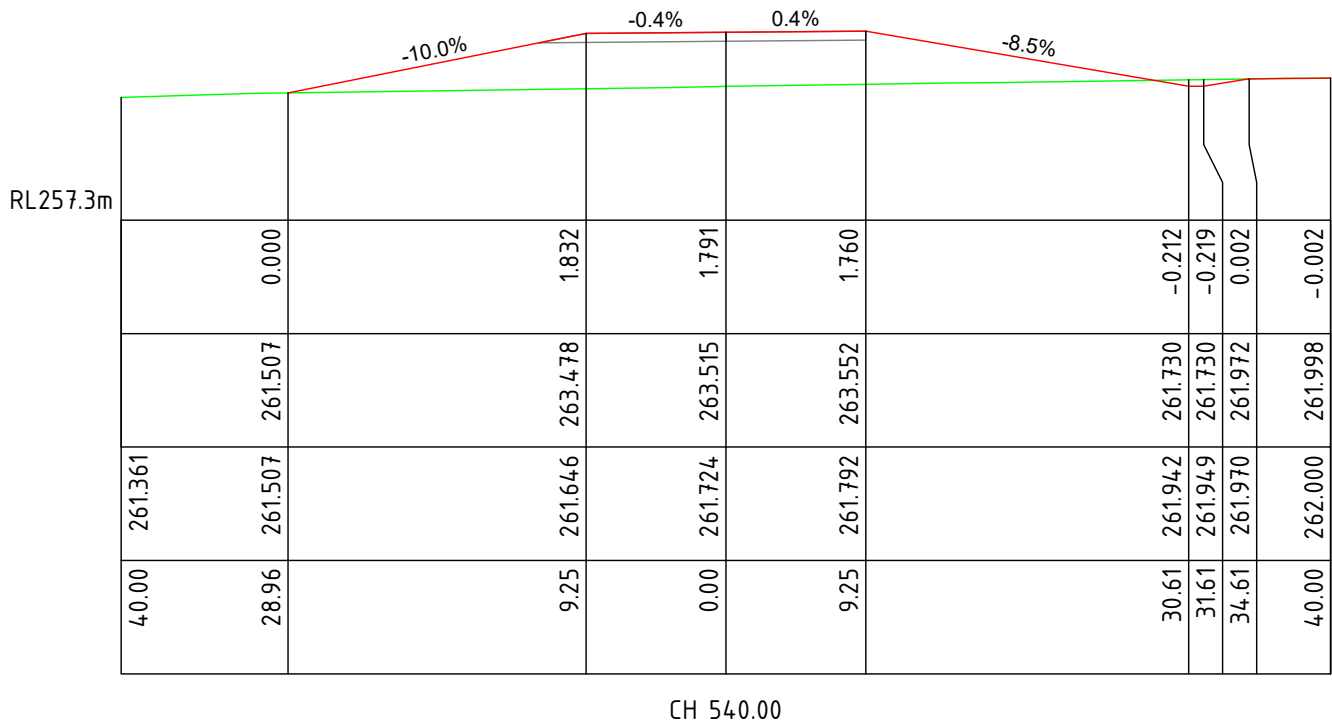
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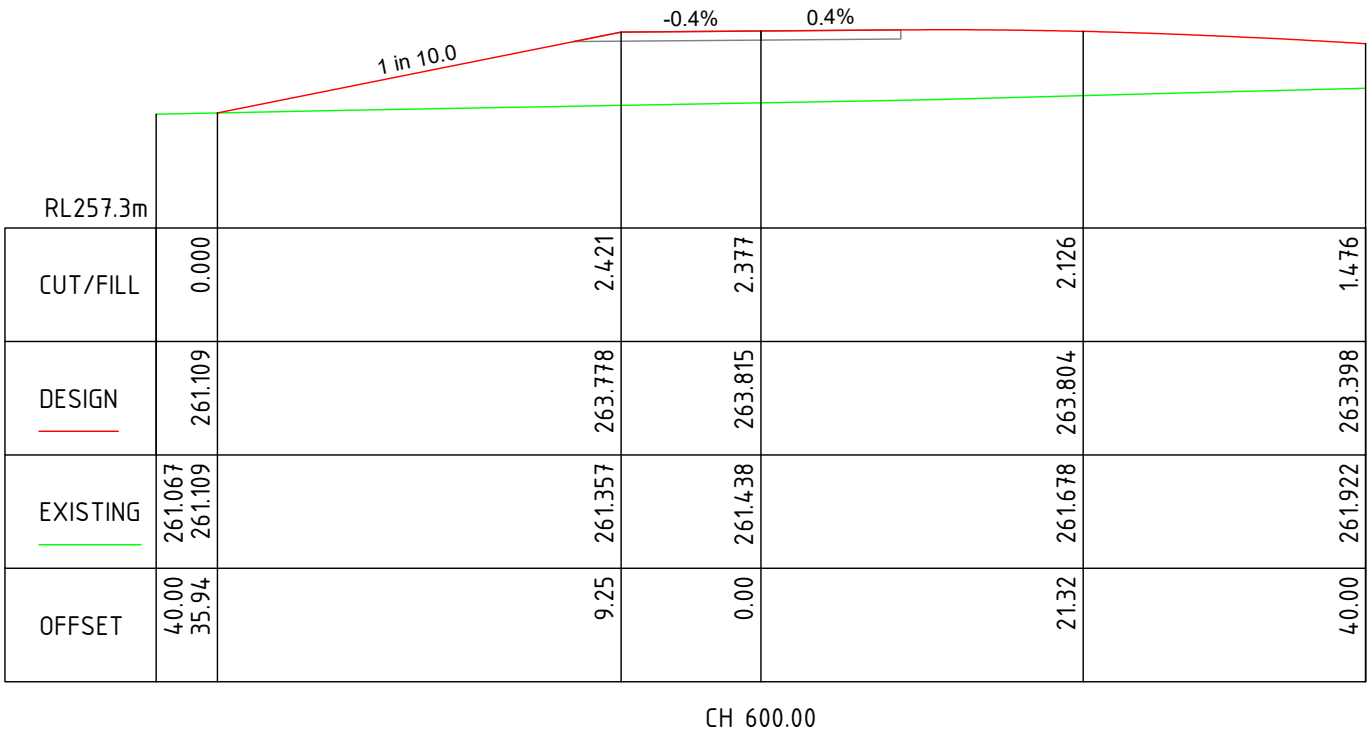
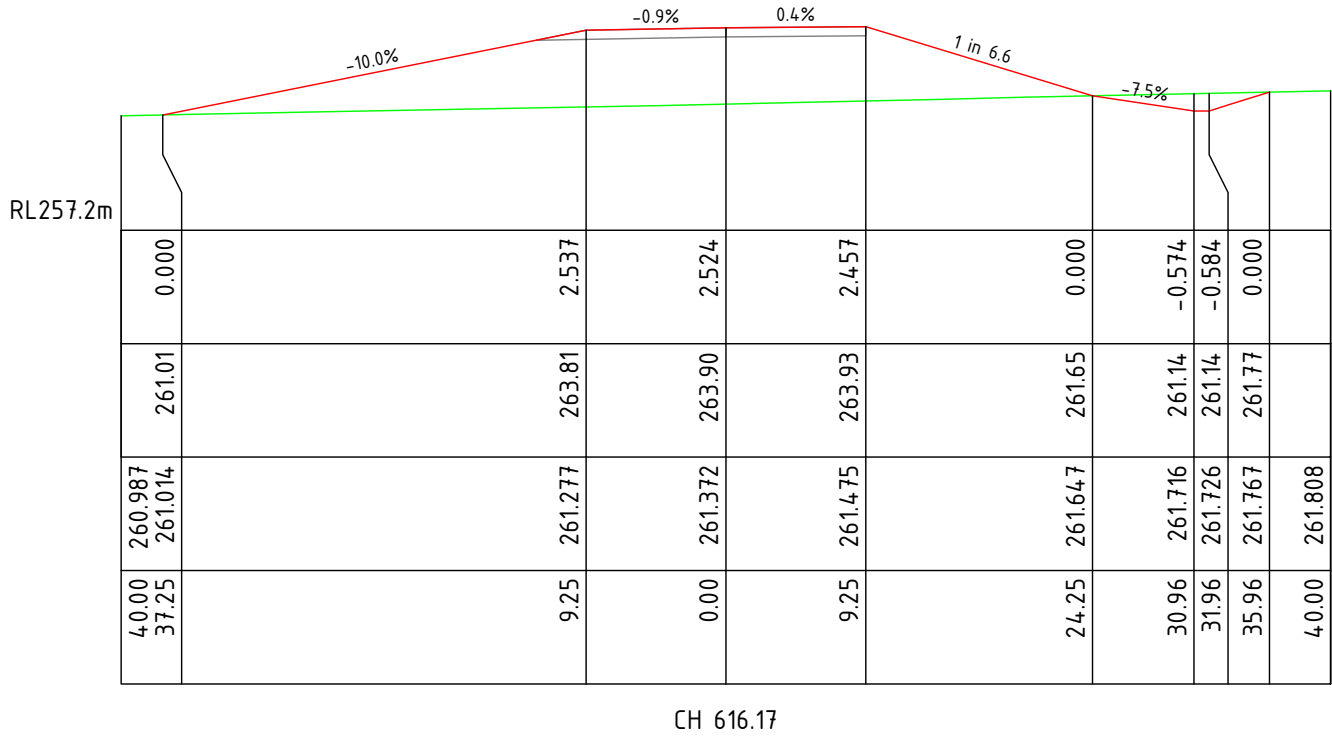
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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: DRAG STRIP CROSS SECTIONS (460 - 520)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					


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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: DRAG STRIP CROSS SECTIONS (525.17 - 580)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					


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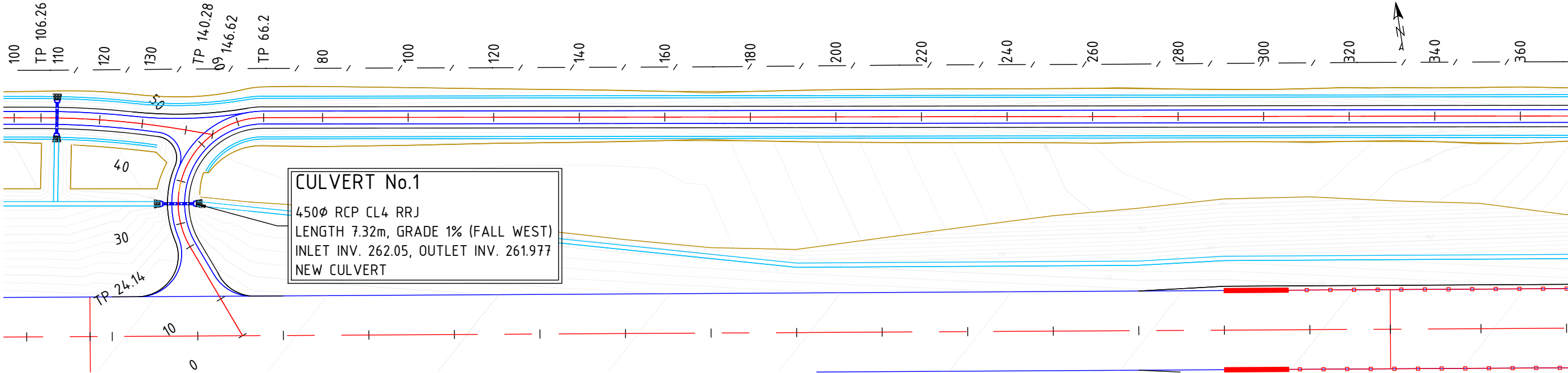
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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: DRAG STRIP CROSS SECTIONS (600 - 616.17)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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SCALE: 1:1000H 1:100V		REV.	DESCRIPTION	DATE	 <div>REGIONAL engineering SERVICES</div> <div>REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404</div>	CLIENT:  BLAND SHIRE COUNCIL	PROJECT:  WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 15	SHEETS 35
DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE: ROAD 2 PLAN & LONG SECTION (0 - 360)					
	C	FOR COUNCIL APPROVAL	22-06-23						
	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						

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	262.000	262.069	262.086	262.101	262.102	262.129	262.134	262.156	262.177	262.200	262.246	262.301	262.325	262.401	262.440	262.502	262.519	262.534	262.561	262.559		262.552		262.519		262.521		262.532		262.543		262.742		263.145		263.535		263.892		264.072		264.257		264.410		264.437	264.440	264.440		264.379		264.187		264.026		263.856																																		
DESIGN CENTRELINE	263.263	263.327	263.334	263.328	263.327	263.280	263.268	263.208	263.155	263.104	263.021	262.927	262.881	262.776	262.737	262.677	262.658	262.642	262.625	262.634		262.694		262.754		262.814		262.923		263.015		263.131		263.437		263.792		264.148		264.325		264.484		264.688		264.742	264.741	264.741		264.643		264.393		264.213		264.012																																		
CUT / FILL	+1.263	+1.258	+1.248	+1.227	+1.225	+1.151	+1.134	+1.052	+0.978	+0.904	+0.775	+0.626	+0.556	+0.375	+0.297	+0.175	+0.139	+0.108	+0.064	+0.075		+0.142		+0.235		+0.293		+0.391		+0.472		+0.389		+0.292		+0.257		+0.256		+0.253		+0.227		+0.278		+0.305	+0.301	+0.301		+0.264		+0.206		+0.187		+0.156																																		
H GEOM EXIST. CL	L=24.14 B=338°29'10"																														L=42.07 R=20.00																														L=493.75 B=98°59'44"																													
R.L. 249.9	<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div><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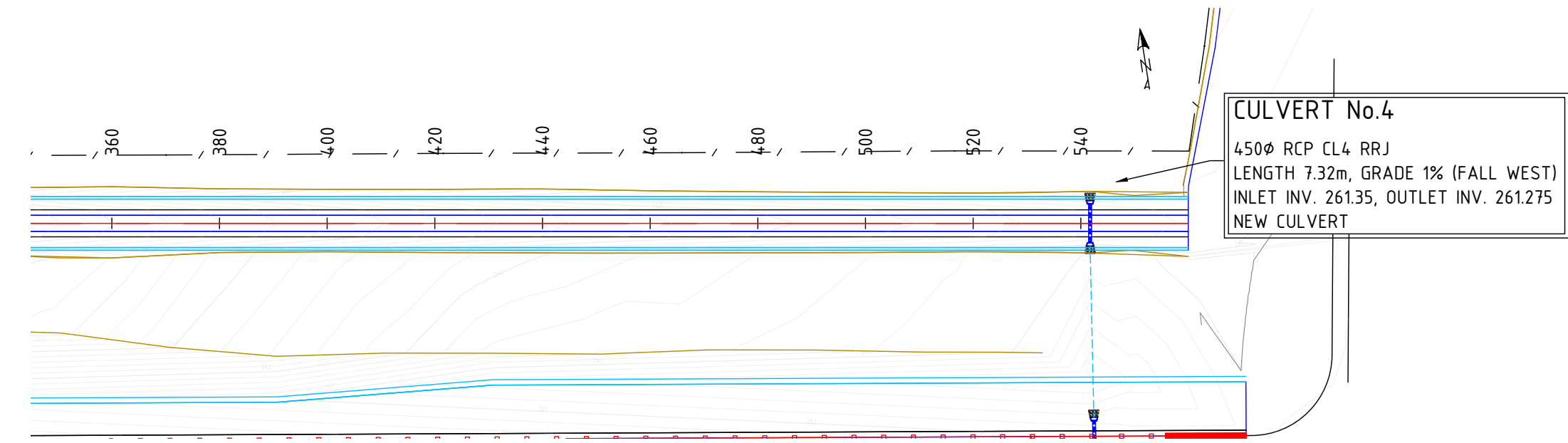
NOTE: INSTALL ROCK RIPRAP ON BOTH ENDS  
OF ALL CULVERTS TO DETAIL ON SHEET 9.

CULVERT No.1

450Ø RCP CL4 RRJ  
LENGTH 7.32m, GRADE 1% (FALL WEST)  
INLET INV. 262.05, OUTLET INV. 261.977  
NEW CULVERT

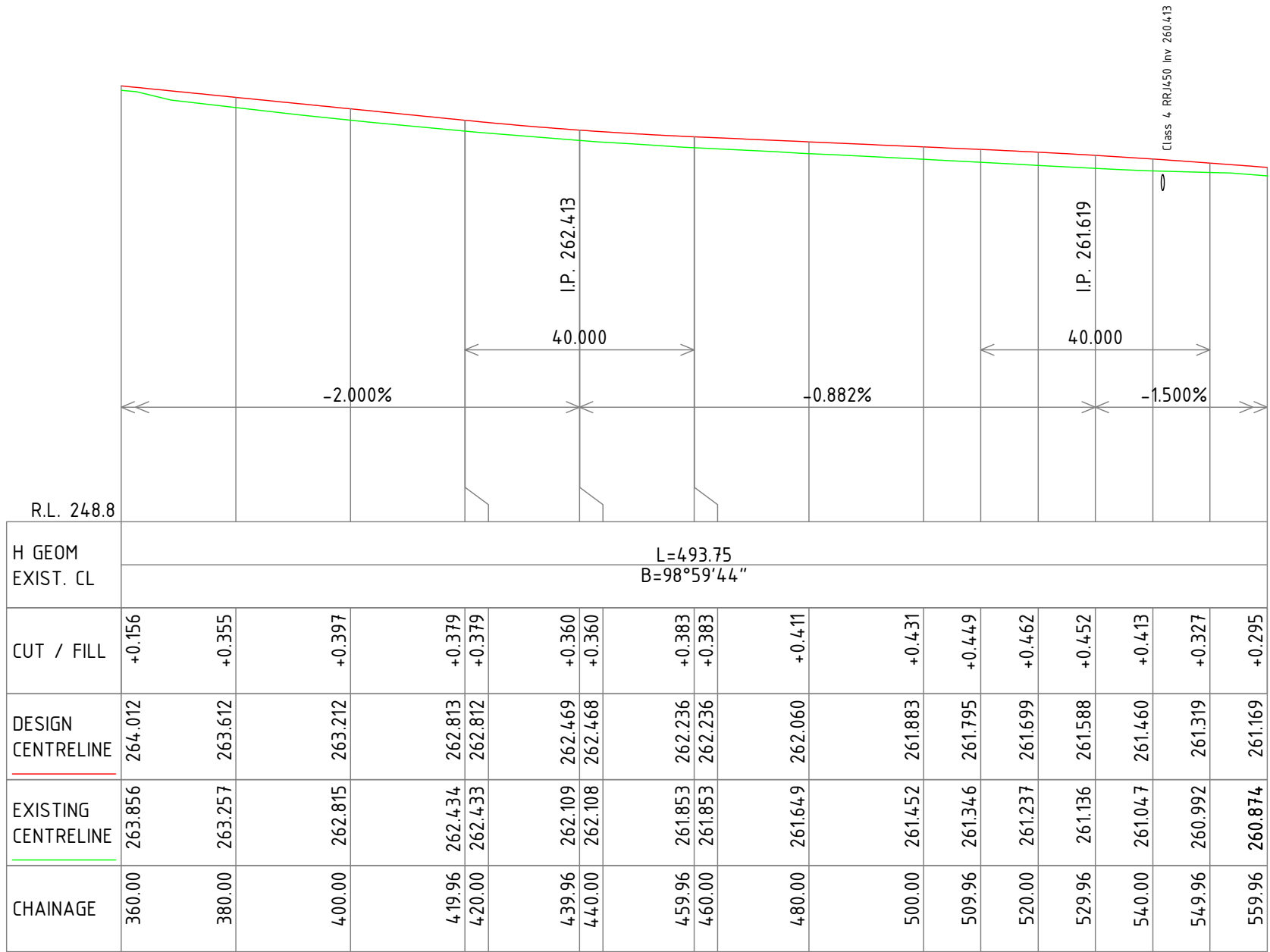
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CULVERT No.4

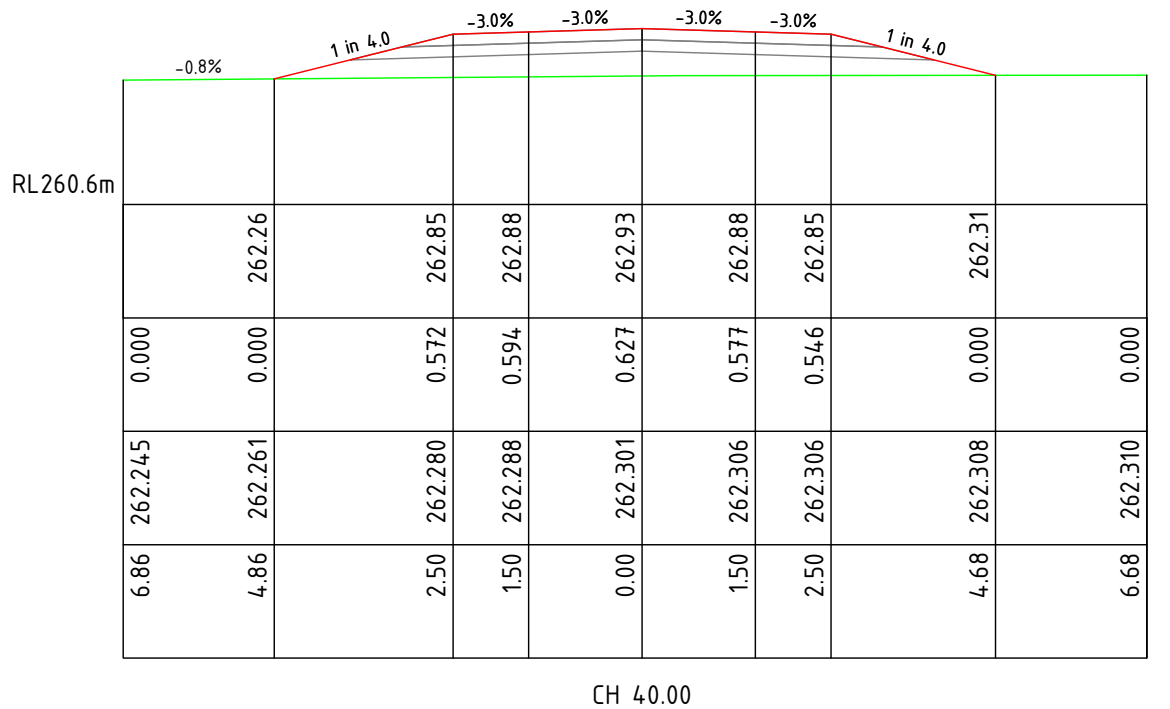
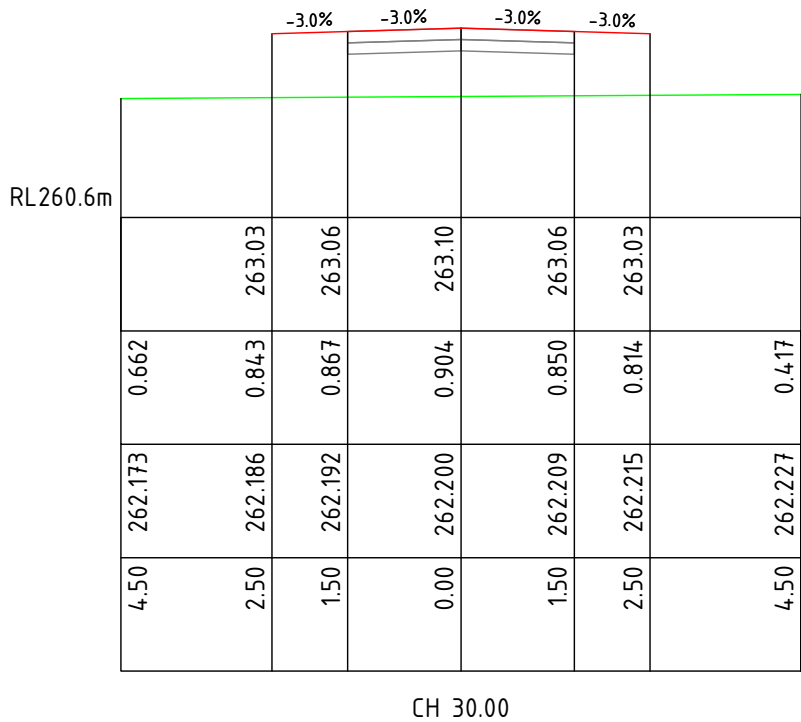
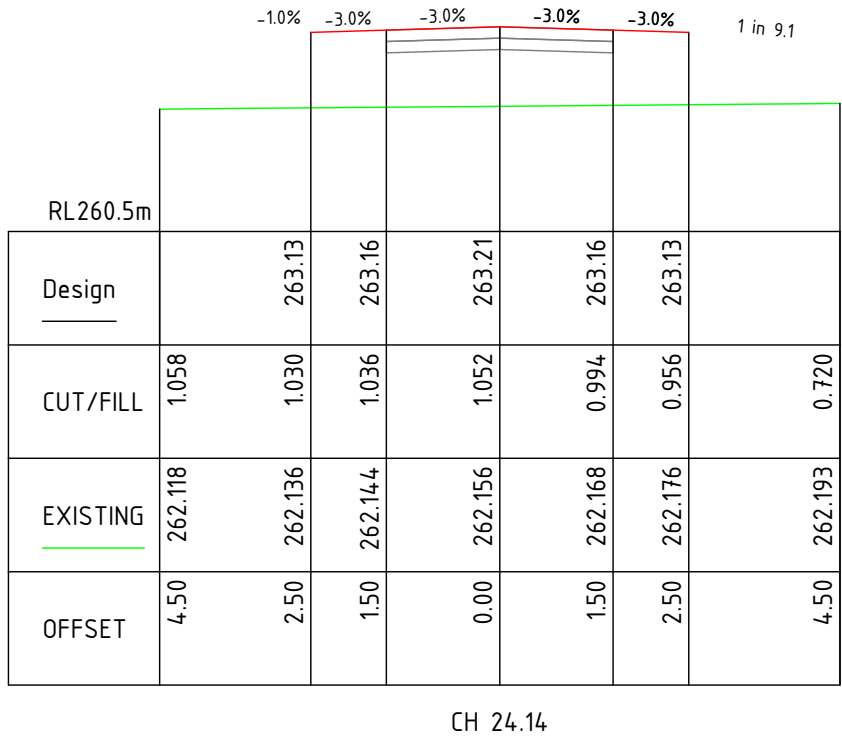
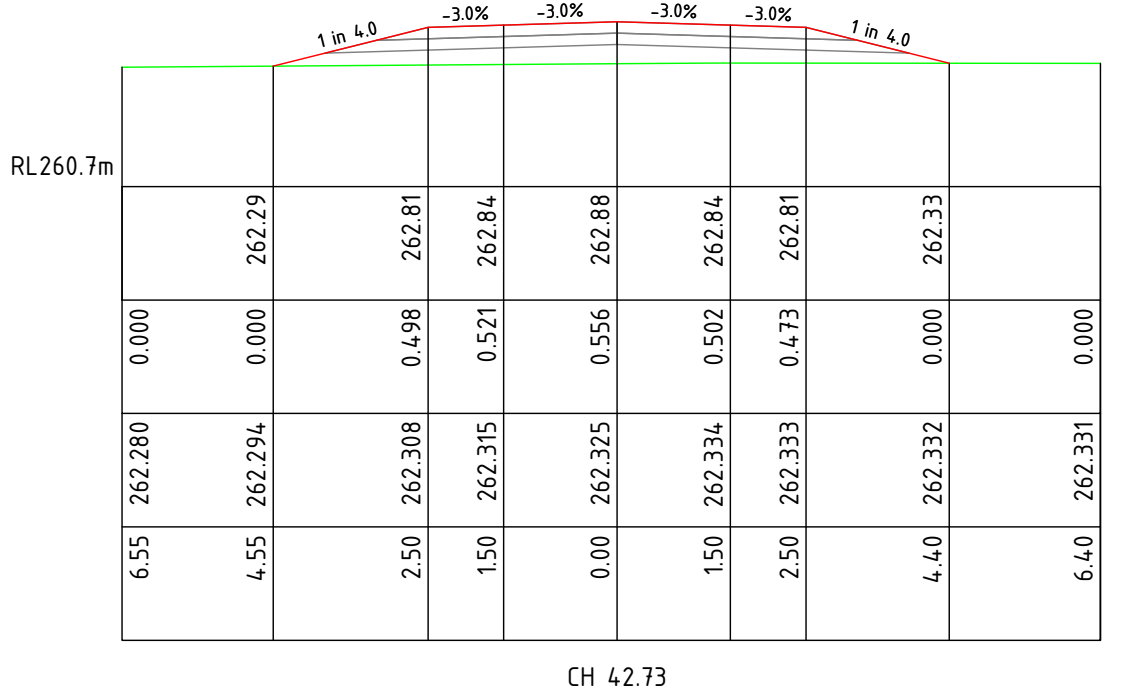
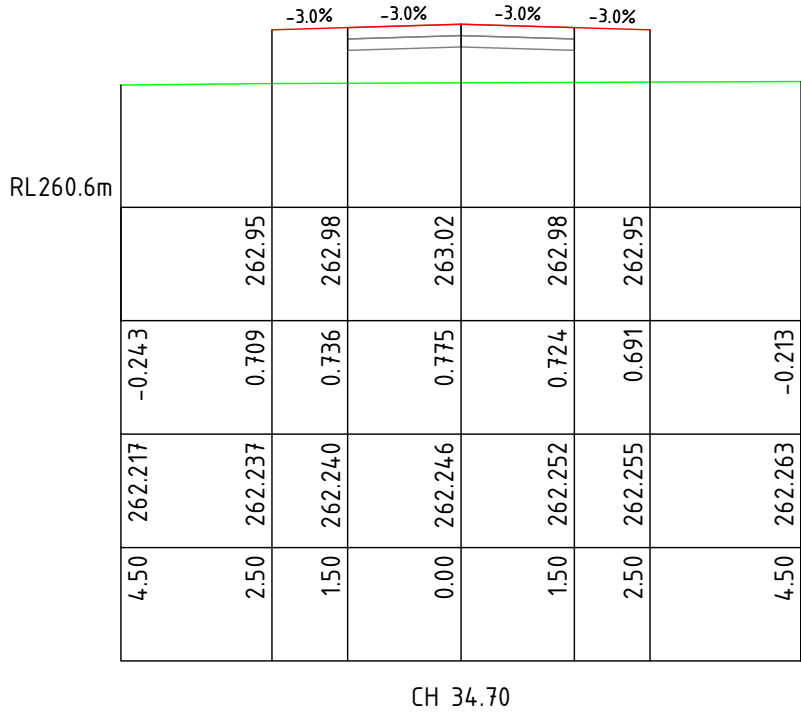
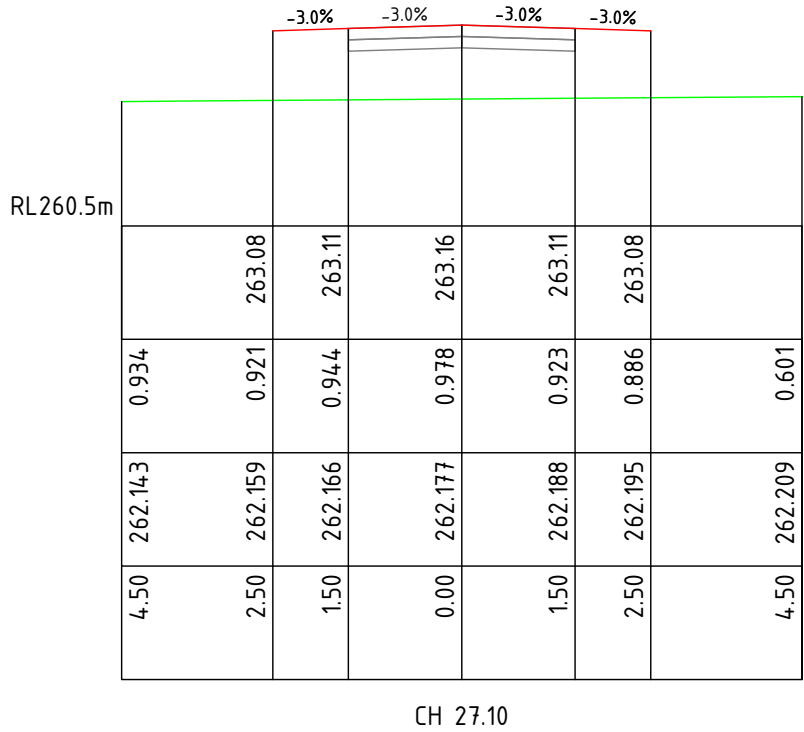
450Ø RCP CL4 RRJ  
LENGTH 7.32m, GRADE 1% (FALL WEST)  
INLET INV. 261.35, OUTLET INV. 261.275  
NEW CULVERT




SCALE: 1:1000H 1:100V  DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	REV.	DESCRIPTION	DATE	 REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404	CLIENT:  BLAND SHIRE COUNCIL	PROJECT:  WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 16	SHEETS 35
	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: ROAD 2 PLAN & LONG SECTION (360 - 560)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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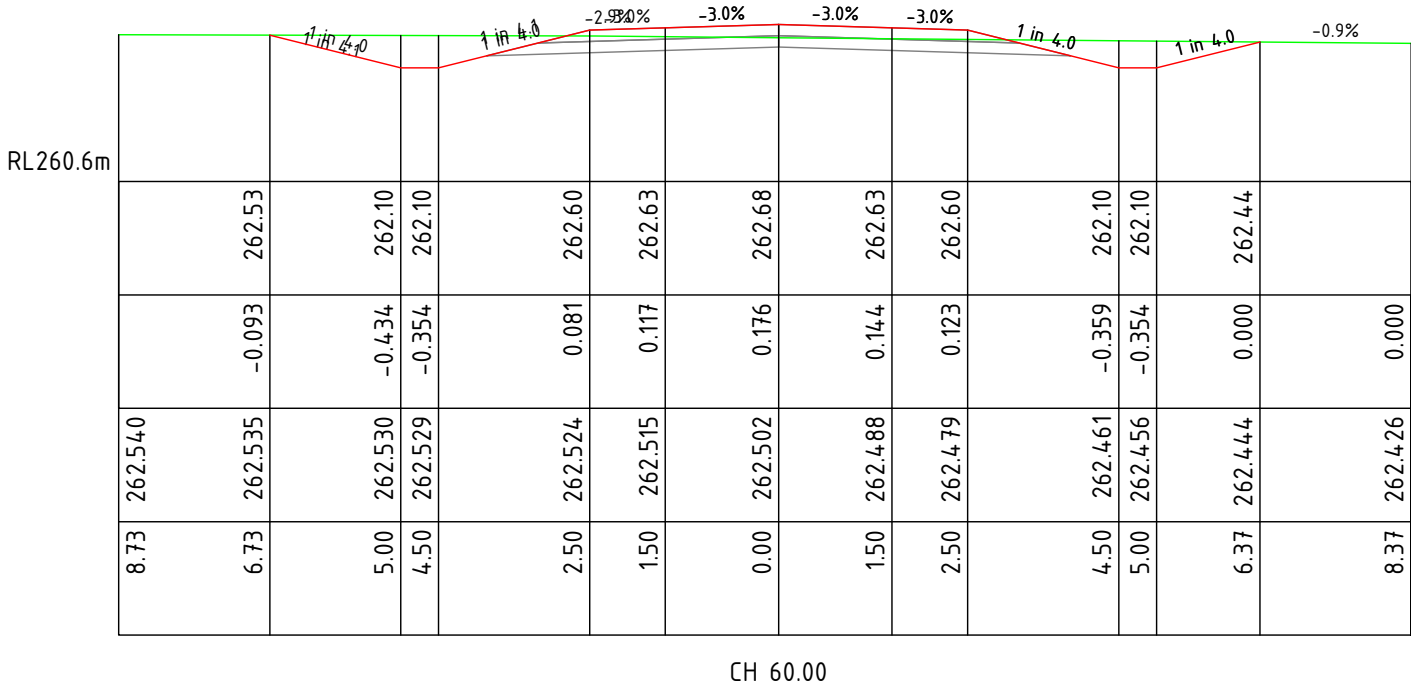
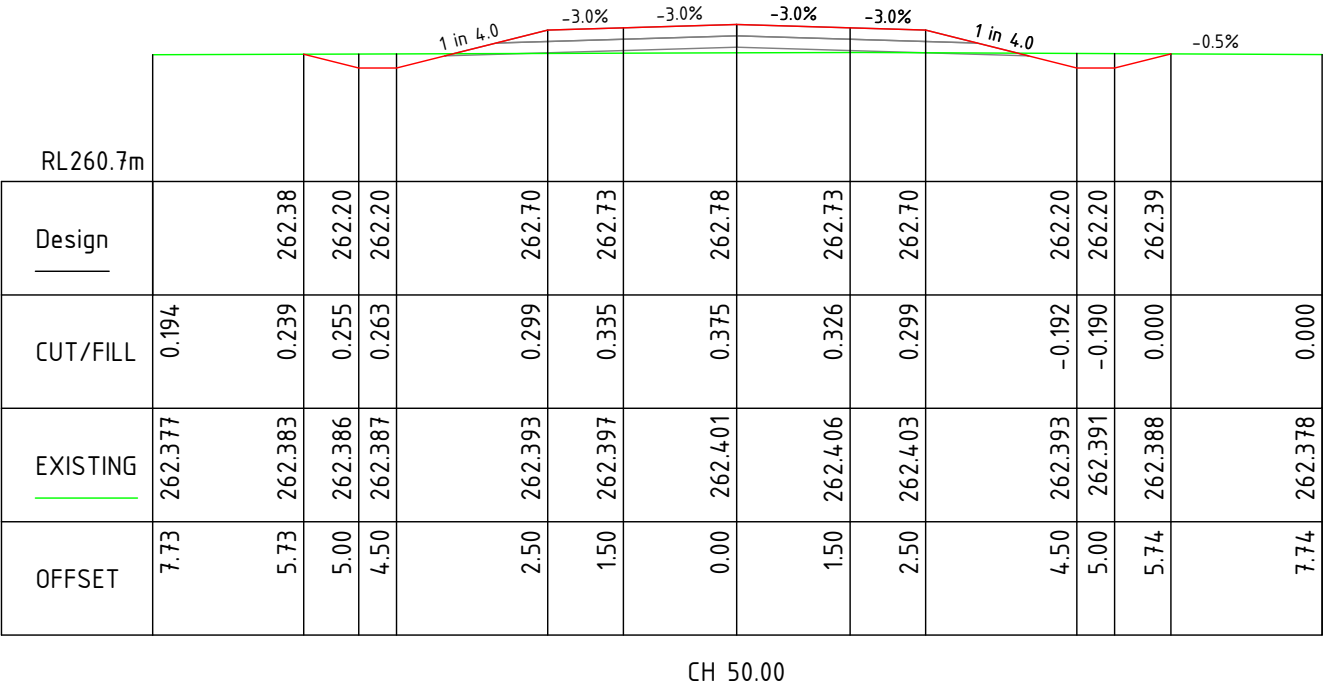
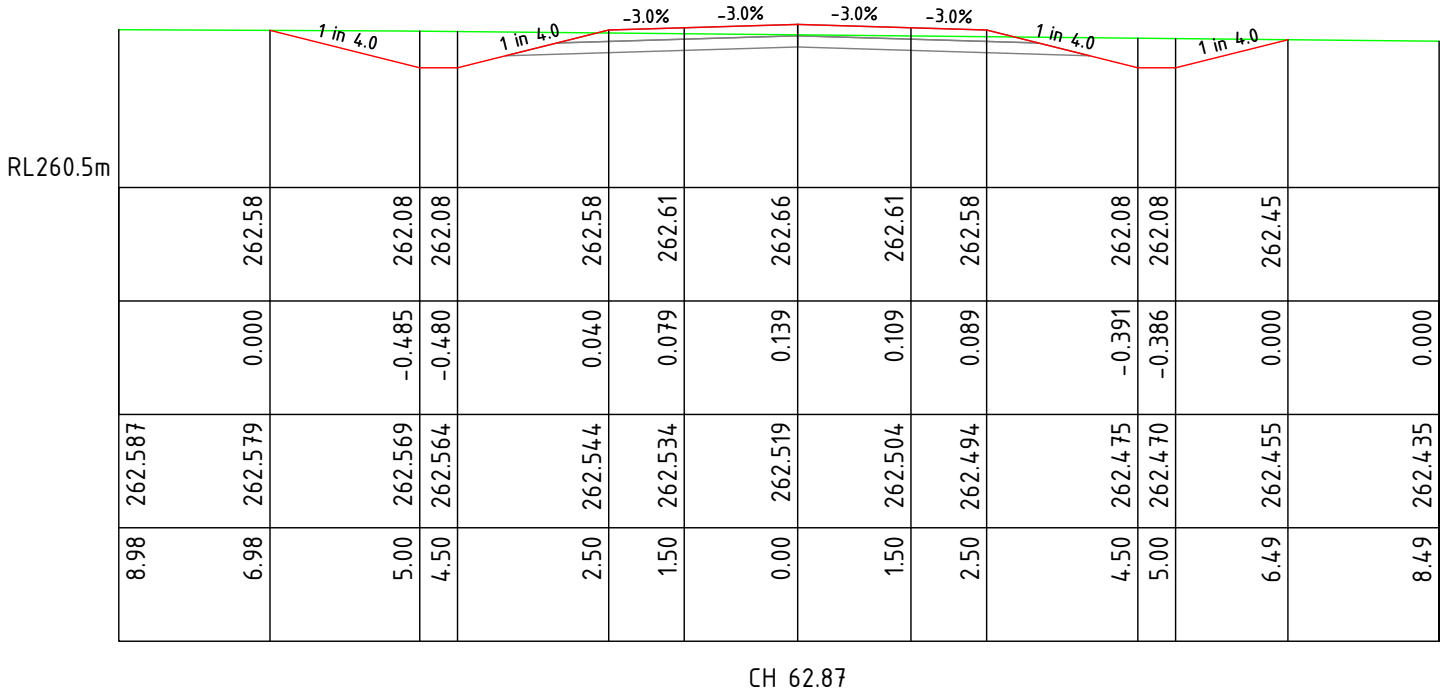
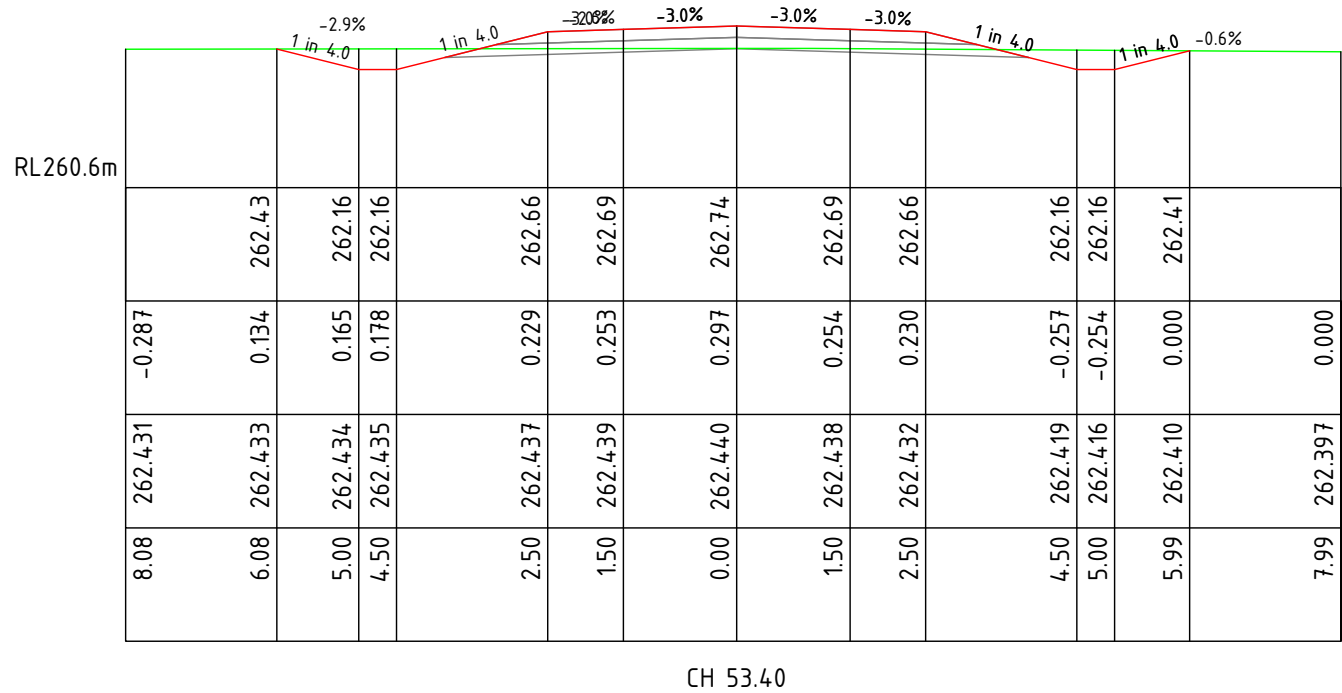
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SCALE: 1:100		REV.	DESCRIPTION	DATE	 REGIONAL engineering SERVICES  REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404	CLIENT:  BLAND SHIRE COUNCIL	PROJECT:  WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 17	SHEETS 35
DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE: ROAD 2 CROSS SECTIONS (24.14 - 42.73)					
	C	FOR COUNCIL APPROVAL	22-06-23						
	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						

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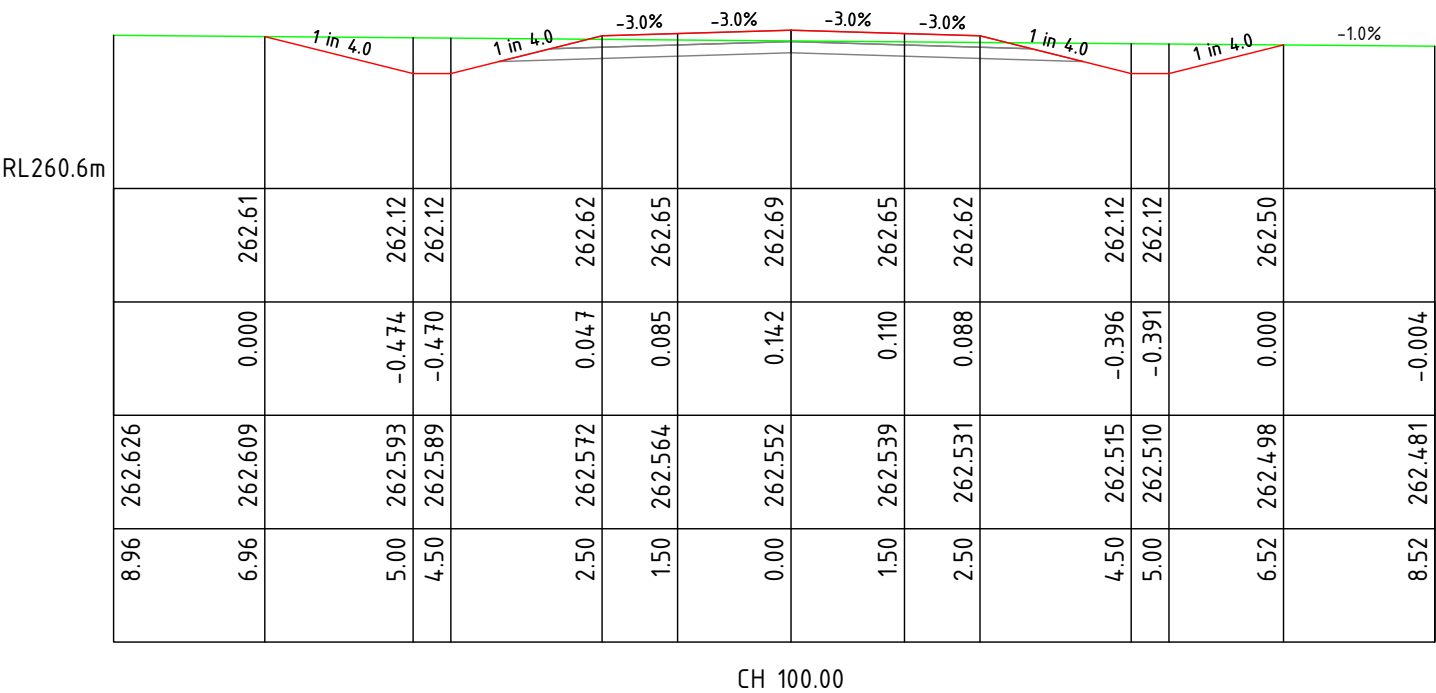
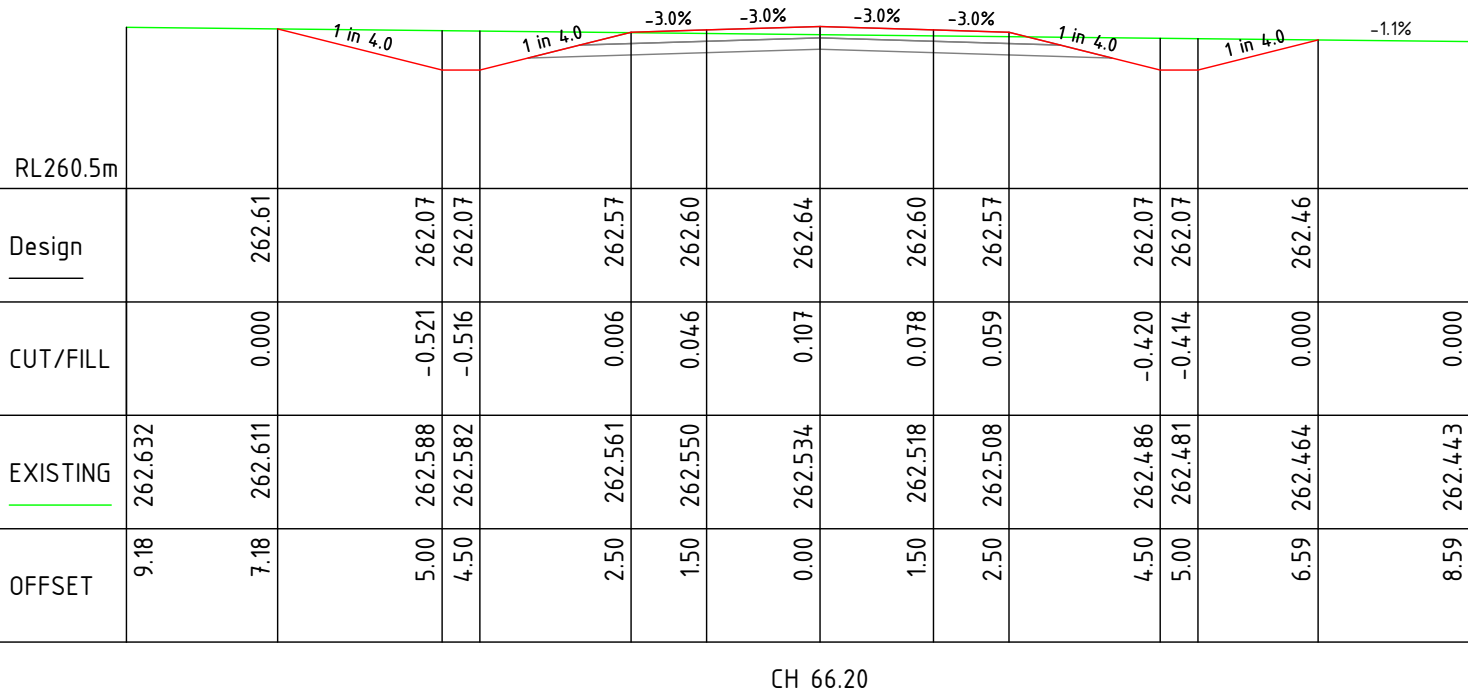
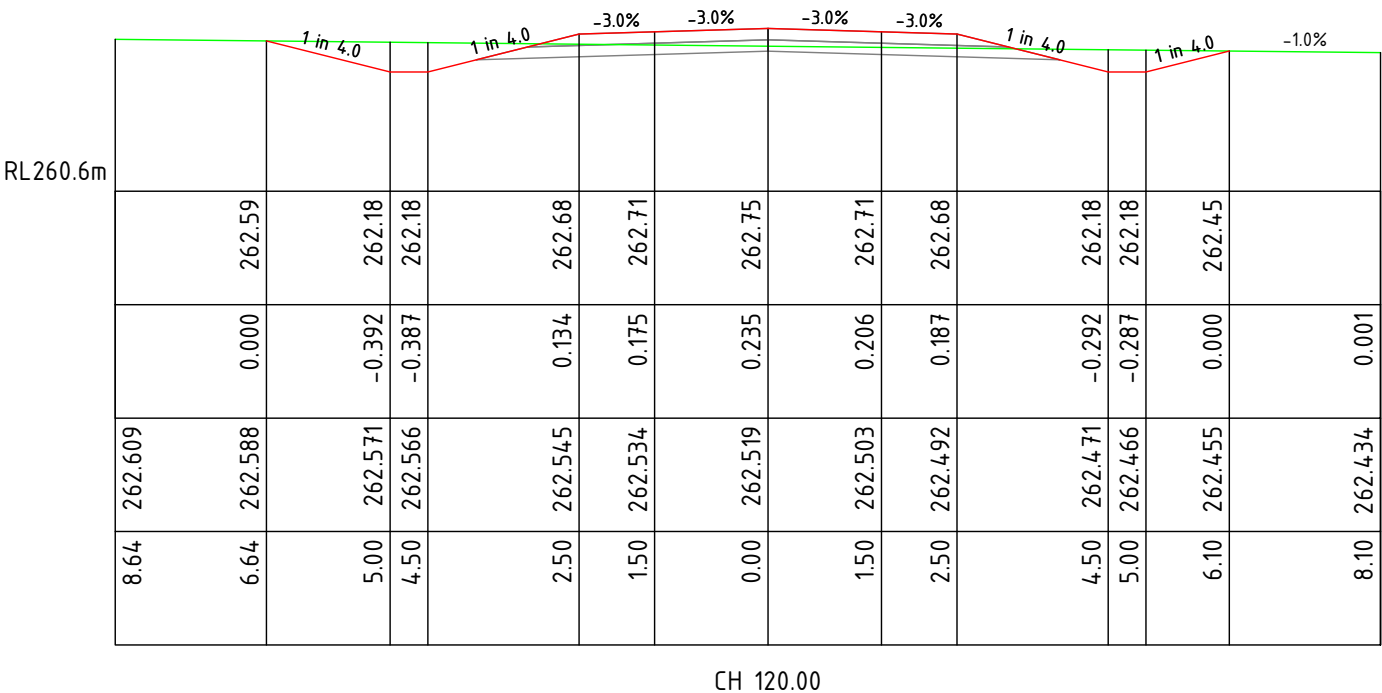
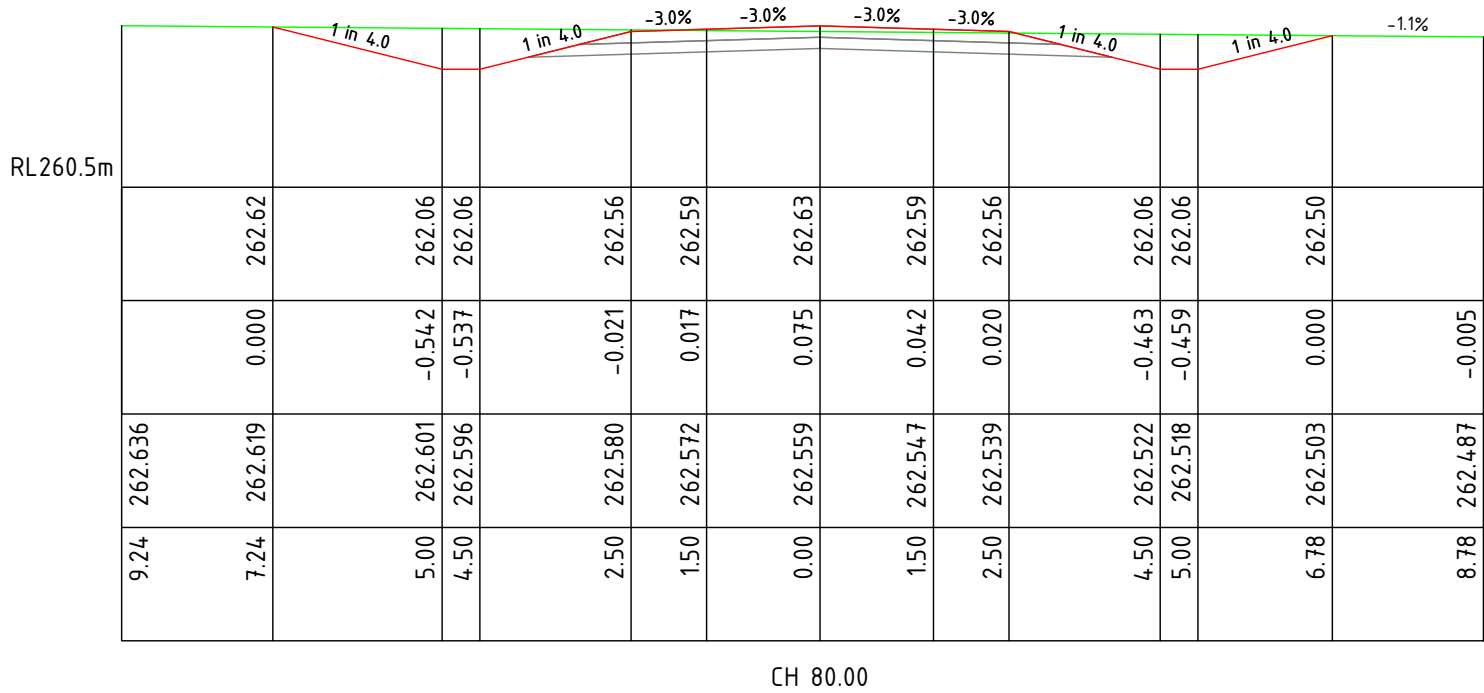



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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE: ROAD 2 CROSS SECTIONS (50 - 62.87)					
	C	FOR COUNCIL APPROVAL	22-06-23						
	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						

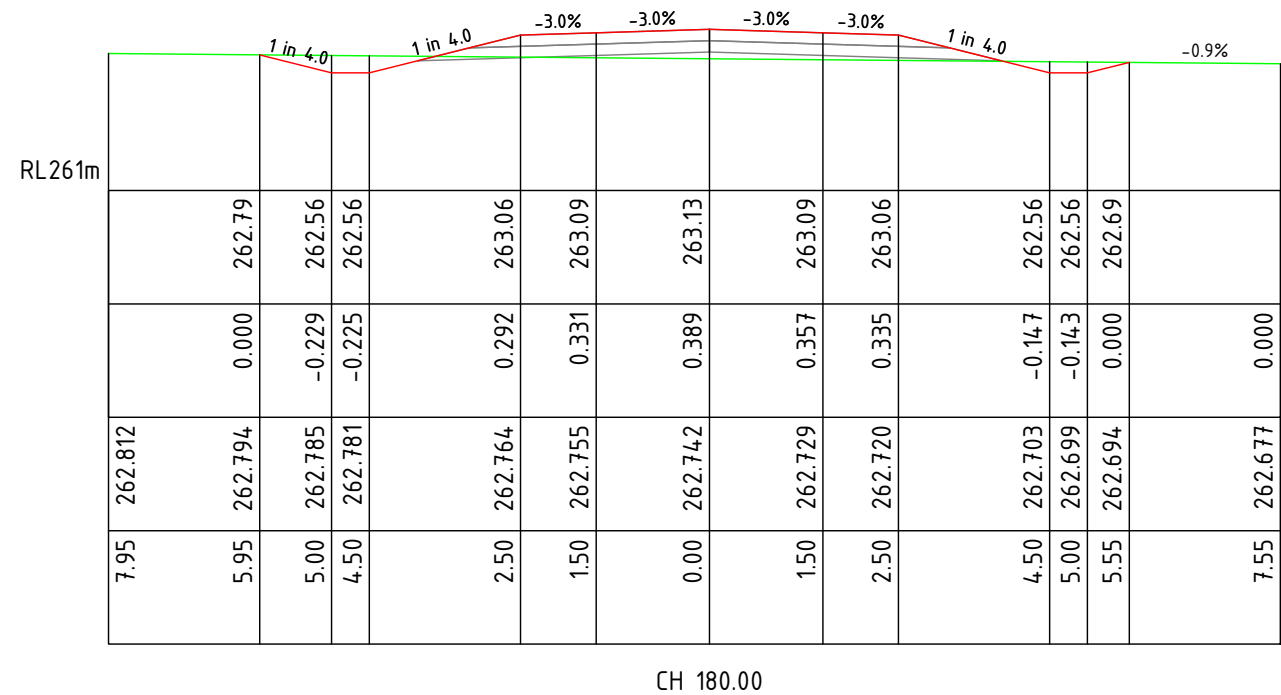
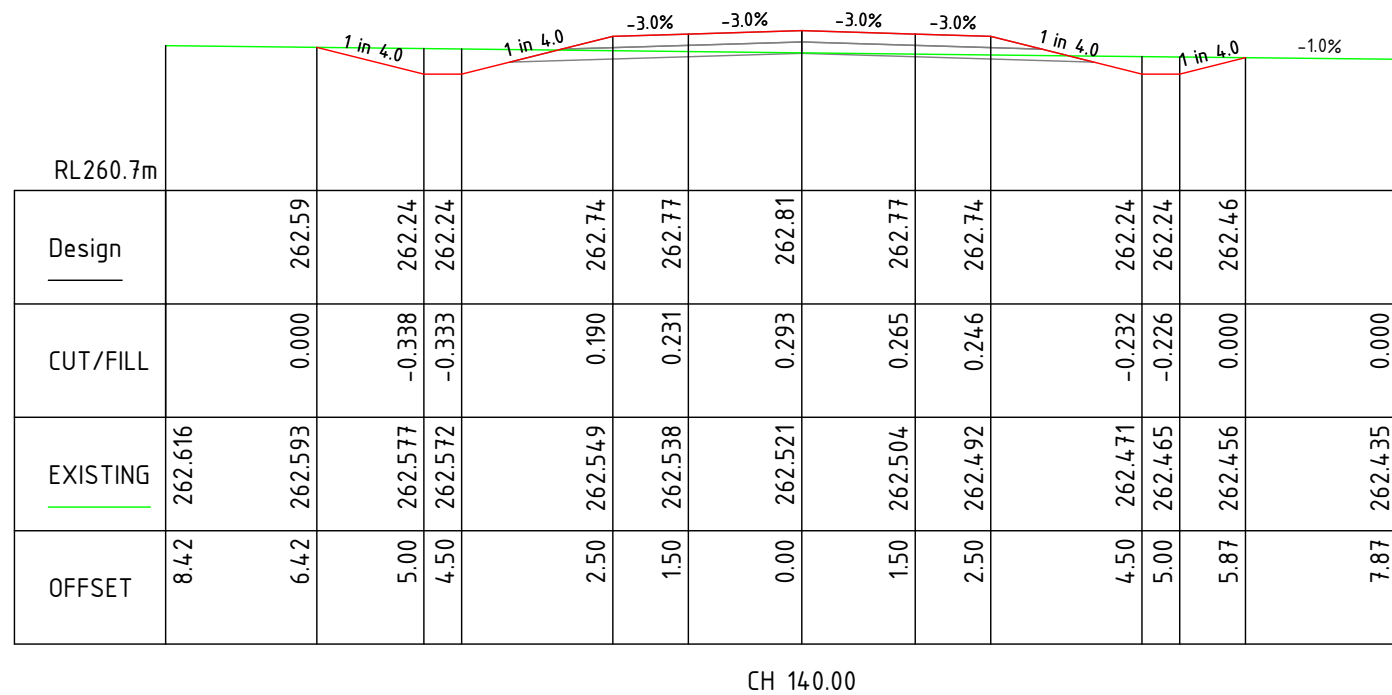
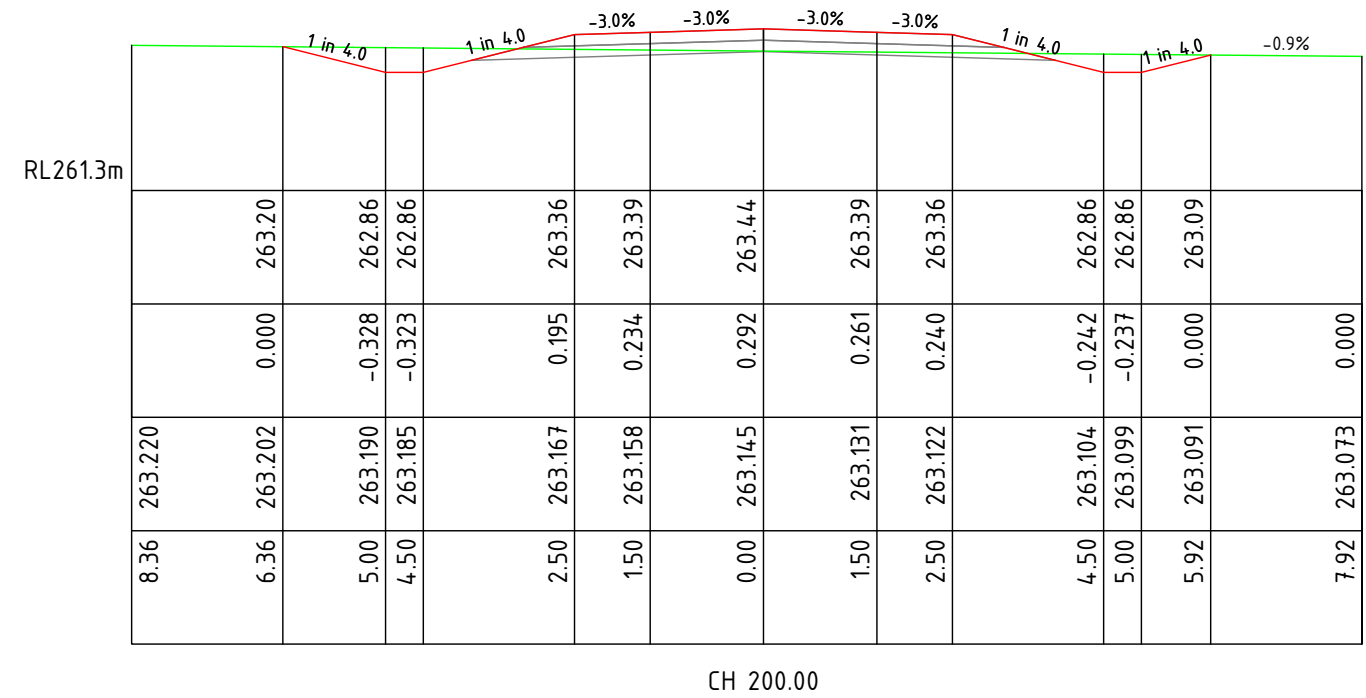
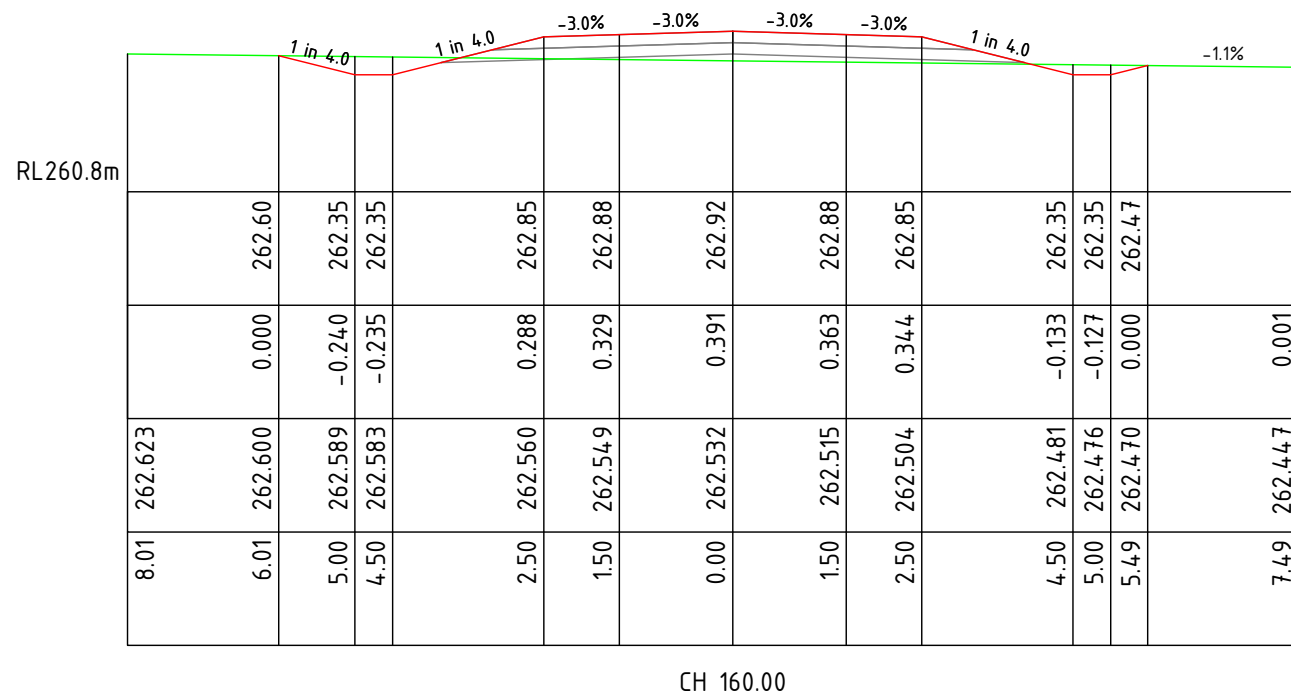



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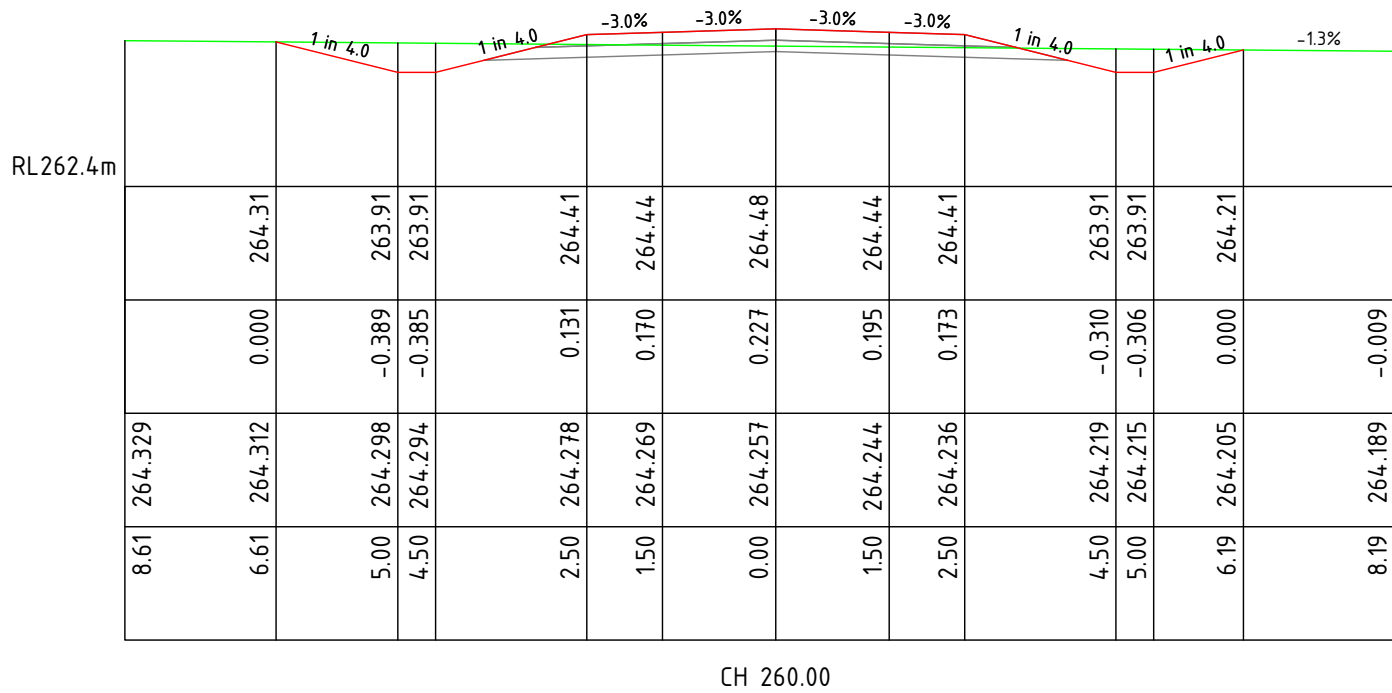
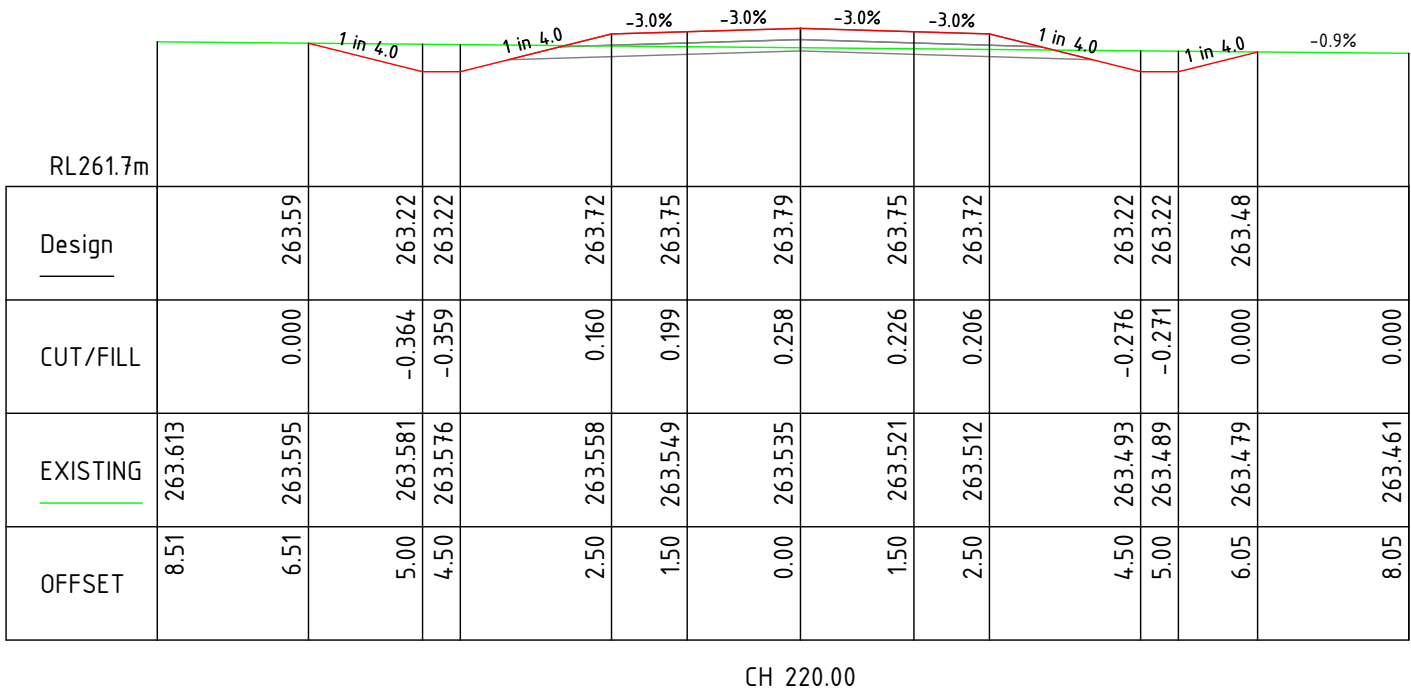
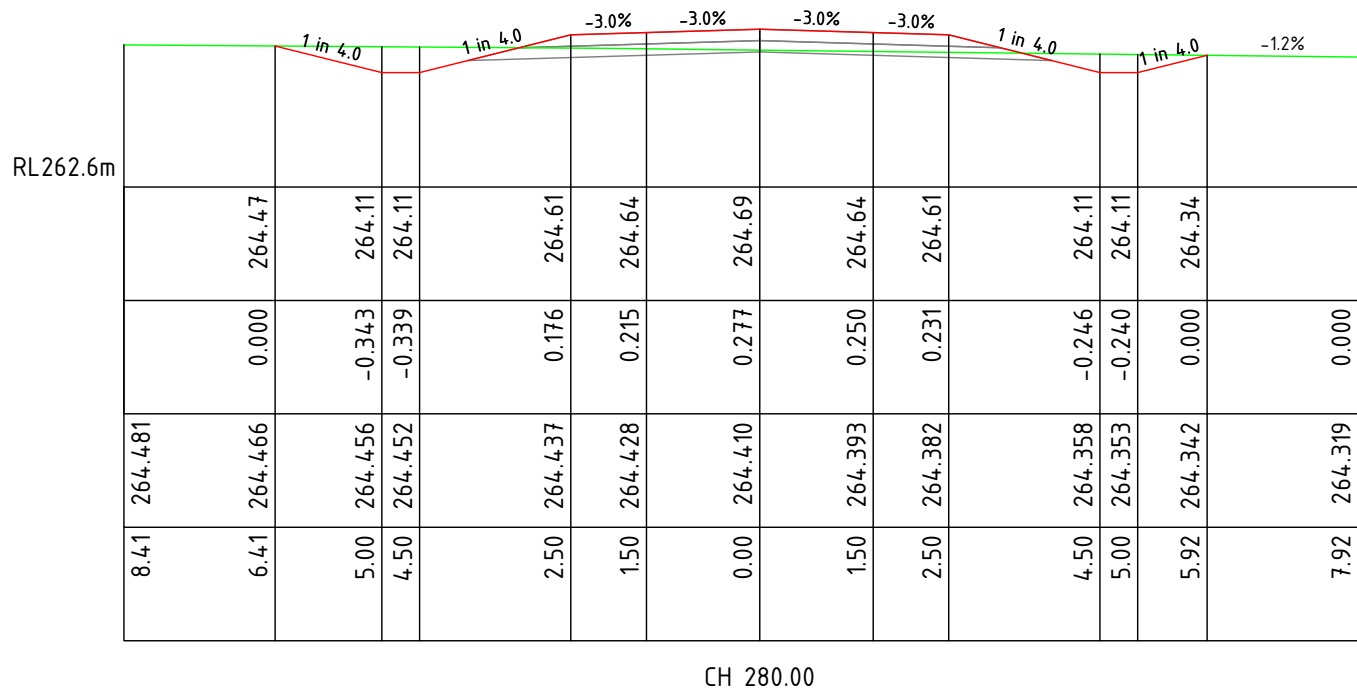
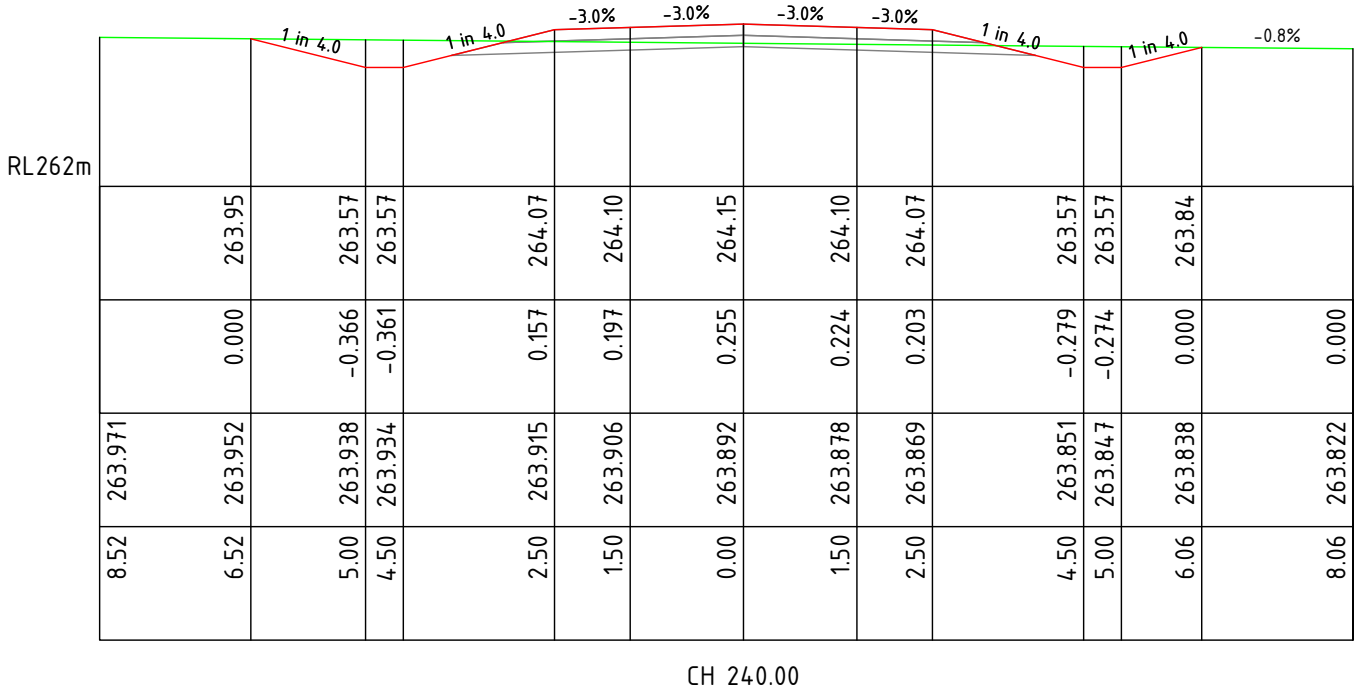
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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: ROAD 2 CROSS SECTIONS (66.2 - 120)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					




SCALE:	1:200H 1:100V	REV.	DESCRIPTION	DATE	 REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404	CLIENT:  BLAND SHIRE COUNCIL	PROJECT:  WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 20	SHEETS 35
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23					
PROJECTION:	GDA2020 (ZONE 55)	C	FOR COUNCIL APPROVAL	22-06-23					
SURVEYED BY:		B	FOR REVIEW	19-06-23					
		A	DRAFT - FOR COUNCIL COMMENT	17-04-23					
					SHEET TITLE: ROAD 2 CROSS SECTIONS (140 - 200)				

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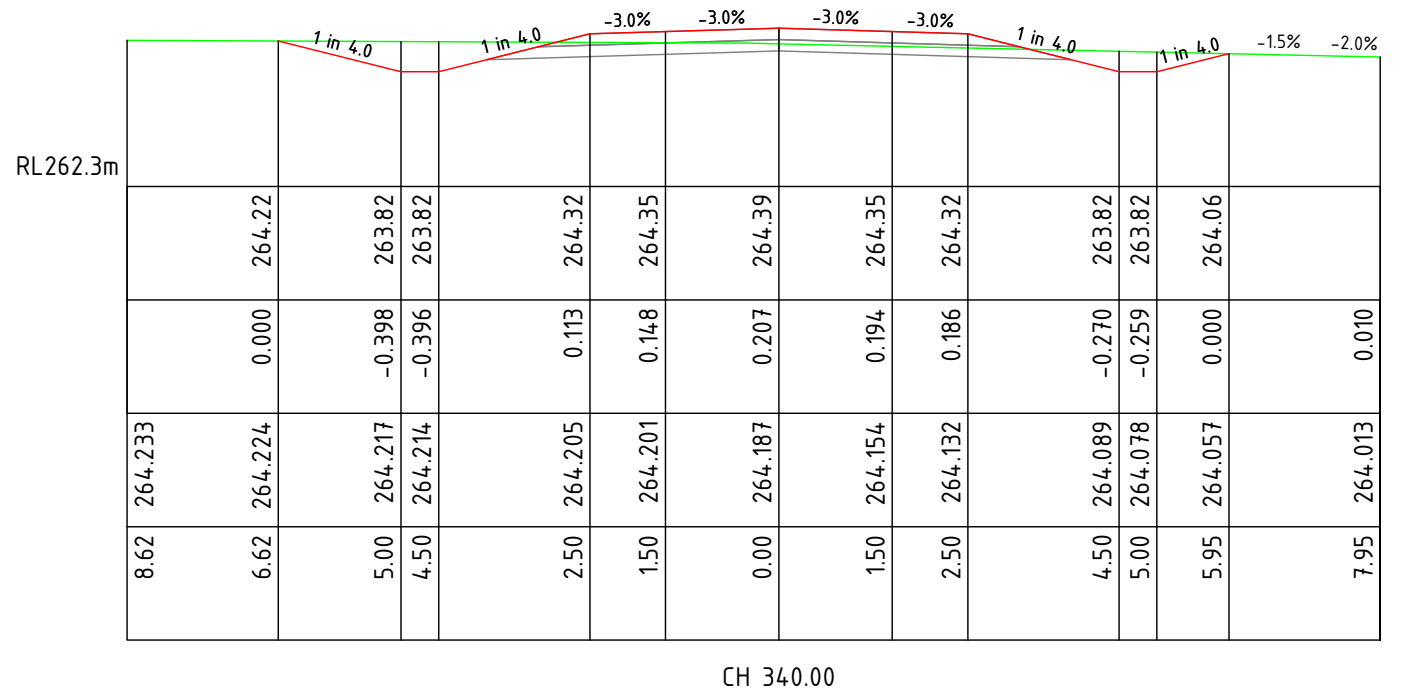
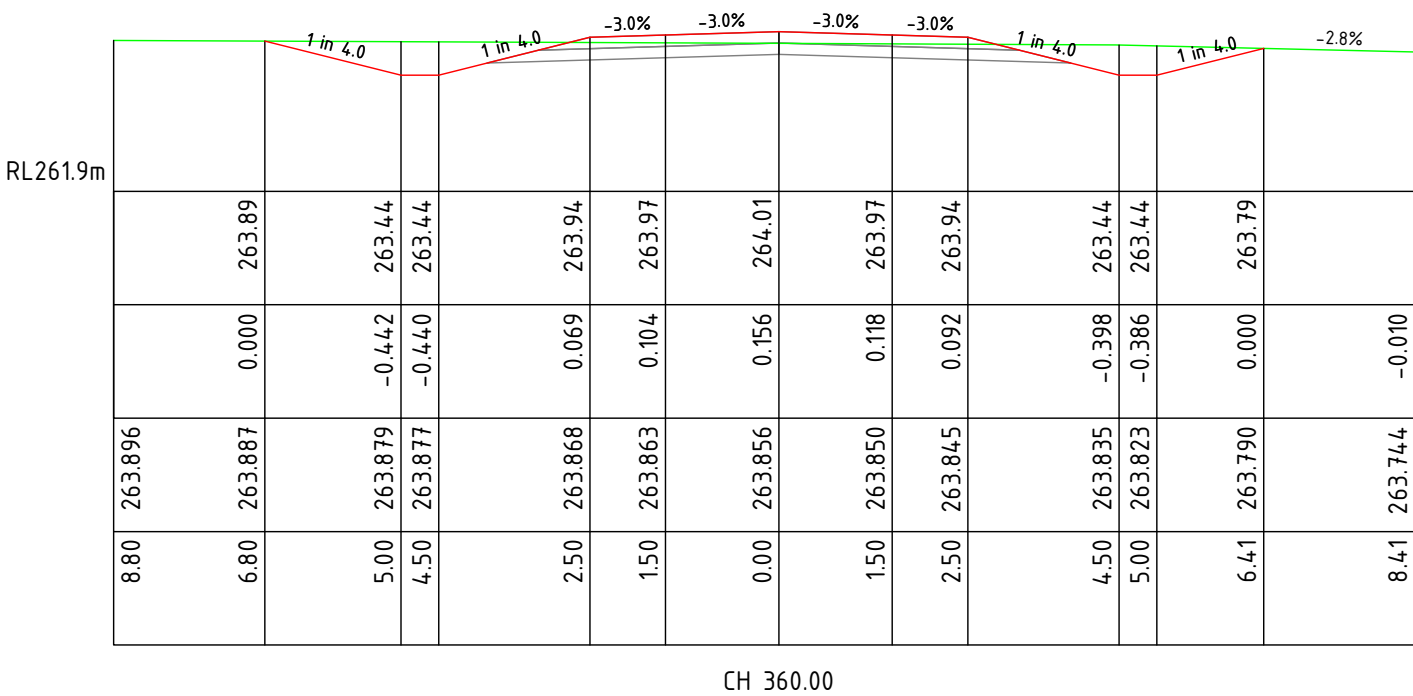
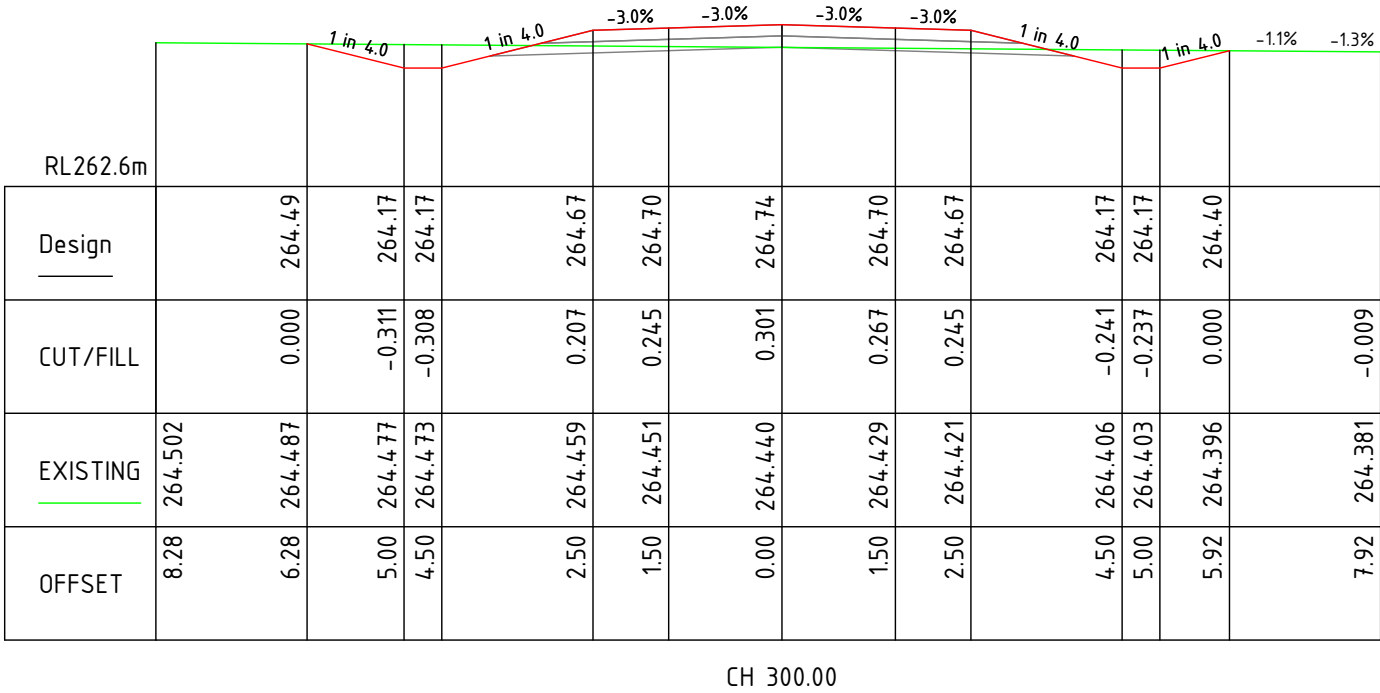
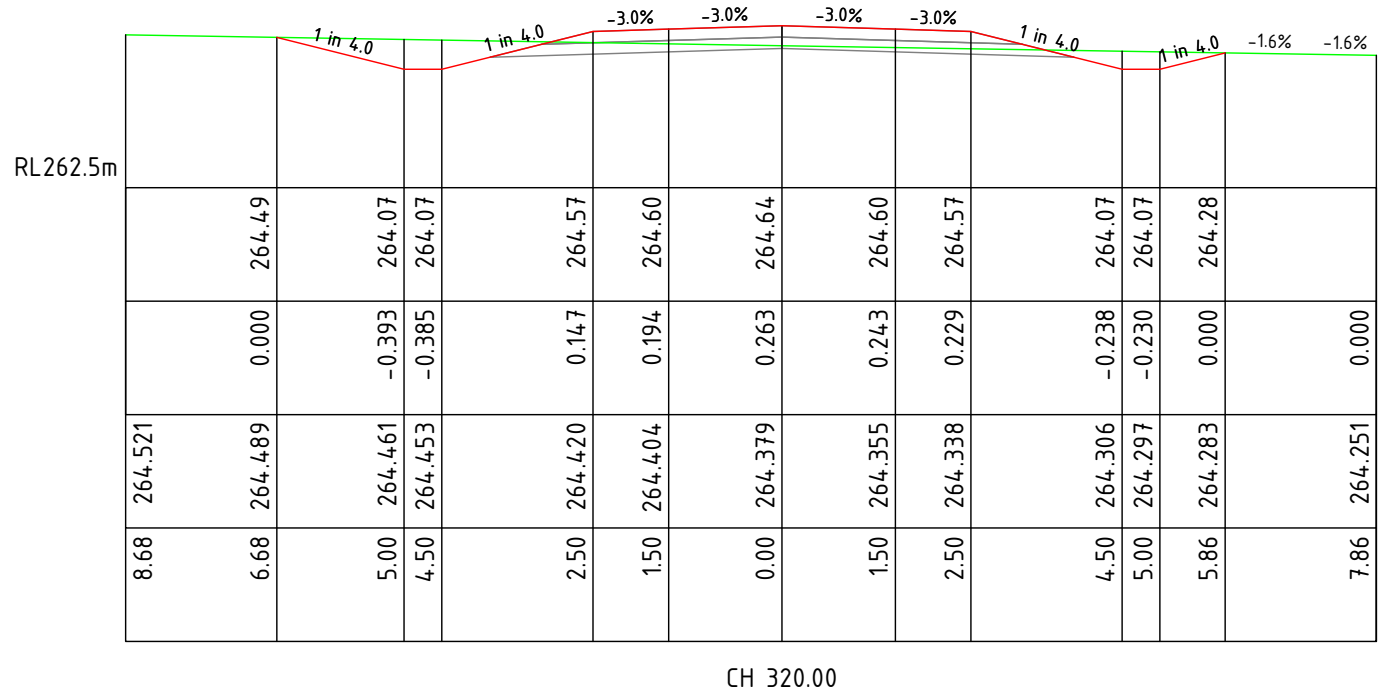
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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE: ROAD 2 CROSS SECTIONS 220 - 280)					
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	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						

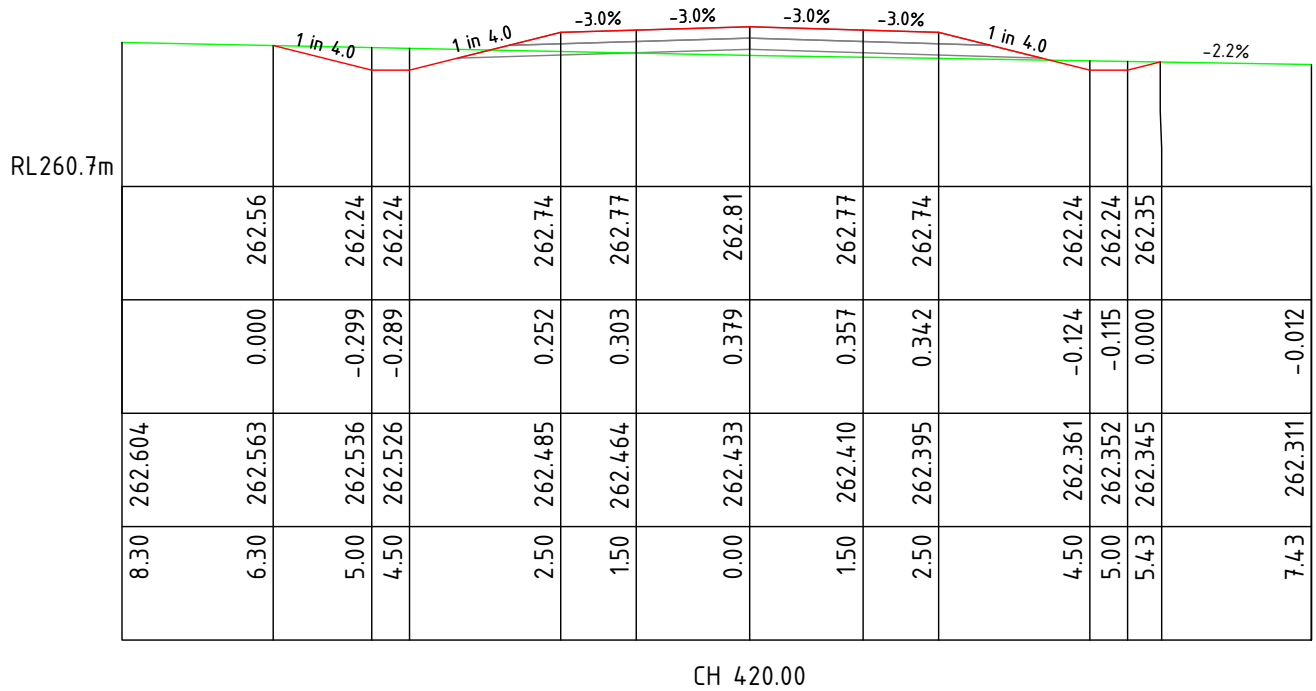
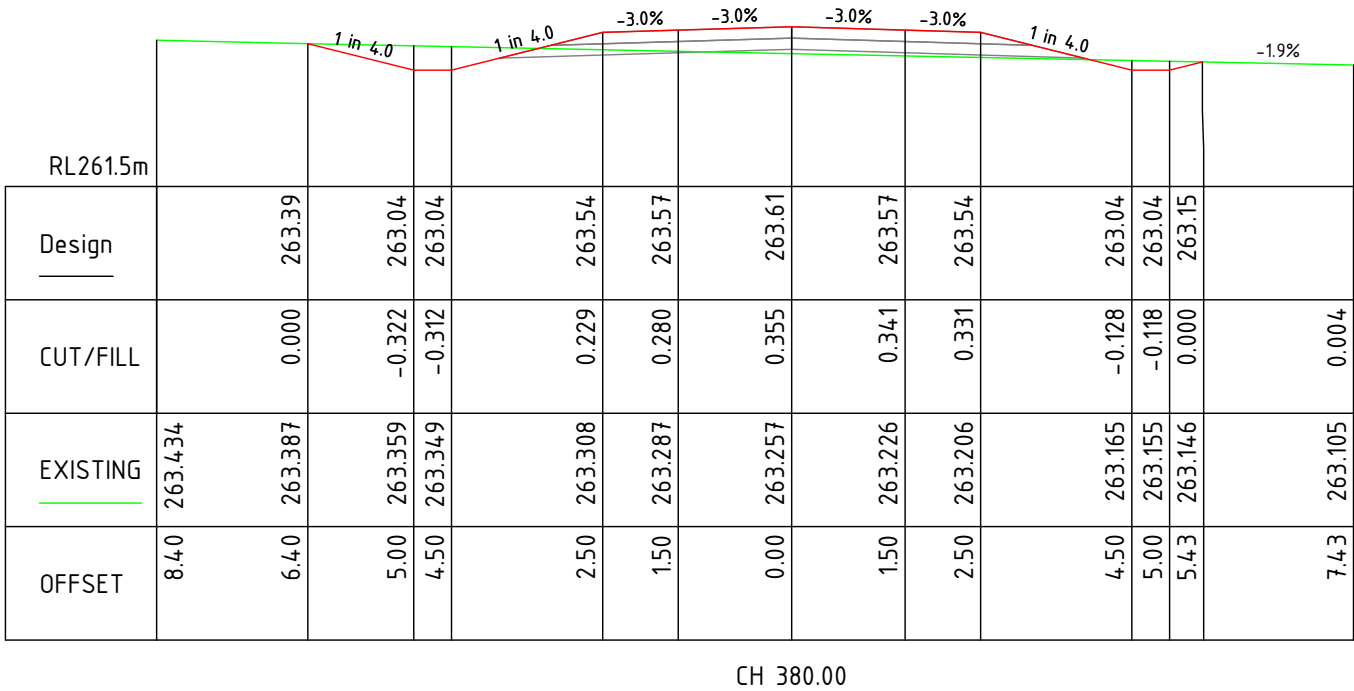
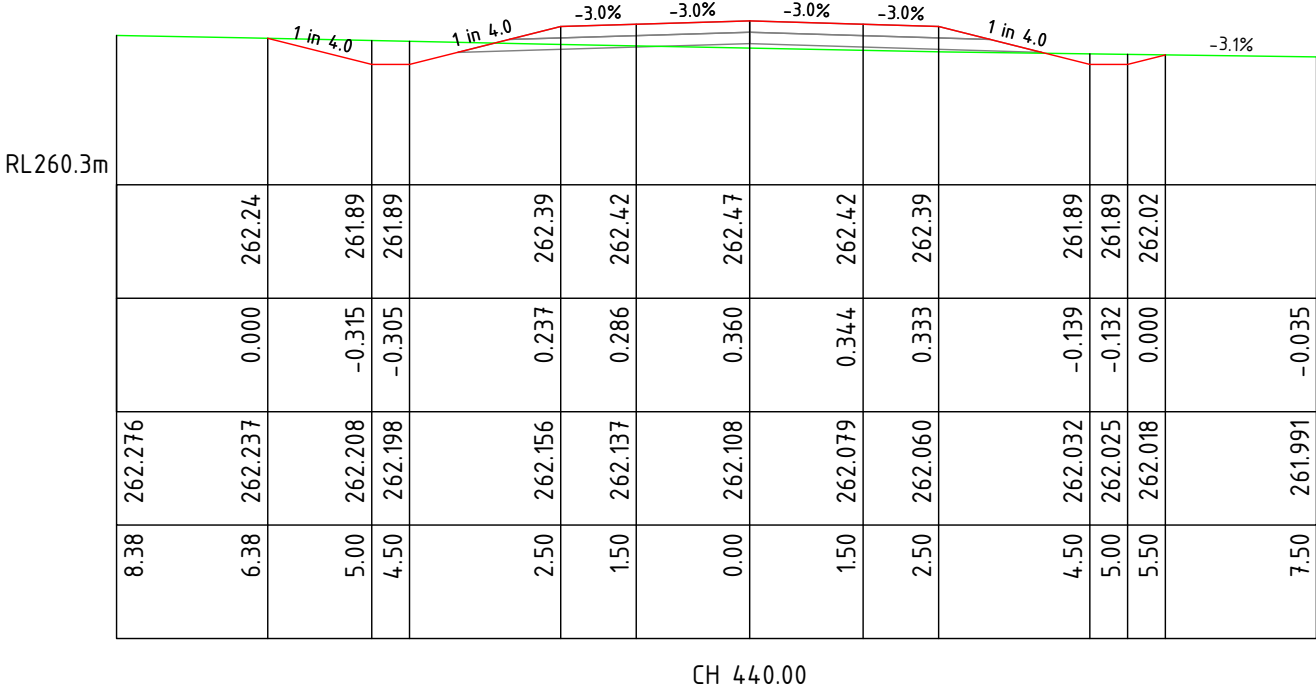
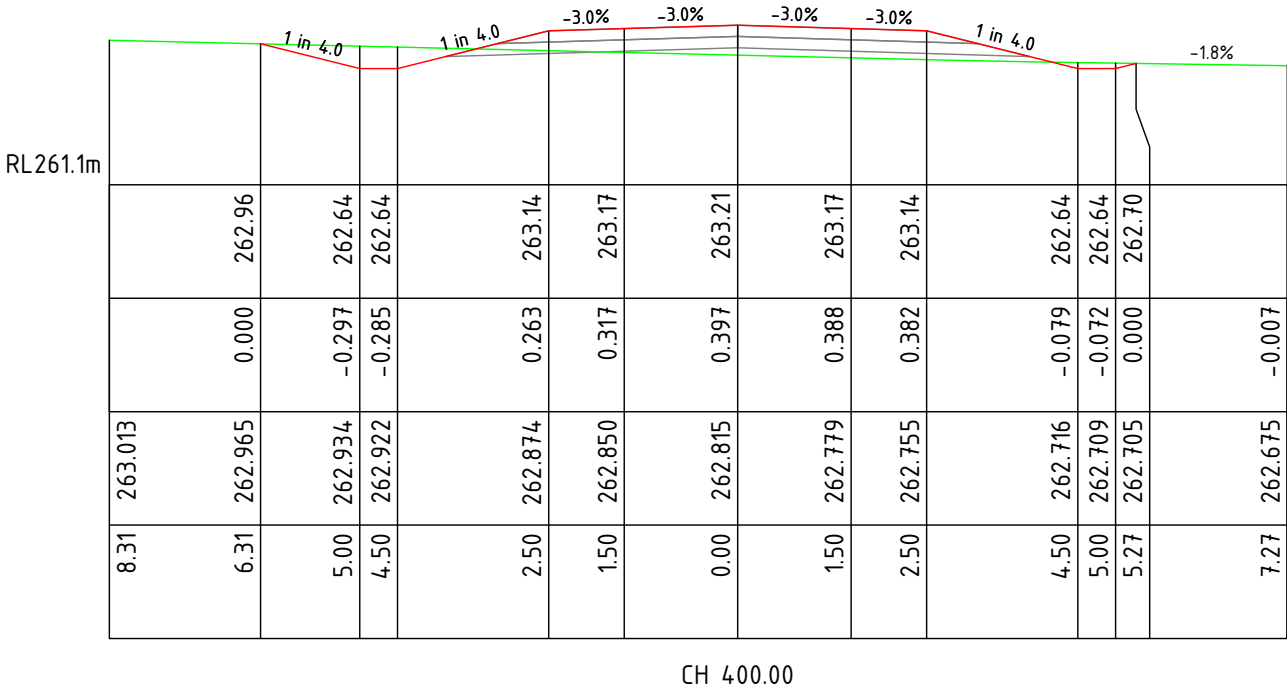
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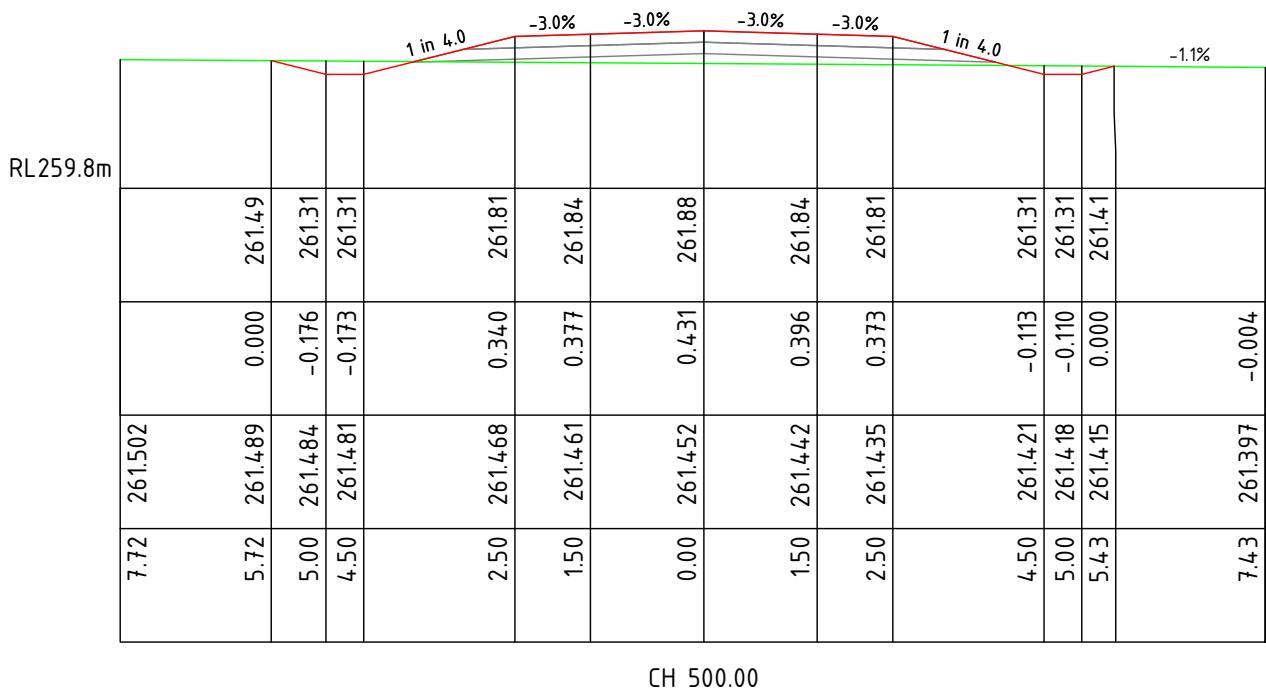
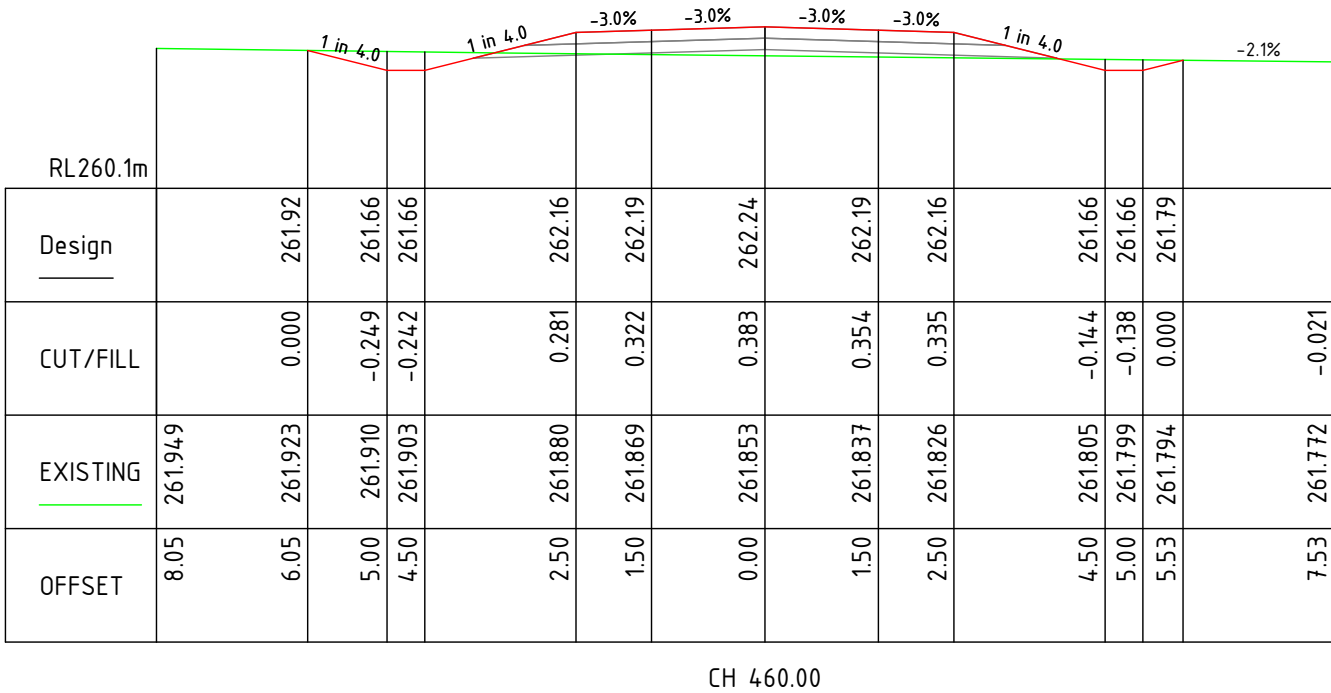
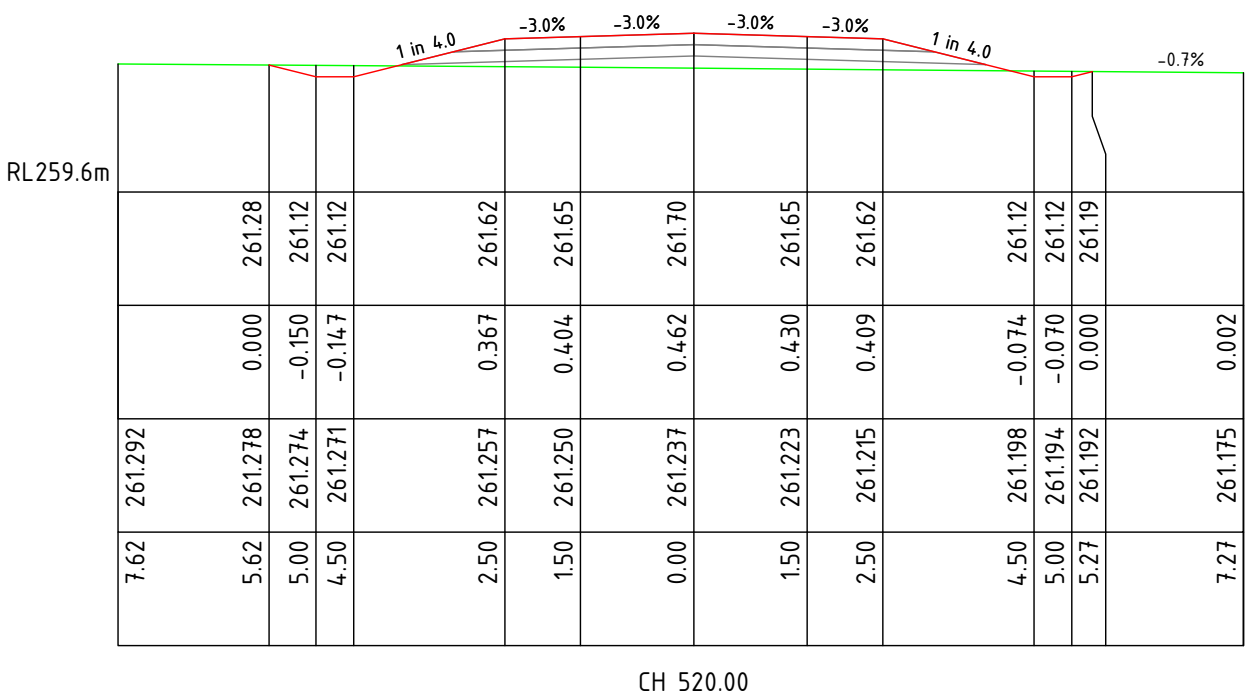
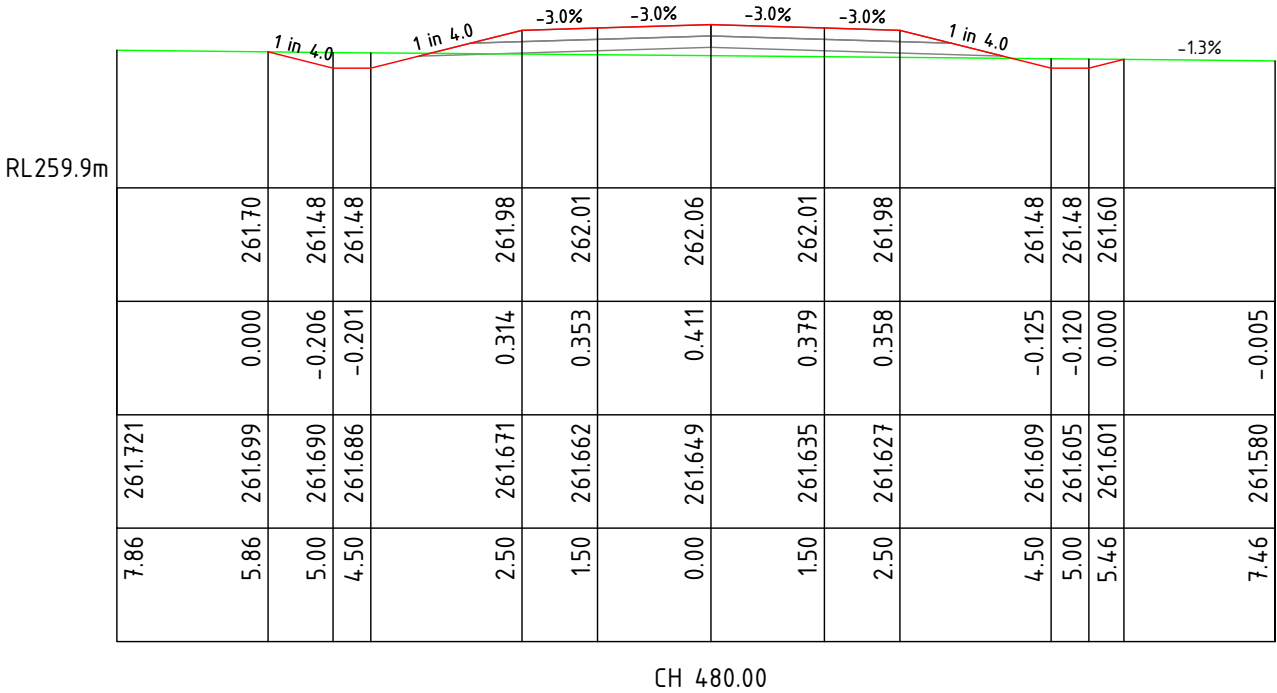
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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:		D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: ROAD 2 CROSS SECTIONS (380 - 440)	
		C	FOR COUNCIL APPROVAL	22-06-23					
		B	FOR REVIEW	19-06-23					
		A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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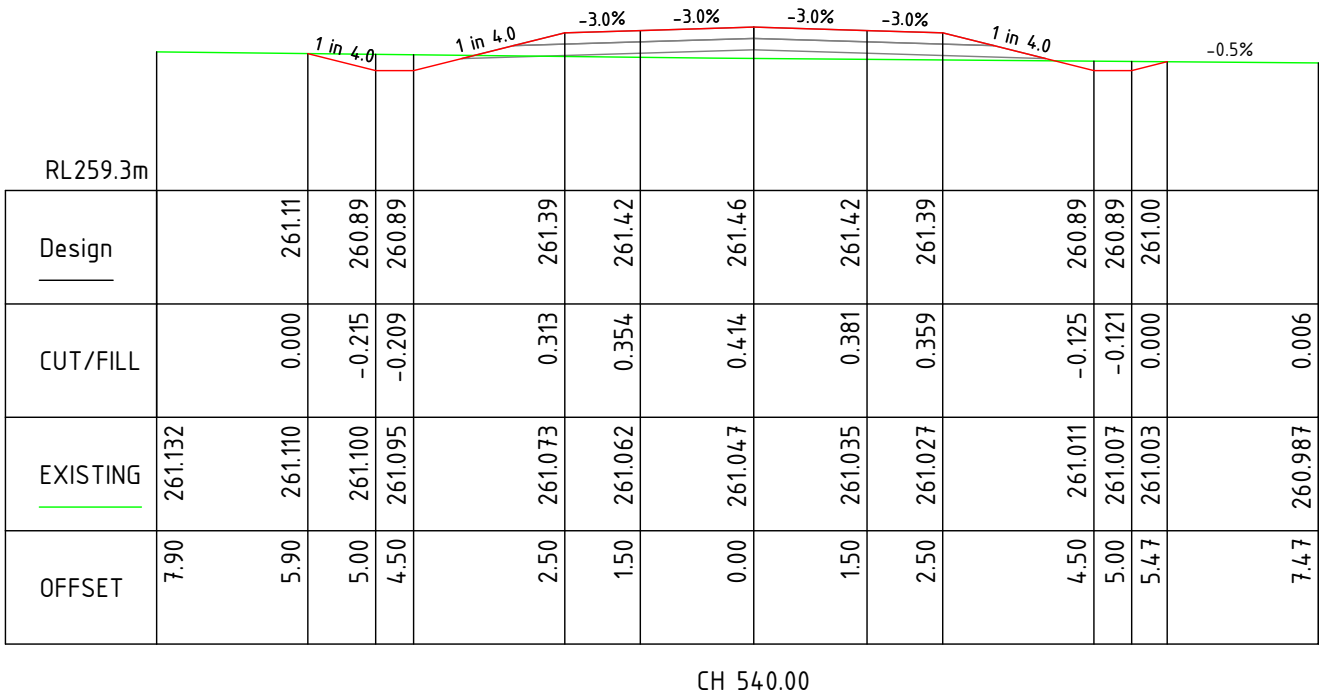
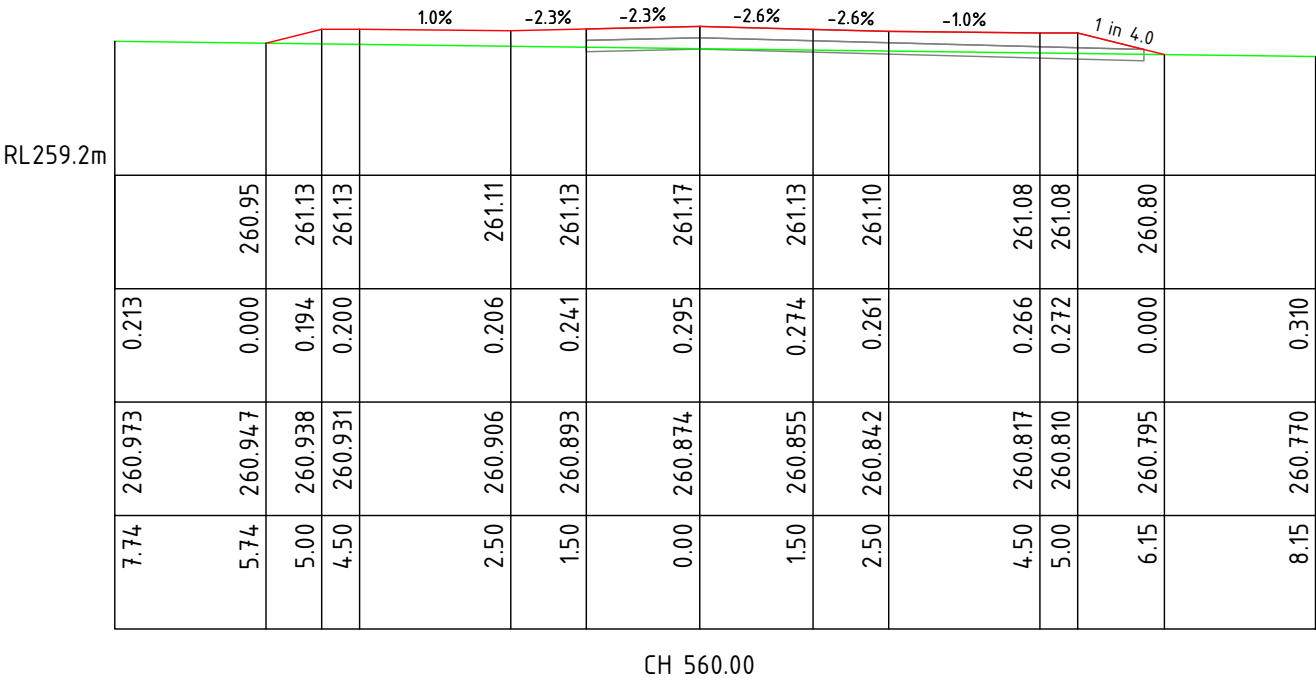
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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: ROAD 2 CROSS SECTIONS (460 - 520)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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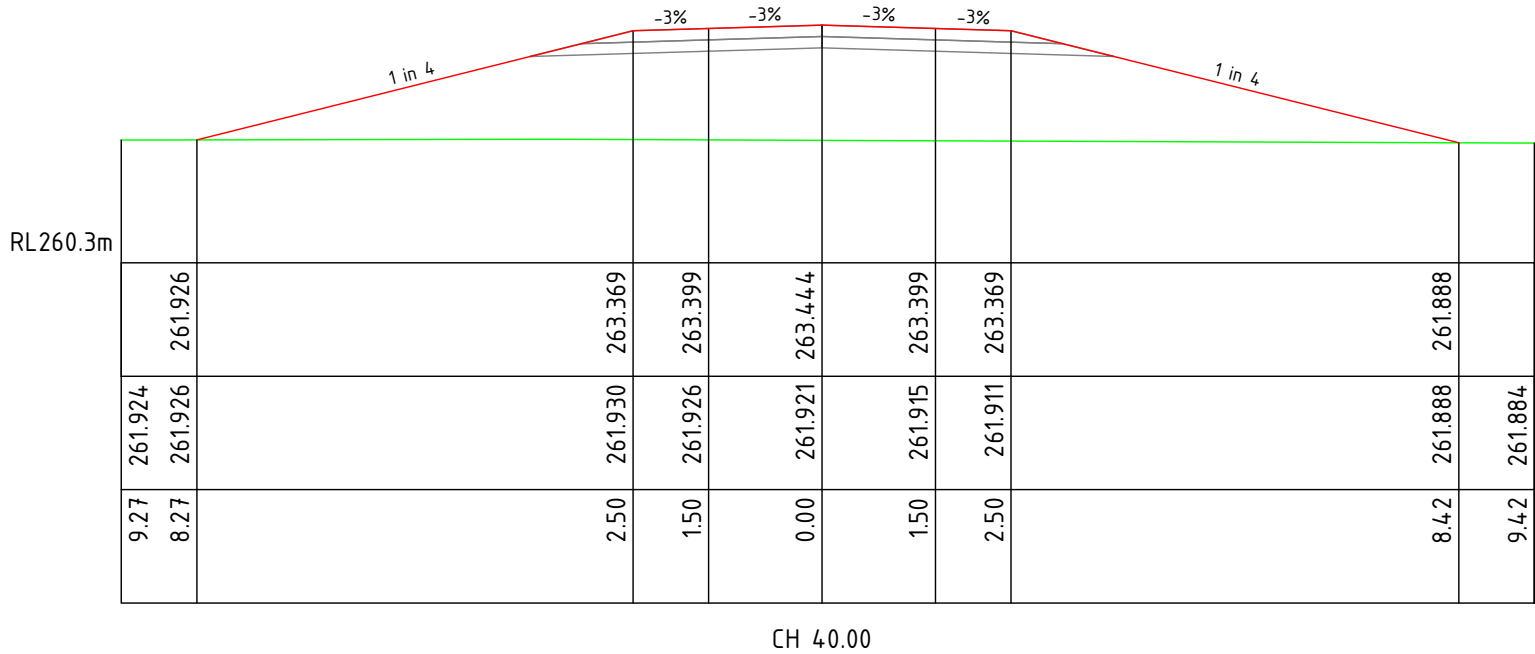
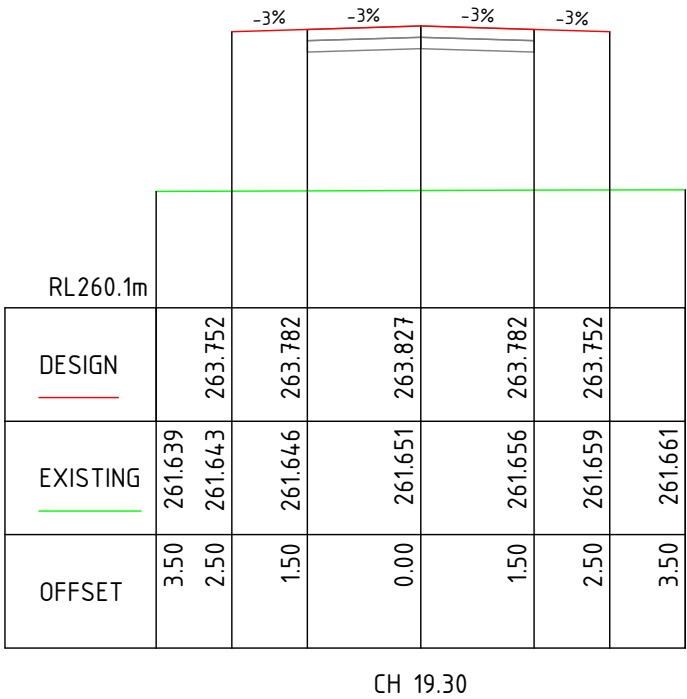
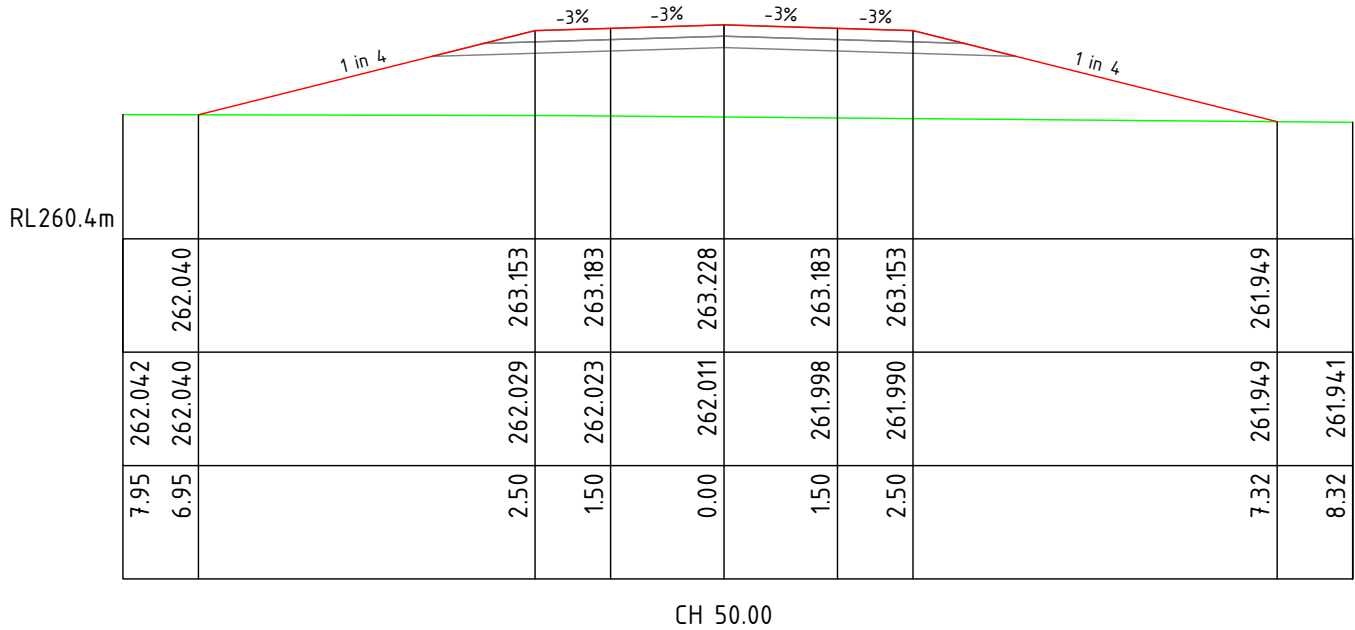
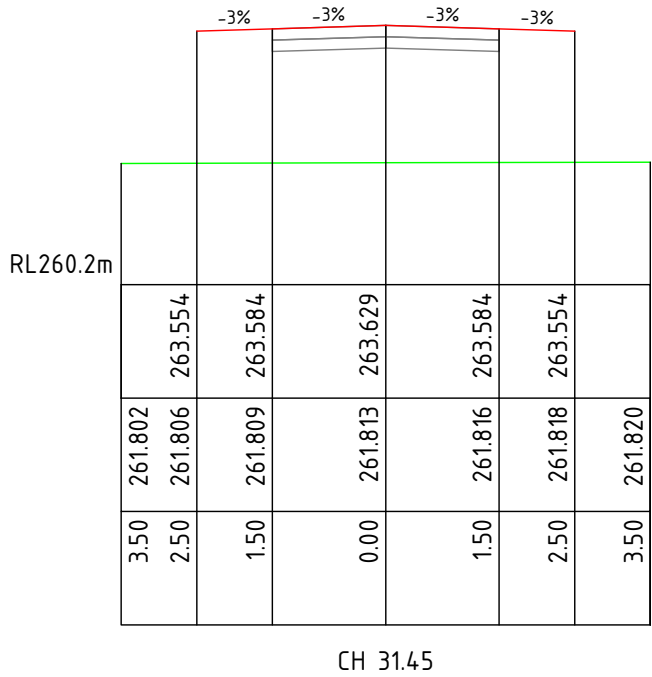
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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:		D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: ROAD 2 CROSS SECTIONS (540 - 560)	
		C	FOR COUNCIL APPROVAL	22-06-23					
		B	FOR REVIEW	19-06-23					
		A	DRAFT - FOR COUNCIL COMMENT	17-04-23					






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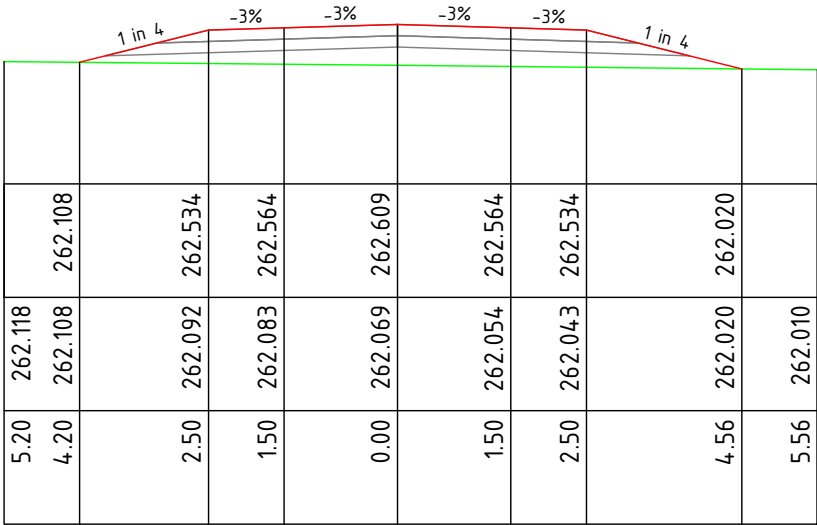


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	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: ROAD 3 CROSS SECTIONS (19.3 - 50)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					
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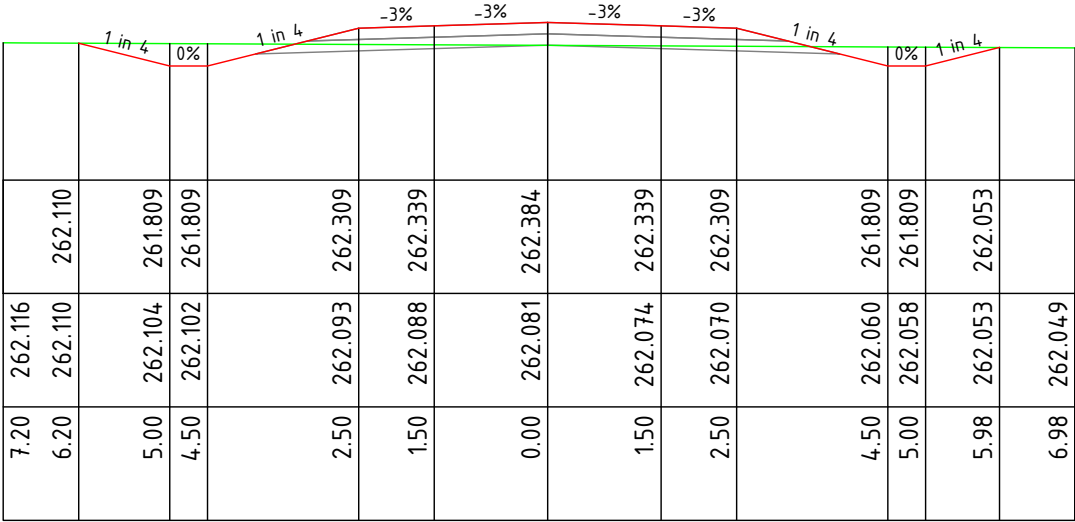
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RL260.5m



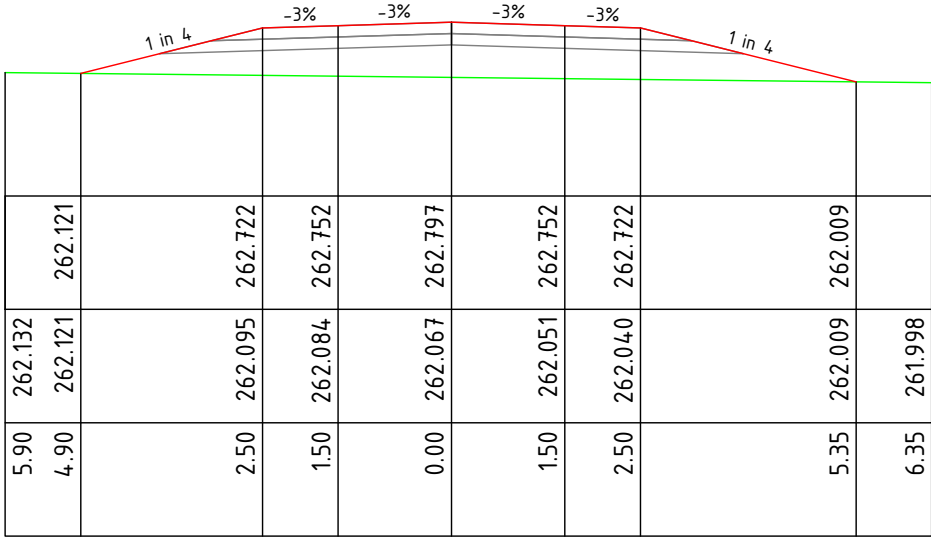
CH 80.00

RL260.3m



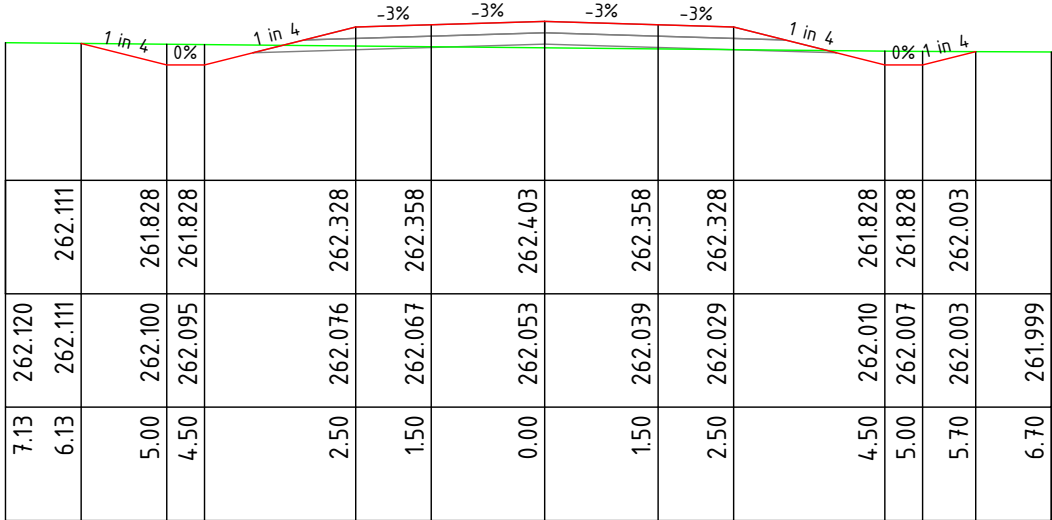
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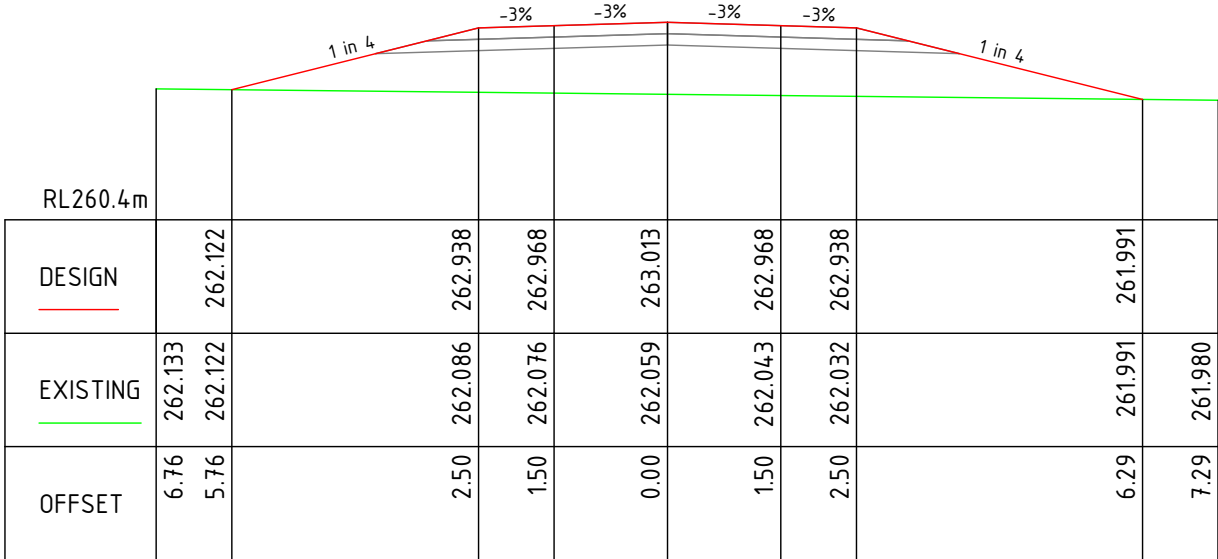
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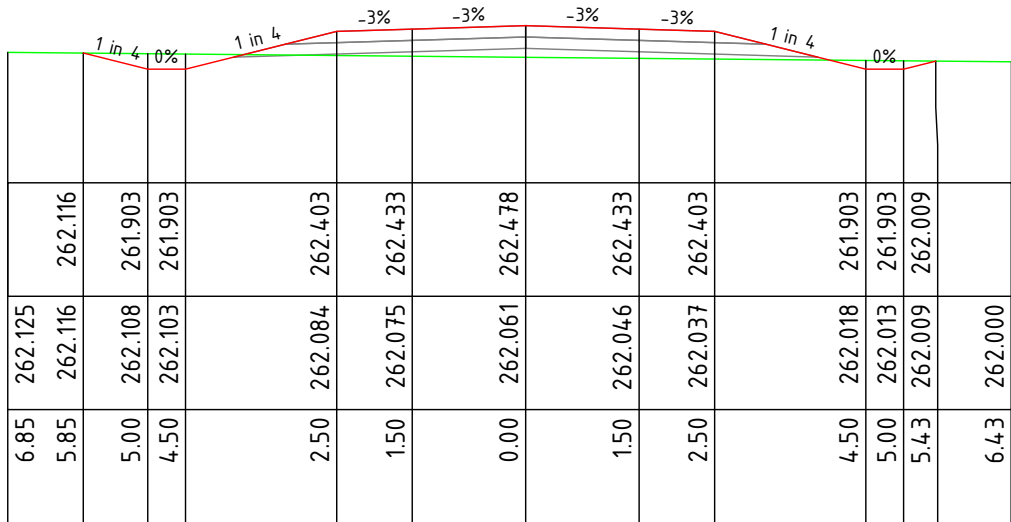
CH 100.00

RL260.4m




CH 60.00

RL260.4m

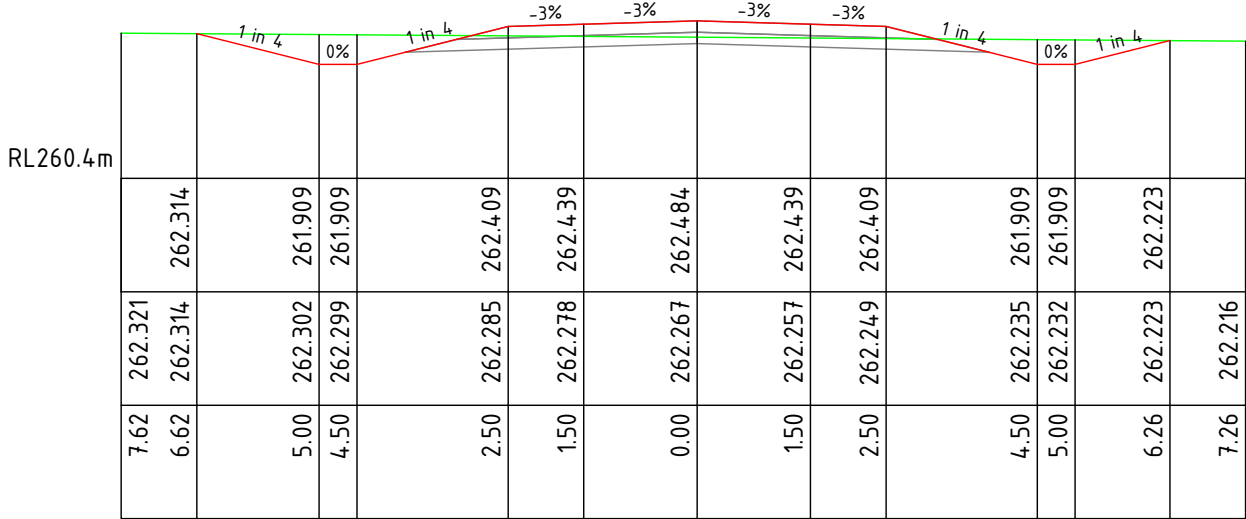


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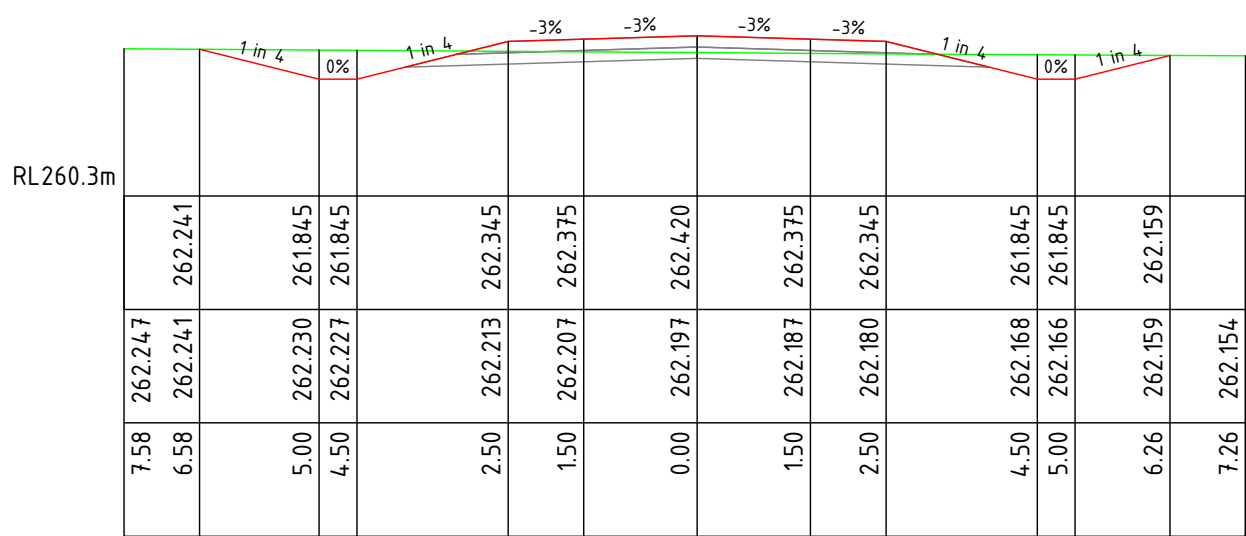
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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:		D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: ROAD 3 CROSS SECTIONS (60 - 106.26)	
		C	FOR COUNCIL APPROVAL	22-06-23					
		B	FOR REVIEW	19-06-23					
		A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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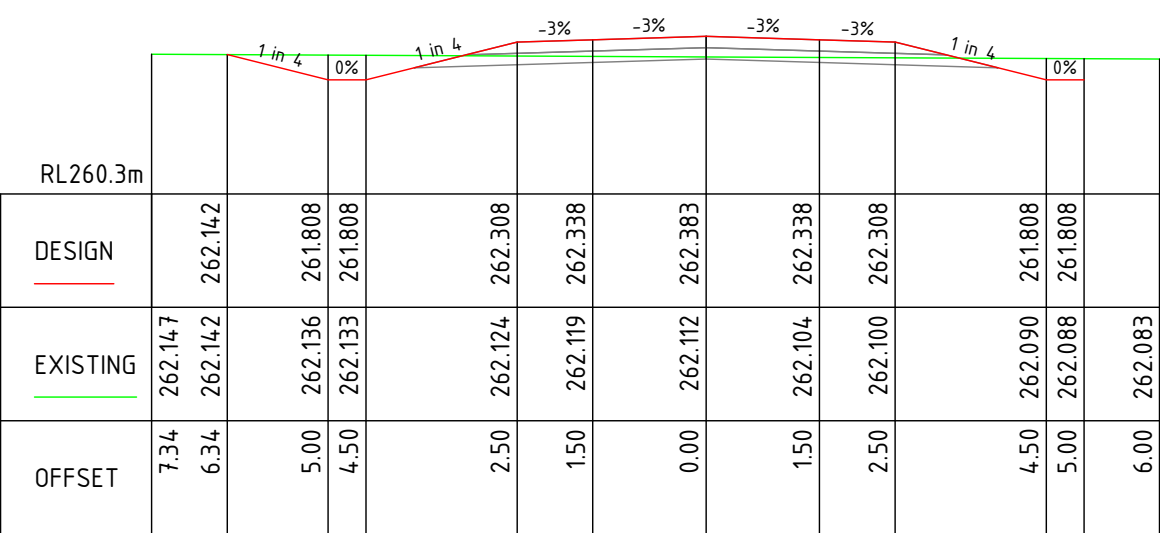
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CH 127.44



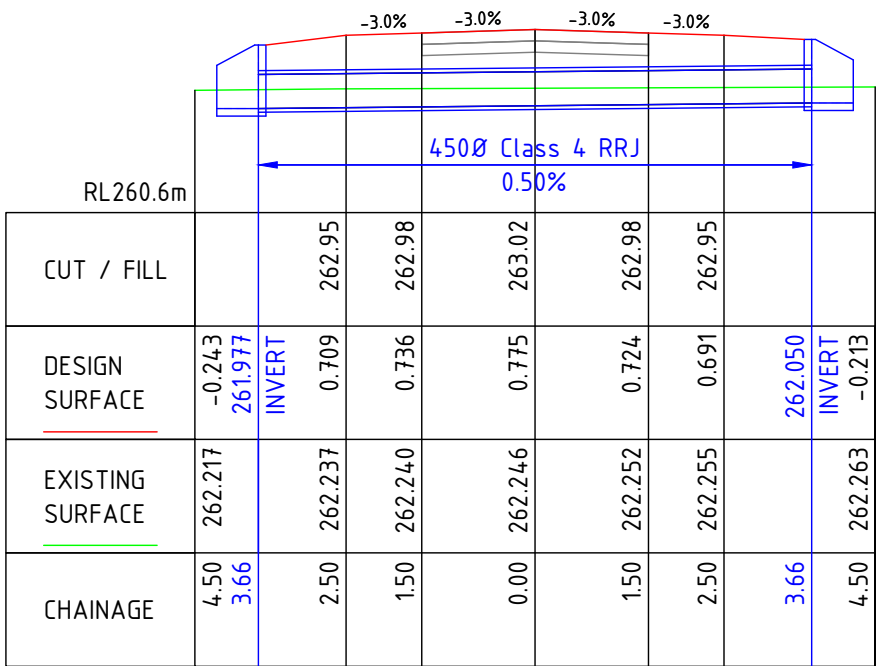
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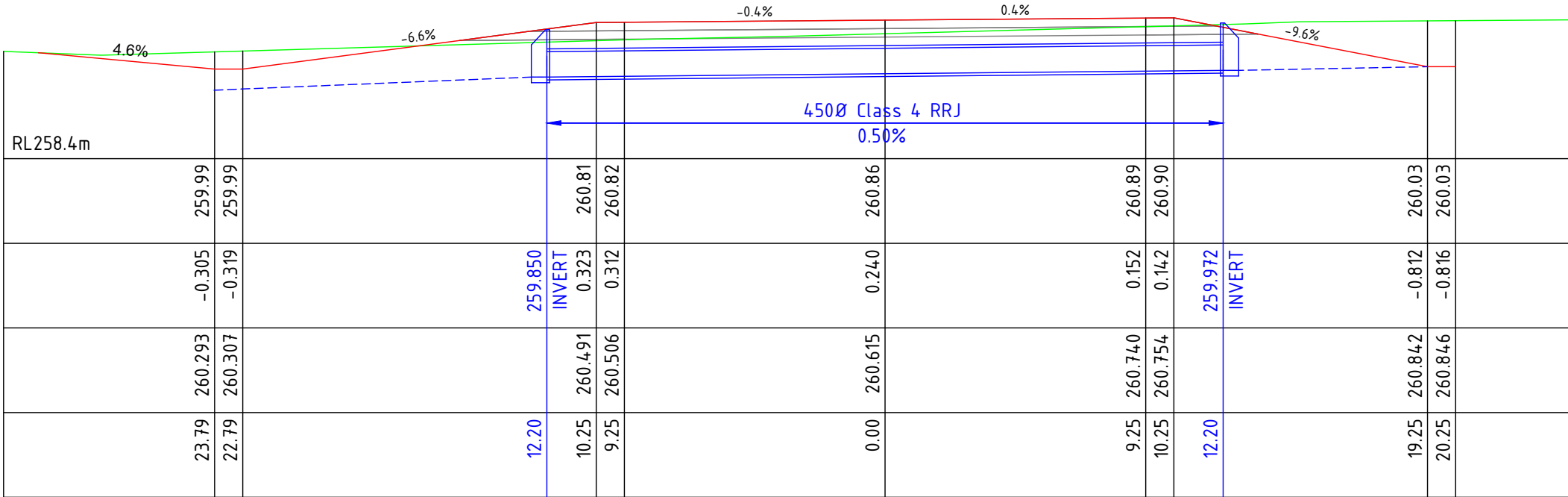
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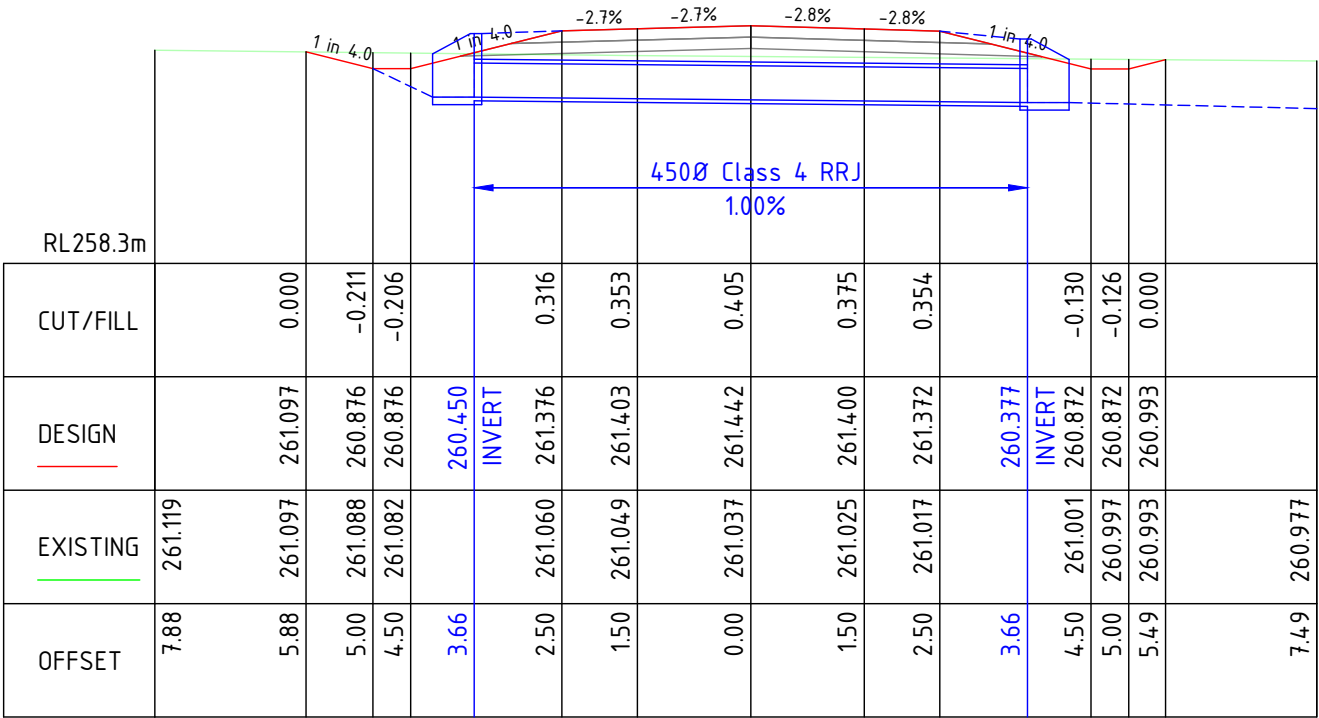
CULVERT No.1 (ROAD 2 - CH34.7)



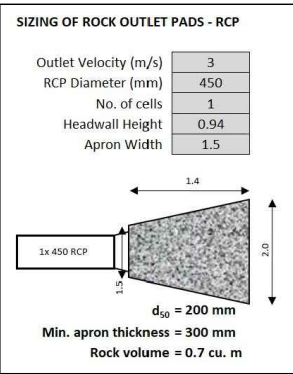
CULVERT No.2 (ROAD 1 - CH 8.06)  
SCALES: HORIZONTAL 1:200 VERTICAL 1:100

EXTENSION OF EXISTING 450mm RCP (GRADE UNKNOWN)  
EXTEND WITH 450mm RCP (CL4 RRJ)  
BY 7.32m TO NORTH AT 1% GRADE.


CULVERT No.3



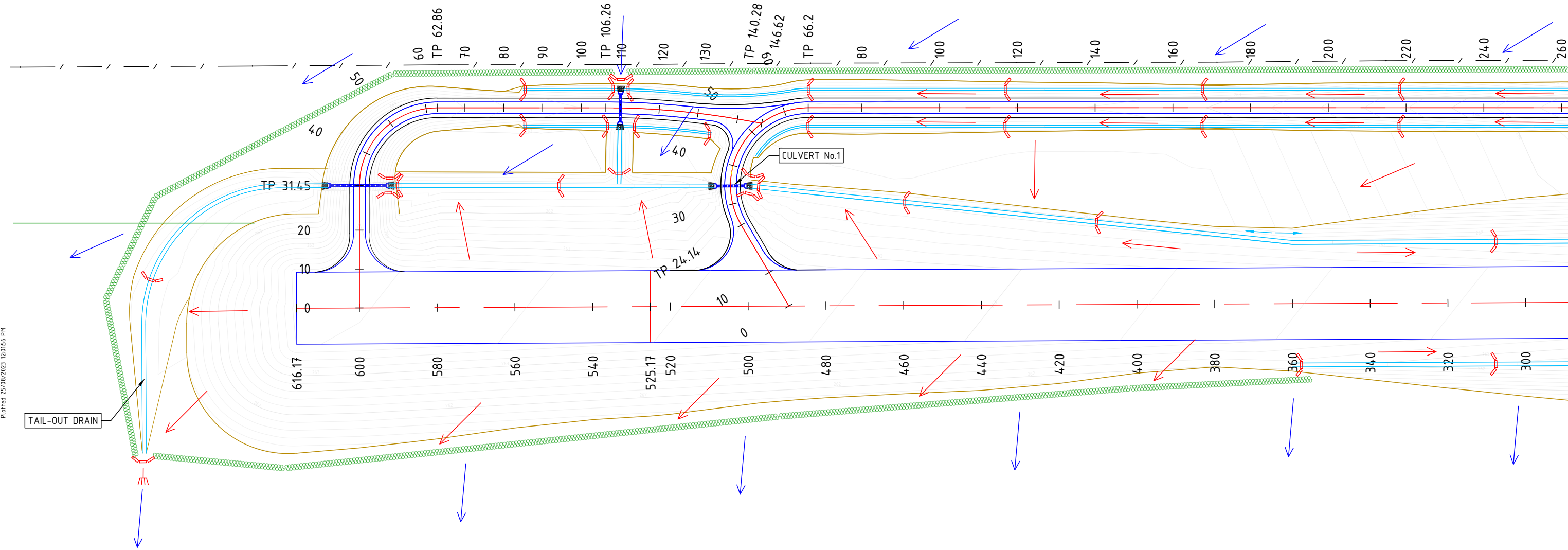
CULVERT No.4 (ROAD 2 - CH 541.76)








450mm RCP

SCALE: 1:100 OR AS SHOWN		REV.	DESCRIPTION	DATE	 REGIONAL engineering SERVICES  REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404	CLIENT:  BLAND SHIRE COUNCIL	PROJECT:  WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 30	SHEETS 35
DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE: CULVERTS 1, 2 & 4 PROFILES					
	C	FOR COUNCIL APPROVAL	22-06-23						
	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						






LEGEND:

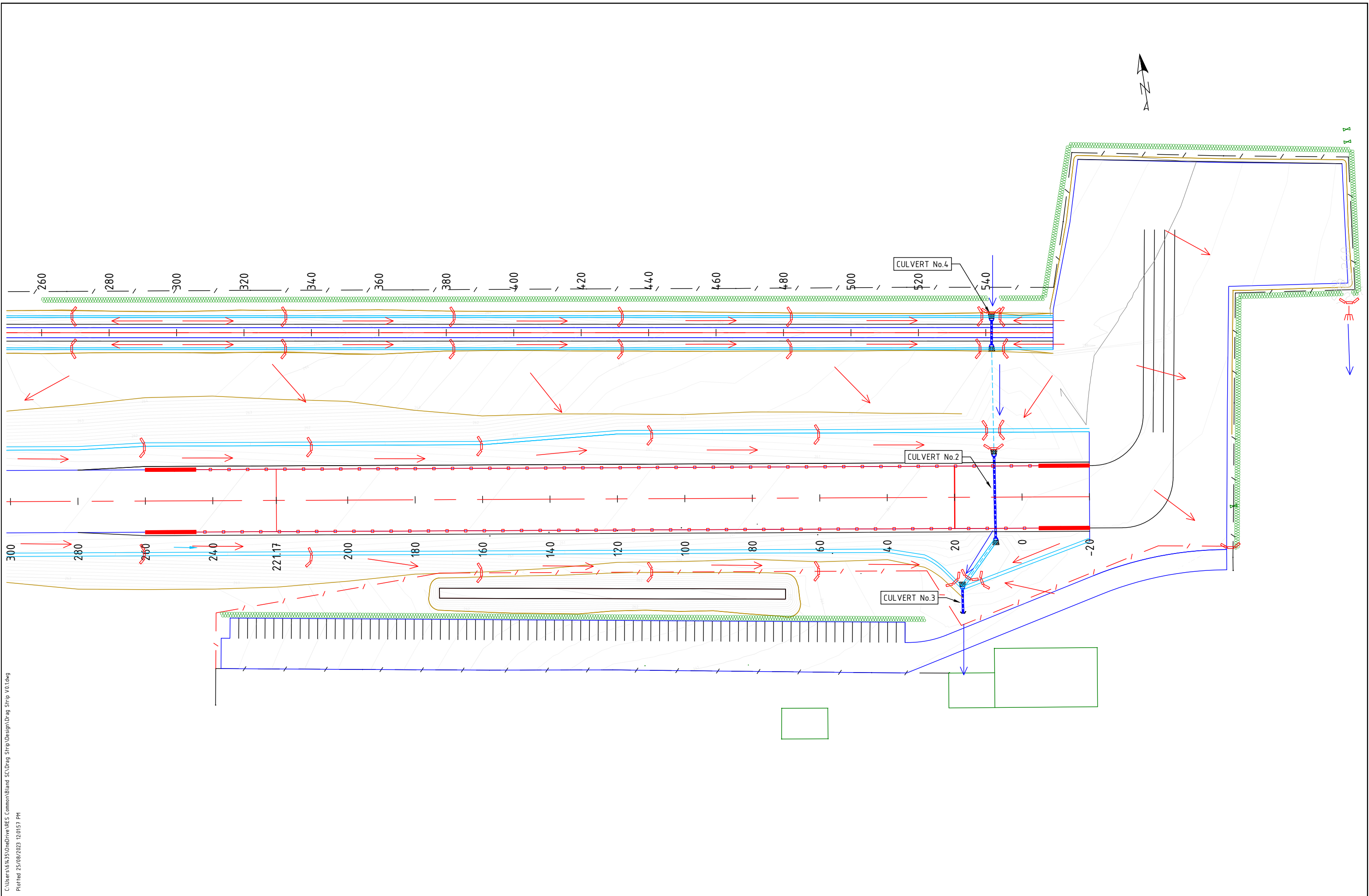
-  'CLEAN' WATER FLOW LINE
-  INDICATIVE 'DIRTY' WATER
-  EARTH BUND
-  STRAW BALES / COCONUT FIBRE LOGS
-  LEVEL SPREADER

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SCALE: 1:1000 DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	REV.	DESCRIPTION	DATE	 REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404	CLIENT: BLAND SHIRE COUNCIL	PROJECT: WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 32	SHEETS 35
	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: EROSION & SEDIMENT CONTROL PLAN (WEST)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					





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SCALE: 1:1000  DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	REV.	DESCRIPTION	DATE	 REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404	CLIENT:  BLAND SHIRE COUNCIL	PROJECT:  WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 33	SHEETS 35
	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: EROSION & SEDIMENT CONTROL PLAN (EAST)	
	C	FOR COUNCIL APPROVAL	22-06-23					
	B	FOR REVIEW	19-06-23					
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					

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1. GENERAL INSTRUCTIONS

1.1 LEGAL RESPONSIBILITIES

- (i) NOTE THAT SD REFERS TO STANDARD DRAWINGS.
- (j) CONTRACTORS WILL ENSURE THAT ALL SOIL AND WATER MANAGEMENT WORKS ARE UNDERTAKEN AS INSTRUCTED IN THIS SPECIFICATION AND CONSTRUCTED FOLLOWING THE GUIDELINES STATED IN LANDCOM, 2004 (THE BLUE BOOK).
- (iii) ALL SUBCONTRACTORS WILL BE INFORMED OF THEIR RESPONSIBILITIES IN MINIMISING THE POTENTIAL FOR SOIL EROSION AND POLLUTION TO DOWNSLOPE AREAS.

1.2 EROSION CONTROL

- (i) SITE DISTURBANCE WILL BE LIMITED IN EXTENT AND NATURE TO THAT DESCRIBED IN THIS SWMP.
- (ii) VEHICULAR ACCESS TO THE SITE WILL BE RESTRICTED TO THOSE VEHICLES THAT ARE ESSENTIAL FOR EFFICIENT CONSTRUCTION.
- (iii) THE SOIL EROSION HAZARD ON THE SITE WILL BE KEPT AS LOW AS PRACTICABLE BY MINIMISING LAND DISTURBANCE.
- (iv) LIMIT DISTURBANCE TO THAT ESSENTIAL FOR WORKS BEING UNDERTAKEN AT ANY GIVEN TIME.

1.3 SEDIMENT CONTROL

- (i) SEDIMENT FENCING (SD 6-8) WILL BE INSTALLED AS SHOWN OR ELSEWHERE AT THE DISCRETION OF THE SITE MANAGER.
- (ii) SEDIMENT REMOVED FROM ANY TRAPPING DEVICE WILL BE DISPOSED IN LOCATIONS WHERE FURTHER EROSION AND CONSEQUENT POLLUTION TO DOWNSLOPE LANDS AND WATERWAYS WILL NOT OCCUR.
- (iii) RECEPTACLES WILL BE PROVIDED FOR WASTE MATERIALS AND LITTER.
- (iv) SAFE STORAGE AREAS WILL BE PROVIDED FOR FUELS, OILS, FERTILISERS, CHEMICALS AND OTHER HAZARDOUS MATERIALS AS REQUIRED.
- (v) NOTE THAT A SEDIMENT BASIN IS NOT REQUIRED AT THIS SITE.

2.2 STAGE 2 - CONSTRUCTION / INSTALLATION

- (i) STRIP AND STOCKPILE TOPSOIL FROM THE ROAD CORRIDOR. STOCKPILES ARE TO BE LOCATED AS SHOWN AND ARE TO BE CONSTRUCTED AS PER SD 4-1.
- (ii) CONDUCT EARTHWORKS AND ROAD CONSTRUCTION ACTIVITIES AND CONSTRUCT THE LONG-TERM ROAD DRAINAGE.
- (iii) UNDERTAKE ONGOING MONITORING AND MAINTENANCE OF EROSION AND SEDIMENT CONTROL WORKS AS DETAILED.

2.3 STAGE 3 - SITE REHABILITATION

- (i) AS WORKS ARE COMPLETED, PROGRESSIVELY STABILISE EXPOSED SOILS
- (ii) REMOVE TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES ONLY AFTER PERMANENT VEGETATIVE STABILISATION IS COMPLETE.
- (iii) UNDERTAKE ONGOING MONITORING AND MAINTENANCE OF REVEGETATION AND STABILISATION WORKS AS DETAILED BELOW.


3. STABILISATION

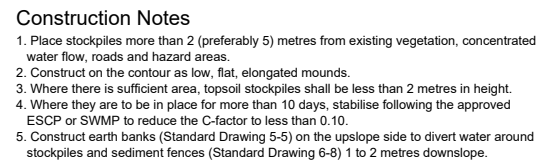
- (i) AREAS OF CONCENTRATED FLOW (DRAINAGE DEPRESSIONS AND DIVERSION DRAINS) WILL BE STABILISED BEFORE THEY CARRY ANY FLOW.
- (ii) DIVERSION DRAINS ARE TO BE LINED WITH JUTE OR COCONUT FIBRE MATTING IMMEDIATELY FOLLOWING CONSTRUCTION.
- (iii) FOLLOWING THE COMPLETION OF EARTHWORKS AND ROAD CONSTRUCTION, AREAS OF EXPOSED SOILS ARE TO BE LEFT IN A ROUGH CONDITION TO FACILITATE KEYING-IN OF TOP SOIL.
- (iv) REPLACE TOPSOIL OVER EXPOSED SOIL AREAS TO A DEPTH OF AT LEAST 75 MM (NB REDUCE THIS TO 50 MM WHERE BATTERS EXCEED 1(V) TO 2(H) (SD 4-2).
- (v) MONITOR VEGETATION AREAS AND WATER THEM AS REQUIRED.
- (w)

4. SITE MONITORING AND MAINTENANCE

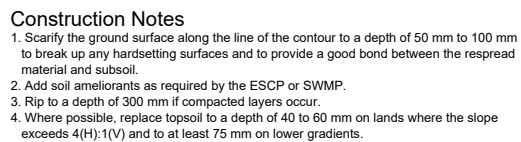
- (i) SITE MONITORING WILL BE UNDERTAKEN DURING AND AFTER CONSTRUCTION TO ENSURE THAT:
  - THIS PLAN IS BEING ADHERED TO; AND
  - REVEGETATION ACTIVITIES ARE SUCCESSFUL.
- (ii) WASTE RECEPTACLES WILL BE EMPTIED AS NECESSARY.
- (iii) A SELF-AUDITING PROGRAM WILL BE INITIATED FOR THE SITE. THIS SITE SUPERINTENDENT WILL INSPECT THE SITE AT LEAST WEEKLY AND FOLLOWING ANY RAIN EVENTS EXCEEDING 2 MM. AN INSPECTION LOG WILL BE MAINTAINED. INSPECTIONS WILL PAY PARTICULAR ATTENTION TO:
  - REMOVAL OF SPILLED SAND OR OTHER MATERIALS FROM NEAR HAZARD AREAS.
  - ENSURING BARRIER FENCING IS MAINTAINED AND EXCLUSION ZONES ARE BEING OBSERVED BY ALL SITE WORKERS AND CONTRACTORS.
  - ENSURING PROGRESSIVE AND PROMPT REHABILITATION OF LANDS, THAT REHABILITATION HAS EFFECTIVELY REDUCED THE EROSION HAZARD AND UPGRADING OR REPAIR AS APPROPRIATE.

- CONSTRUCTING ADDITIONAL EROSION AND / OR SEDIMENT CONTROL WORKS AS MIGHT BECOME NECESSARY TO ENSURE THE DESIRED WATER CONTROL IS ACHIEVED, I.E. MAKE ONGOING CHANGES TO THE SWMP.
- MAINTAINING EROSION AND SEDIMENT CONTROL MEASURES IN A FUNCTIONING CONDITION UNTIL ALL EARTHWORK ACTIVITIES ARE COMPLETED AND THE SITE IS REHABILITATED.
- REMOVAL OF TRAPPED SEDIMENT AND DISPOSAL TO SAFE AREAS.
- REMOVAL OF TEMPORARY SOIL CONSERVATION STRUCTURES AS THE LAST ACTIVITY IN THE REHABILITATION PROGRAM.
- (iv) A RAIN GAUGE WILL BE INSTALLED AT THE SITE AND WILL BE MONITORED DAILY BY THE SITE SUPERINTENDENT TO DETERMINE THE SEVERITY OF ANY RAIN EVENTS.
- (v) REVEGETATION AREAS WILL BE INSPECTED REGULARLY TO INVESTIGATE FAILURES AND PROGRAM NECESSARY REPLANTING OR RESPRAYING AS NECESSARY.
- (vi) AN ADEQUATE WATERING AND FERTILISING SYSTEM WILL BE MAINTAINED IN REVEGETATION AREAS.
- (vii) AREAS OF LOCALISED SOIL EROSION WILL BE IDENTIFIED AND APPROPRIATE PREVENTIVE MEASURES IMLEMENTED. THESE MIGHT INCLUDE:
  - PLANTING ADDITIONAL STABILISING VEGETATION.
  - STABILISING SOILS WITH MULCHES OR ALTERNATIVE SOIL BINDERS.
  - TAKING STEPS TO MINIMISE ANY CONCENTRATED STORMWATER FLOWS.
- (viii) ANY AREAS OF LOCALISED POOR DRAINAGE WILL BE IDENTIFIED AND APPROPRIATE REMEDIAL ACTION TAKEN. THIS MIGHT INCLUDE:
  - INSTALLING FORMALISED DRAINAGE CHANNELS OR PIPES.
  - IMPROVING SOIL PERMEABILITY BY CULTIVATING THE SOIL SURFACE.
  - IMPROVING SOIL PERMEABILITY BY INSTALLING INFILTRATION TRENCHES.
  - PLANTING MOISTURE TOLERANT VEGETATION IN PROBLEM AREAS.

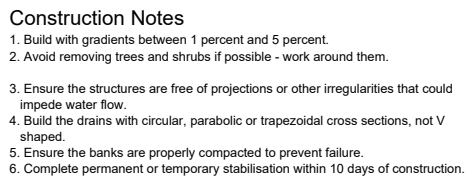
SCALE:	REV.	DESCRIPTION	DATE	 REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404	CLIENT:  BLAND SHIRE COUNCIL	PROJECT:  WESTERN WHEELERS MOTORSPORT PARK	SHEET No. 34	SHEETS 35
	D	AMENDED FOR COUNCIL APPROVAL	25-08-23				SHEET TITLE: EROSION & SEDIMENT CONTROL NOTES	
DATUM: AHD	C	FOR COUNCIL APPROVAL	22-06-23					
PROJECTION: GDA2020 (ZONE 55)	B	FOR REVIEW	19-06-23					
SURVEYED BY:	A	DRAFT - FOR COUNCIL COMMENT	17-04-23					



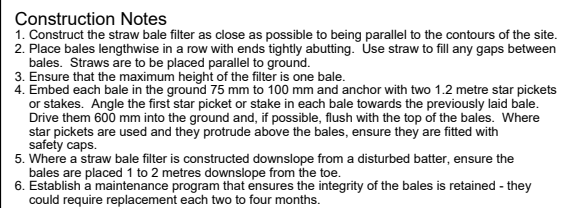
SD 4-1




SD 4-2



SD 5-5



SD 6-7

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DATUM: AHD PROJECTION: GDA2020 (ZONE 55) SURVEYED BY:	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SHEET TITLE: <b>EROSION &amp; SEDIMENT CONTROL DETAILS</b>					
	C	FOR COUNCIL APPROVAL	22-06-23						
	B	FOR REVIEW	19-06-23						
	A	DRAFT - FOR COUNCIL COMMENT	17-04-23						