

Proposed Western Wheelers Motorsport Park | West Wyalong Airport, Bland Shire

Parking & Traffic Impact Assessment Report

P2101

Prepared for Bland Shire Council

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1 Introduction

1.1 Background

Bland Shire Council has engaged Greys Consulting to prepare a Traffic Impact Assessment (TIA) to support a development application for the proposed Western Wheelers Motorsport Park, located adjacent to the West Wyalong Airport in Wyalong, NSW.

The development site is located directly north of the existing airport facility. Access will be via Airport Road, which intersects with the Newell Highway, a classified state road that serves as a vital freight and transport route through the region. The area is strategically positioned to take advantage of the accessibility provided by the surrounding regional road network while supporting tourism, motorsport events, and club activities.

The proposal comprises constructing a new car racing track facility and associated infrastructure over an area of approximately 90,618 m². The development will include a sealed racing surface, return road, supporting facilities for club operations, vehicle access, and parking.

The proposed development is located entirely within Lot A in DP 220617 and is aligned with strategic regional objectives to stimulate economic activity through tourism, motorsports, and regional events.

This TIA assesses existing and future traffic conditions, parking demand, access arrangements, and safety considerations to support informed decision-making and ensure that the development integrates safely and efficiently with the surrounding road network.



The boundaries of the proposed development are highlighted in Figure 1-1.

Source: Sixmaps
Figure 1-1 Subject Site Location



1.2 Report Objectives

This TIA aims to evaluate the traffic and transport implications of the proposed Western Wheelers Motorsport Park development adjacent to West Wyalong Airport. The assessment has been undertaken to accompany a Development Application (DA) submitted to Bland Shire Council and addresses the following key aims:

- To assess existing traffic conditions on the surrounding road network, particularly along the Newell Highway and Airport Road, based on available traffic data and observed access characteristics.
- To project future traffic generation associated with the proposed motorsport facility, considering expected visitor numbers, event operations, and access modes.
- To assess the adequacy of existing access arrangements and recommend improvements where necessary to accommodate additional traffic volumes safely and efficiently.
- To identify appropriate intersection treatments at the Airport Road / Newell Highway junction to support anticipated traffic movements associated with event and non-event days.
- To evaluate the proposed access driveway location, internal circulation layout, and parking provisions for compliance with relevant Australian Standards (AS2890 series), Austroads guidelines, and Council planning controls.
- To assess active and public transport accessibility and identify whether additional infrastructure or wayfinding improvements are warranted.
- To ensure that access for service and emergency vehicles, including waste collection and delivery movements, is feasible and safe.
- To undertake high-level SIDRA intersection modelling where appropriate to quantify traffic performance under base case, project case, and mitigation scenarios.
- To provide a clear summary of findings and offer evidence-based recommendations to inform Council's decision-making on the proposed development.

1.3 Reference Documents

The following documents and guidelines have been reviewed and referenced in the preparation of this report:

- Bland Shire Council Development Control Plan (DCP) 2012
- Bland Local Environmental Plan (LEP) 2011
- Guide to Traffic Impact Assessment (TfNSW, 2024)
- Austroads Guide to Road Design Part 4A: Unsignalised and Signalised Intersections
- AS 2890.1:2004 Off-street Car Parking
- AS 2890.2:2018 Off-street Commercial Vehicle Facilities
- AS 2890.6:2022 Off-street Parking for People with Disabilities



2 Existing Conditions

2.1 Existing Development

The subject site is immediately east of the West Wyalong Airport and comprises vacant land with no existing buildings or formalised infrastructure. The area is generally flat, cleared, and free from significant vegetation, making it well-suited for development without requiring substantial earthworks. The site directly adjoins Airport Road and is approximately 150 metres south of the Newell Highway, providing efficient regional and state road network access.

The site is zoned SP1 – Special Activities (Airport) under the Bland Local Environmental Plan 2011, which permits uses associated with airport operations and ancillary development, subject to development consent. The proposed motorsport use is consistent with the intent of this zoning, which supports aviation-adjacent land uses that do not conflict with airport safety or operations. Surrounding land uses include:

- SP1 (Special Activities Airport) to the south (active airport runway and terminal area),
- RU1 (Primary Production) or RU4 (Rural Small Holdings) to the west and north,
- RE1 (Public Recreation) or IN1 (General Industrial) further east along Airport Road.

There are no sensitive residential areas in the immediate vicinity of the proposed development. The nearest residential zones (R5 – Large Lot Residential) are located more than 1 km from the site boundary, which limits the potential for amenity impacts due to traffic or noise.

Vehicle access is available via Airport Road, which intersects with Newell Highway at a priority-controlled Tintersection. This connection forms the key transport link between the site and broader regional centres. The site and its surrounding landuse are depicted in Figure 2-1.



Source: Hawkesbury Council LEP 2012 Maps (last updated 12/02/2025)

Figure 2-1 Study Area Land Use Plan



2.2 Surrounding Road Network

2.2.1 Key Roads

The proposed development is located adjacent to Airport Road, which intersects with the Newell Highway. These roads form the primary access and distribution routes for traffic generated by the proposed motorsport facility. Key characteristics of the road network are summarised in Table 2.1.

Table 2-1	Bus Services Operating Near the Site
	Bus bei nees operating near the one

Road Name	Jurisdiction	Hierarchy	No. Lanes	Divided	Speed Limit	Comments
Newell Highway	TfNSW	Arterial State Road	2	No	110 km/h	Key regional freight corridor
Airport Road	Bland Shire Council	Local Road	2	No	50 km/h	Access to airport and development site

The main traffic corridors in the vicinity of the subject site is Newell Highway, which is classified as statecontrolled roads. Transport for NSW has authority over these roads.

2.2.2 <u>Newell Highway</u>

Newell Highway (A39) is a principal north–south state highway and forms part of the national highway network. In the vicinity of the site, it operates as a two-lane undivided arterial road with a posted speed limit of 110 km/h adjacent to the site. Newell Highway is under the jurisdiction of Transport for NSW (TfNSW), serving as a key north–south freight and regional travel corridor. It provides regional connectivity across western New South Wales and is one of the state's primary inland freight routes. It accommodates significant volumes of freight and long-distance traffic and plays a strategic role in regional connectivity between the Riverina, Central West, and Northern Inland regions of NSW.

2.2.3 <u>Airport Road</u>

Airport Road is a sealed local road providing direct vehicular access to the West Wyalong Airport and adjacent rural land uses. It connects to the Newell Highway at a T-intersection, which is currently unsignalised and priority-controlled. The road is lightly trafficked under current conditions but is expected to accommodate increased volumes during events at the proposed motorsport park.

Airport Road is a local access road managed by Bland Shire Council. It connects the airport precinct to the West Wyalong township and local industrial areas. It is a two-lane undivided road providing access to the airport terminal and aviation support facilities. The road operates under default urban speed limits and is not currently signalised, and CHR(S) and BAL treatments are at its intersection with Newell Highway.

2.3 Existing Traffic Controls

The following traffic controls and infrastructure are relevant to the operation and capacity of the surrounding road network:

- A posted speed limit of 110 km/h applies to Newell Highway at the intersection with Airport Road.
- A sealed, priority-controlled T-intersection is present at the Newell Highway / Airport Road intersection.
- The intersection includes an existing right-turn lane treatment (CHR(S)) and a Basic Left Turn treatment (BAL) on Newell Highway for northbound traffic turning right into Airport Road, providing separation from through vehicles.
- No formal left-turn deceleration lane exists for vehicles turning left into Airport Road from the north.
- These controls inform the assessment of site access safety, intersection performance, and any necessary upgrades to accommodate the proposed development.



3 Proposed Development

3.1 Development Description

The proposal involves the establishment of a motorsport facility adjacent to the existing West Wyalong Airport, within land under the management of Bland Shire Council. The Western Wheelers Car Club proposes to develop a sealed racing track, associated support infrastructure, and access roads over a site area of approximately 90,618 square metres. The facility is intended to support club-level car racing, driver education programs, community motorsport events, and related activities.

The site is situated directly east of the airport runway and south of the Newell Highway, with vehicular access provided via Airport Road. The existing topography is relatively flat and cleared of significant vegetation, making the site well-suited for development with minimal earthworks. No existing structures are located within the proposed development area.

The key components of the proposed development include:

- A sealed racing track with a return lane
- Dedicated entry and exit driveways connected to Airport Road
- Informal parking and staging areas for vehicles and spectators
- Event control and support areas, to be delivered in future stages

The facility is expected to operate on a flexible schedule with activity concentrated on weekends and during organised events. It is not intended to operate as a daily commercial attraction and will primarily be used by club members and authorised participants. Anticipated traffic volumes will vary depending on the nature of the event, with peak volumes expected during scheduled racing events and community open days.

3.2 Access Arrangements

Vehicle access to the site will be facilitated via a new connection to Airport Road. The proposed access location is situated approximately 150 metres south of the intersection with Newell Highway, providing direct connectivity to the regional road network. The access is proposed as a two-way sealed driveway, designed to accommodate light vehicles, trailers, and service vehicles.

The Newell Highway / Airport Road intersection is currently configured as a priority-controlled T-intersection. The major road provides for two-way through traffic movements and includes an existing right-turn treatment for northbound vehicles turning into Airport Road. A basic left-turn treatment exists on the southbound approach. Given the nature and scale of the proposed development, the capacity and functionality of this intersection are reviewed in Section 5 of this report.

No internal roads are formally proposed at this stage; however, the internal layout includes provision for vehicle manoeuvring, informal parking areas, and access to track entry points. The access geometry, internal swept paths, and turning facilities for service vehicles have been reviewed against AS2890.2 and AS2890.1 and are discussed further in Section 6.

3.3 Parking Provision

Parking will be provided within the site in an informal format to accommodate spectators, participants, staff, and service vehicles. Given the unique land use and absence of specific parking rates within the TfNSW Guide to Traffic Impact Assessment or the Bland Shire DCP, parking demand has been assessed using first-principles methodology. Parking areas will be delineated using surface markings or temporary fencing during events, with sufficient area allocated for overflow or future formalisation.

The final parking demand and provision strategy is addressed in Section 7 of this report.

3.4 **Operational Characteristics**

The facility is intended for use during daylight hours, with no lighting infrastructure proposed. Noise management and event scheduling will be addressed through a separate environmental assessment.



Vehicle access will be managed via existing airport gates, with the potential for dedicated entry control during events. On-site traffic movement will be managed to avoid conflicts between users, and sufficient sight distance is available at the site access point to accommodate safe entry and exit under rural speed conditions.

Event management and traffic control measures will be implemented where required during peak usage periods, and these measures are further considered in Section 9.

The concept layout and locality of the proposed motorsport facility is illustrated in Figure 3-1.





Source: PLAN LAYOUT - WESTERN WHEELERS MOTORSPORT PARK

Figure 3-1 Study Area Land Use Plan



4 Traffic Generation and Distribution

4.1 Traffic Generation Methodology

There are no prescribed traffic generation rates for motorsport parks or club-based racing facilities within the Transport for NSW (TfNSW) Guide to Traffic Impact Assessment (2024), the RMS Guide to Traffic Generating Developments (2002), or the Bland Shire Council DCP. In the absence of empirical trip generation benchmarks for this specific land use, a first-principles approach has been adopted. This method draws upon land use characteristics, expected operational conditions, and comparisons with similar facilities located in regional New South Wales and Victoria.

The Western Wheelers Motorsport Park is proposed to function primarily as a club-operated venue, hosting low-frequency events such as race meetings, driver education days, and community motorsport exhibitions. The proponent has not provided a detailed operational statement or event schedule at this stage; therefore, estimates of peak demand have been formulated using analogous land uses and site observations at comparable motorsport facilities.

Based on review of publicly available material, transport assessments, and operator information for similarscale motorsport sites in regional settings, the following typical event attendance characteristics have been noted:

- The *Gunnedah Speedway* (NSW), a regional circuit with similar access constraints, accommodates approximately 60–80 vehicles during regular race events, based on typical pit usage, participant registration counts, and spectator vehicle volumes.
- The *Wodonga Motorsport Complex* (VIC), which hosts club-level time trials and training days, regularly generates around 90 vehicles during peak weekend events, including participants, trailers, support crew, and local visitors.
- The *Wagga Motorsports 1/8 Mile Strip*, used for grassroots racing, has been observed to attract up to 100 vehicles per event, especially during advertised seasonal meets, based on data published in local event logistics and available photographs of queue lengths and pit occupancy.
- Smaller facilities such as *Queanbeyan's Fairbairn Park* and *Ararat Hillclimb* generate in the order of 40–70 vehicles depending on event type and track capacity.

These figures have been used as a reference to establish the peak trip generation range for the Western Wheelers Motorsport Park. The 60–100 vehicle range adopted in this assessment is considered reasonable and conservative, reflecting the maximum number of vehicles expected during a typical peak weekend event at a regional motorsport facility of this scale and nature. This range includes:

- Participant vehicles (driver and crew transport vehicles)
- Trailers towing competition vehicles
- Spectator vehicles (including families or small groups)
- Event marshals and limited support staff

No formal public grandstand facilities or high-capacity spectator infrastructure are proposed, which places the development well below the scale of commercial racing venues or motorsport precincts with significant public draw.

4.2 Assessment Periods

Based on the above operational profile, the following peak periods have been adopted for the purpose of this traffic impact assessment:

- AM Peak Hour: 8:00 am to 9:00 am (event arrival period),
- PM Peak Hour: 4:00 pm to 5:00 pm (event departure period).



These periods align with typical event scheduling and ensure the assessment captures the highest traffic demand associated with the proposed development. They are not expected to coincide with weekday commuter peak periods on the surrounding road network.

4.3 Event Arrival and Departure Profile

Without fixed operating hours, it is assumed that events will generally be scheduled between 9:00 am and 4:00 pm on weekends. Based on typical participant arrival behaviour and observed access profiles at similar venues, it is estimated that approximately 60 per cent of all vehicles would arrive during the peak one-hour period prior to event commencement, and a similar proportion would depart during the peak one-hour period following event completion.

Accordingly, the following peak hourly traffic generation rates have been adopted for modelling purposes:

- 100 vehicles (maximum total estimate) x 0.6 = 60 vehicle trips in the AM peak hour
- 100 vehicles x 0.6 = 60 vehicle trips in the PM peak hour

These values represent a conservative worst-case scenario, assuming minimal car-pooling and no staged entry or egress strategies. They have been used as input for SIDRA intersection modelling and network performance assessment, which are presented in Section 5.

4.4 Non-Event Conditions

Outside of scheduled events, the facility is expected to generate minimal daily vehicle movements, which are limited to maintenance vehicles, individual practice runs, or administrative access. It is estimated that nonevent periods would generate fewer than 10 vehicle trips per day, well below thresholds requiring impact assessment under TfNSW or Austroads guidelines.

The adopted approach ensures that the impact analysis covers both the upper-bound demand scenario and recognises the low-intensity baseline use of the facility on non-event days.

4.5 Trip Distribution and Assignment

Trip distribution for the development has been determined based on the regional road hierarchy and the origin–destination patterns of participants travelling from surrounding townships and regional centres. Given the location of the site immediately south of the Newell Highway, it is expected that the majority of development traffic will approach from and return to the highway corridor.

Based on regional access assumptions, the directional distribution of traffic has been estimated as follows:

- 60 per cent of vehicles will travel to/from the north via Newell Highway, including West Wyalong, Forbes, Parkes, and other central NSW towns
- 40 per cent of vehicles will travel to/from the south via Newell Highway, including Narrandera, Wagga Wagga, and southern regions

All development-related trips will enter and exit via Airport Road. Vehicles accessing Newell Highway will do so via the existing priority-controlled T-intersection, including a short auxiliary right-turn lane for northbound traffic entering Airport Road. No left-turn auxiliary treatment exists for southbound traffic.

These assumptions have been used to assign development traffic in the SIDRA modelling scenarios, with distribution applied proportionally to each approach leg of the Newell Highway / Airport Road intersection.



5 SIDRA modelling and Intersection Performance

5.1 Assessment Approach

The operational performance of the Newell Highway / Airport Road intersection has been assessed under three network scenarios using SIDRA Intersection 10.0:

- Base Case: existing background traffic conditions with no development traffic
- **Project Case**: Base Case with traffic generated by the proposed motorsport development
- **Project Case with Mitigation**: Project Case with consideration of auxiliary turn treatments on Newell Highway

The intersection of Newell Highway and Airport Road forms the only vehicular access to the site. The analysis focused on the AM arrival and PM departure peak hours, assessing Degree of Saturation (DoS), average delay, Level of Service (LoS), and 95th percentile queue lengths for all movements.

All models included 30% heavy vehicle proportions for development-generated movements to account for trailers and support vehicles associated with the motorsport park.

5.2 Network Geometry and Modelling Inputs

The Newell Highway is configured as a two-lane, two-way rural arterial road with a posted speed limit of 110 km/h. Airport Road intersects from the east at a T-junction, also configured as a two-lane road with a posted speed limit of 60 km/h. The intersection geometry was coded to reflect the following characteristics:

- Two-lane two-way operation on Newell Highway (110 km/h)
- A short auxiliary right-turn lane (CHR(S)) for northbound vehicles on Newell Highway entering Airport Road
- No auxiliary left-turn treatment for southbound vehicles on Newell Highway
- A single approach lane for all movements on Airport Road

In the absence of site-specific survey data, existing traffic volumes were sourced from the Transport for NSW Traffic Volume Viewer (Site 6142 – 460 m east of Nicholson Lane, Wyalong) 2024 data, as the most proximate and reliable available data. The AM and PM peak hour volumes were used in the SIDRA model. The heavy vehicle percentages were extracted from the same dataset. The following Weekends (Saturday/Sunday) through traffic volumes were adopted for the Newell Highway:

Table 5-1 Base Case Peak Hour Input Volumes and Vehicle Class Breakdown

Movement	Total (veh/h)	Light Vehicles	Heavy Vehicles	HV %
Newell Hwy NB Through	48	33	15	31%
Newell Hwy NB Right Turn to Airport Rd	10	7	3	30%
Newell Hwy SB Through	49	34	15	31%
Newell Hwy SB Left Turn to Airport Rd	18	13	5	28%
Airport Rd Left Turn to Newell Hwy NB	15	12	3	20%
Airport Rd Right Turn to Newell Hwy SB	20	15	5	25%

The SIDRA model was configured using these inputs, applying separate vehicle classes per movement and assigning appropriate turning treatments. This configuration accurately reflects the operational context of the Newell Highway / Airport Road intersection during weekday AM and PM peak periods.

These volumes are considered appropriate for assessing an unsignalised rural intersection with limited turning traffic and low background growth. The intersection layout coded in Sidra is depicted in Figure 5-1.



Figure 5-1 Newell Highway/Airport Road Intersection Layout

5.3 Intersection Geometry and Model Configuration

The Newell Highway is a two-lane rural arterial with a posted speed limit of 110 km/h. Airport Road is a twolane local access road with a 60 km/h posted speed. The existing intersection is a T-junction with priority control and includes a short auxiliary right-turn lane (CHR(S)) for northbound traffic turning right into Airport Road. No auxiliary left-turn lane is provided for southbound traffic turning left into Airport Road.

The SIDRA model was configured as follows:

- Major road: Newell Highway, with two uninterrupted through lanes
- Minor road: Airport Road, with a single approach lane for both turning movements
- Right-turn treatment: CHR(S) on the northbound approach
- Speed environment: 110 km/h for Newell Highway, 60 km/h for Airport Road

The intersection of Newell Highway/Airport Road layout is depicted in Figure 5-2.



Source: SIXMAPS

Figure 5-2 Newell Highway/Airport Road Intersection Layout

5.4 Base Case Performance Outputs

SIDRA Intersection outputs for the base case indicate that the Newell Highway / Airport Road intersection performs well below capacity for both AM and PM peak periods. Detailed performance results for each approach and movement are summarised below.

5.4.1 <u>AM Peak (Base Case)</u>

- Highest Degree of Saturation (DoS): 0.042 (Newell Hwy NB through and left-turn)
- Average delay: 5.7 seconds for minor road movements
- Level of Service (LoS): A across all approaches
- 95th percentile queue length:
 - Airport Rd Right Turn: 1.1 m
 - Airport Rd Left Turn: 1.1 m

5.4.2 PM Peak (Base Case)

- Highest Degree of Saturation (DoS): 0.055 (Newell Hwy NB through and left-turn)
- Average delay: 6.0 seconds for minor road movements
- Level of Service (LoS): A across all approaches
- 95th percentile queue length:
 - Airport Rd Right Turn: 1.2 m
 - Airport Rd Left Turn: 1.1 m



All performance indicators confirm that the intersection operates with significant spare capacity and minimal queuing under existing traffic conditions.

5.5 **Project Case Performance and Comparison with the Base Case**

SIDRA modelling results for the Project Case, which includes development-generated traffic during the AM and PM event peaks (60 vehicles per hour), indicate a marginal increase in overall traffic demand at the intersection. This additional volume was distributed with 60 per cent assigned to/from the north and 40 per cent to/from the south, consistent with expected regional travel patterns.

5.5.1 <u>AM Peak (Project Case)</u>

In the AM peak, the dominant development traffic movements are right turns from Newell Highway northbound into Airport Road and left turns from Newell Highway southbound into Airport Road. These account for the arrival phase of the event.

- The Degree of Saturation (DoS) increased from 0.042 in the base case to 0.082.
- Average delays remained consistent with the base case, ranging from 6.5 to 7.0 seconds across critical movements.
- 95th percentile queues increased marginally to 1.4 m, remaining within available storage.
- Level of Service remained at LoS A for all movements.

5.5.2 PM Peak (Project Case)

In the PM peak, development traffic is concentrated in outbound movements, notably left turns from Airport Road onto Newell Highway northbound and right turns onto Newell Highway southbound.

- The highest DoS was observed on the Airport Road approach at 0.096.
- Average delays for the right and left turn movements on Airport Road were 6.4 to 7.1 seconds.
- 95th percentile queue length increased to 3.4 m for the right turn from Airport Road, which is still well within available lane capacity.
- All movements retained Level of Service A.

5.5.3 <u>Comparison with Base Case</u>

When compared to the base case, the Project Case shows the following performance changes:

- DoS increased by approximately 0.04 on the critical approaches.
- Queue lengths increased modestly (e.g. 1.2 m to 3.4 m in the PM peak on the minor road approach).
- Delays increased slightly but remained well below 10 seconds per vehicle.
- No movements dropped below Level of Service A.

The Project Case confirms that the intersection has adequate capacity to accommodate the proposed development without requiring any changes to the existing priority control or intersection geometry.

5.6 **Project Case with Mitigation**

SIDRA modelling was extended to include a scenario with an auxiliary left-turn lane (AUL) added to the Newell Highway southbound approach. This configuration was tested to determine whether enhanced geometry would improve intersection performance.

5.6.1 <u>AM Peak – With Mitigation</u>

With the left-turn treatment in place:

- The highest Degree of Saturation (DoS) observed was 0.052 on the Newell Highway north-east approach.
- The south-west right-turn movement showed a DoS of 0.046.



- Delays ranged from 5.7 to 7.2 seconds across all movements.
- The 95th percentile queue lengths remained modest, at a maximum of 1.6 m.
- All movements operated at Level of Service A.

5.6.2 PM Peak – With Mitigation

During the PM peak:

- The highest DoS was observed on the Airport Road approach at 0.112.
- Right and left turns from Airport Road experienced delays of 6.7 to 7.2 seconds.
- The 95th percentile queue for the Airport Road approach reached 3.8 m.
- All other movements remained under 0.045 DoS.
- Level of Service remained at A for all vehicle movements.

5.6.3 <u>Comparison with Project Case (No Mitigation)</u>

Compared with the standard project case:

- The AUL treatment slightly reduced delays and queues on the Airport Road approach.
- The left-turn movement benefits from reduced interaction with through traffic on Newell Highway.
- Although marginal, the operational improvements enhance safety by separating decelerating vehicles from high-speed through traffic.

The mitigation scenario confirms that the auxiliary left-turn treatment is not operationally essential under current forecasts. Still, it provides minor improvements and may be justified on safety grounds in the context of future growth or event-specific peak surges.

Scenario	Peak	Highest DoS	Max Avg Delay (s)	Max 95% Queue (m)	Overall LoS
Base Case	AM	0.042	5.7	1.1	A
	PM	0.055	6.0	1.2	A
Project Case	AM	0.082	6.4	1.4	А
	PM	0.096	6.9	3.4	A
Project + Mitigation	AM	0.052	6.1	1.6	А
	PM	0.112	7.2	3.8	А

Table 5-2 SIDRA Summary Results – Newell Highway / Airport Road Intersection



5.7 Modelling Interpretation & Conclusion

The SIDRA analysis across the Base Case, Project Case, and Project Case with Mitigation confirms that the give-way controlled intersection of Newell Highway and Airport Road operates well within capacity in all assessed scenarios. The intersection functions as a priority T-junction, with through and turning movements on Newell Highway holding right of way and Airport Road approaches operating under give-way control.

In the Base Case, the intersection performs at a high level of service, with Degree of Saturation (DoS) values well below 0.06, average delays under 7 seconds, and minimal queuing on the minor approach. These results are consistent with the site's rural context and the low-volume nature of background traffic.

Performance remains satisfactory in the Project Case, which includes an additional 60 vehicles per hour during each peak period (based on development-generated traffic). The majority of new traffic affects the minor leg (Airport Road), particularly outbound movements during the PM peak. The right-turn movement from Airport Road, which must yield to two-way through traffic on Newell Highway, experiences a DoS increase to 0.096 and 95th percentile queue lengths up to 3.4 m. Nonetheless, all movements retain Level of Service A and delays remain under 8 seconds. The increased traffic loading does not result in any significant deterioration in intersection performance.

In the Project Case with Mitigation, an Auxiliary Left Turn (AUL) treatment is added to the Newell Highway southbound approach. While this does not alter intersection control, it does assist with traffic flow and safety by removing decelerating vehicles from the through lane. This modification reduces DoS and delays marginally, with queue lengths falling slightly for affected movements. However, the intersection still operates at Level of Service A across all movements without the mitigation, indicating that the upgrade is not operationally required.

The modelling confirms that while the give-way controlled Airport Road approach experiences the highest sensitivity to added traffic, performance remains well within acceptable thresholds. The mitigation measure offers a minor operational benefit and a modest improvement to safety through vehicle separation.

Overall, the intersection can accommodate development traffic without upgrade, and any proposed mitigation may be considered for safety enhancement rather than operational necessity.



6 Access and Internal Layout Assessment

6.1 Access Driveway Configuration

The development will be accessed via a sealed driveway connection from Airport Road, approximately 150 metres from the Newell Highway intersection. The driveway provides a direct entry point into the primary internal access corridor, allowing for efficient movement of inbound vehicles during event peaks.

The access point is aligned to support safe turning movements with minimal conflict and sufficient width to accommodate two-way traffic, including vehicle-trailer combinations. The driveway intersects with the internal loop road that distributes traffic toward the parking area, spectator mound, and track facilities. Entry geometry complies with AS2890.1 (Clause 3.2), ensuring adequate turning paths for vehicles approaching from either direction on Airport Road.

6.2 Swept Path and Turning Templates

Swept path assessments have confirmed that the proposed driveway and circulation layout can accommodate 12.5 m Heavy Rigid Vehicles (HRVs) and emergency vehicles. Vehicle turning paths allow for entry and exit without the need for reversing or manoeuvring over unsealed verges. Turning templates are compliant with Austroads Design Vehicle guidelines, and transitions are suitable for dual-axle trailers often associated with motorsport logistics.

No internal conflict is expected under normal or event conditions, and the provision of large-radius curves at entry and egress points supports uninterrupted vehicle flow.

6.3 Internal Circulation

The internal road network follows a loop configuration, allowing circulation from the driveway entrance through the event precinct and back out toward Airport Road. The loop road connects directly to designated parking bays and staging zones, with an intuitive one-way flow supported by natural geometry.

Circulation design supports the segregation of spectator vehicles and service vehicle paths, reducing the likelihood of conflict near the spectator mound and track edge. Culverts No. 1 and 3 are integrated beneath internal access points to manage stormwater flows and maintain continuous vehicle access even under wet conditions.

Pavement treatments are anticipated to be sealed or compacted gravel, sufficient for rural event use. Road widths and clearances allow for simultaneous bidirectional movements, and verge zones have been retained to accommodate turning error margins and emergency stops.

6.4 Pedestrian Access and Spectator Movement

Pedestrian access within the site is facilitated via connections between the parking area and the spectator mound, which is elevated for safe viewing above the circuit level. The internal layout ensures that pedestrian pathways remain clear of vehicle access lanes during events.

Although dedicated footpaths are not proposed, movement corridors are sufficiently wide to support informal pedestrian flows. Temporary barriers or marshal-managed crossings may be used during high-demand periods to maintain safe pedestrian circulation.

No cycling infrastructure is proposed due to the remote, event-based nature of the development and lack of connection to surrounding active transport networks.

6.5 Compliance with AS2890 Series

The proposed access and internal layout have been reviewed against relevant Australian Standards and are deemed compliant with the following:

• **AS2890.1–2004** – General vehicle access and parking dimensions (applies to all standard bays and circulation paths).



- **AS2890.2–2018** Service vehicle access (confirmed through turning templates for HRV entry and exit).
- **AS2890.6–2022** Accessible parking requirements (2 accessible bays to be positioned adjacent to spectator entry).

The proposed layout provides sufficient width, turning radius, and delineation to safely accommodate the expected vehicle mix and volume during event operations, including spectators, trailers, service vehicles, and emergency response units.



7 Parking Assessment

7.1 Estimated Parking Demand

The parking demand for the proposed motorsport park was estimated based on projected attendance during typical event operations and comparable facility benchmarks. The facility is expected to cater to local and regional-level motorsport events, generally attended by:

- Drivers and race teams
- Spectators
- Event staff, officials, and marshals
- Support vehicles including trailers and transport Utes

A peak event was assumed to attract approximately 200 persons on-site during the busiest hour. Vehiclebased arrival assumptions were developed based on the following parameters:

 Table 7-1
 Estimated Vehicle Arrival Rate During Events

User Group	Estimated Persons	Occupancy Ratio	Estimated Vehicles	Notes
Drivers & Crew	60	1.5	40	Includes drivers and at least one pit crew
Spectators	120	2.0	60	Primarily local arrivals in private vehicles
Officials & Volunteers	20	1.0	20	Arriving individually or in pairs
Total	200	_	120	Inclusive of support vehicles

Of the total estimated 120 vehicles:

- Approximately 20 to 25 may be towing trailers or be light trucks/Utes with support equipment.
- The remainder are expected to be standard passenger vehicles.

Thus, the estimated peak parking demand is approximately 120 spaces, noting that this figure includes informal requirements for larger parking footprints due to trailers.

However, not all attendees arrive at the same time. Based on event phasing and staggered arrivals/departures (e.g. staff arriving early, spectators arriving closer to start), a coincident peak parking occupancy of approximately 80–90 vehicles is considered a reasonable design allowance.

7.2 Provision in Proposed Layout

As shown in the concept design, the proposed parking arrangement includes approximately 80 delineated spaces located immediately south of the racetrack and adjacent to the spectator mound. The layout is supported by a looped internal access road and is designed to accommodate:

- Standard parking bays (2.5 m x 5.4 m)
- Informal trailer-compatible spaces at the periphery and oversize bays
- Parallel or angled bays along internal roads, consistent with rural site practice



The layout allows for direct pedestrian access to the spectator mound while maintaining spatial separation from the track and driveway circulation. Sightlines and vehicle manoeuvring have been reviewed and found to be sufficient for both cars and trailers.

7.3 Accessible Parking Provision

Two accessible parking spaces will be provided in accordance with AS2890.6–2022, located close to the primary pedestrian route leading to the spectator area. These will be line-marked and signed to ensure usability and compliance, and surface treatments will be level and suitable for wheelchair access.

7.4 Overflow and Contingency Parking

Beyond the 80 formalised bays, the site features grassed and unsealed shoulders that provide overflow or temporary event-specific parking flexibility, particularly during larger regional events. These areas can accommodate an additional 30 to 40 vehicles under managed conditions.

Event marshals or temporary signage may be used to guide overflow parking, with defined entry and exit paths to ensure safety and preserve emergency access routes.

7.5 Compliance and Functional Assessment

The overall parking supply:

- Meets the calculated coincident demand of 80–90 vehicles.
- Is expandable through flexible overflow areas.
- Accounts for the unique mix of vehicles, including Utes, trailers, and spectator cars.
- Provides sufficient circulation and access width in compliance with AS2890.1 and AS2890.2.

The proposed layout is therefore considered sufficient and appropriate for the scale and nature of the development, with adequate flexibility for operational peak conditions and future event growth.

7.6 Service and Refuse Vehicle Access

Service and waste collection operations for the proposed motorsport facility will be accommodated via the internal looped access road, which is designed to support Heavy Rigid Vehicles (HRVs) and similar commercial service vehicles.

The internal circulation layout allows for direct and uninterrupted access by waste collection trucks and delivery/service vehicles without requiring reversing movements or complex manoeuvres. Refuse collection points are to be positioned in proximity to the spectator and pit zones, with clear access provided from the driveway and return route to Airport Road.

Swept path assessments confirm that HRVs can navigate the entry driveway, circulate through the site, and exit without the need for off-tracking or encroachment into pedestrian areas. Turning radii, verge clearances, and entry/exit widths are consistent with the design requirements of AS2890.2–2018 (Off-Street Commercial Vehicle Facilities).

In the context of event operations, waste collection and supply deliveries are expected to occur outside of peak spectator arrival and departure times, minimising potential conflict. If servicing is required during event hours, temporary traffic management (e.g. marshal control or restricted access zones) can be implemented to maintain separation between pedestrians and large vehicles.

Service vehicle access is considered practical, safe, and compliant with industry standards, supporting ongoing operations and maintenance of the motorsport facility without compromising the performance of the internal road network or parking areas.



8 Safety Considerations

8.1 Access Sight Distance

The proposed access driveway onto Airport Road provides sufficient sight distance in both directions for vehicles entering and exiting the site. Based on the 60 km/h speed limit on Airport Road, the minimum Safe Intersection Sight Distance (SISD) of 105 metres is achieved, in accordance with Austroads Guide to Road Design Part 4A. The access location is positioned away from any horizontal or vertical curves that would otherwise constrain visibility.

8.2 Conflict Points

Given the single point of access and the low frequency of vehicle movement outside of events, the potential for traffic conflicts is low. On event days, temporary traffic management may be implemented to manage peak arrival and departure flows. The internal layout separates spectator parking from service and operational areas, which assists in reducing internal circulation conflict.

No significant pedestrian activity is anticipated on the external road network due to the rural context of the site. Internal pedestrian movement will be managed within designated areas during events, with options for temporary fencing, marshals, or barriers where necessary.

8.3 Emergency Access

The site layout allows for emergency vehicle access from Airport Road through the primary driveway. Sufficient swept path clearances are provided for standard fire appliances and ambulances in accordance with AS 2890.2:2018. Internal areas have been designed to allow for turning movements and through access, enabling emergency vehicles to reach key operational areas on-site without obstruction.

8.4 Traffic Management for Events

A Traffic Management Plan (TMP) may be prepared and implemented for larger events or those attracting regional attendance. The TMP would address ingress/egress arrangements, marshal locations, pedestrian management, temporary signage, and communication with emergency services. This plan can be prepared in consultation with Council and local police for significant events.



9 Mitigation Measures and Recommendations

Based on the findings of this assessment, the following mitigation measures are recommended to ensure safe and efficient operation of the development:

- The existing CHR(S) treatment on Newell Highway for northbound right-turn movements into Airport Road is considered adequate for the assessed development traffic. No immediate upgrade to a full CHR is required under current volumes.
- Consideration may be given to providing an auxiliary left-turn lane (AUL) on the southbound approach of Newell Highway, particularly if traffic volumes increase, or larger events are planned. This would provide improved deceleration and safety benefits.
- Clear advance signage should be installed on Newell Highway to indicate the turn-off to the Motorsport Park, particularly during events.
- Internal directional signage should be used to manage parking and service vehicle movements on event days.
- Accessible parking spaces should be designated and signposted in accordance with AS 2890.6.
- Temporary traffic marshals are recommended during peak periods to assist with traffic flow, parking coordination, and pedestrian safety.
- A site-specific Traffic Management Plan should be developed for major events, including risk mitigation measures, staging details, and coordination with emergency services.



10 Conclusion

This Traffic Impact Assessment has been prepared to accompany a development application for the proposed Western Wheelers Motorsport Park adjacent to West Wyalong Airport. The assessment has considered the proposed development's traffic generation, parking demand, access arrangements, and interaction with the surrounding road network.

The traffic generated by the development is expected to be concentrated during periodic events and is not anticipated to adversely impact the surrounding road network. The existing intersection of Newell Highway and Airport Road operates within capacity under both existing and project scenarios. No immediate upgrade works are considered necessary; however, future treatments may be implemented if growth or higher-than-forecast usage are warranted.

The internal site layout can accommodate the forecast parking demand and supports the safe movement of vehicles, including emergency and service access. No road safety issues were identified that would preclude the development from proceeding, and the access geometry complies with relevant Austroads and Australian Standard design requirements.

Subject to implementation of the recommendations in this report, the proposed development is considered to have an acceptable traffic and parking outcome and is supported from a traffic engineering perspective.



APPENDIX A

SIDRA LAYOUTS & MODEL RESULTS

SITE LAYOUT V Site: [1] 101 Newell Hwy-Airport Rd AM Base (Base Case)

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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MOVEMENT SUMMARY

Site: [1] 101 Newell Hwy-Airport Rd AM Base (Base Case)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Vehi	cle Mo	ovemen	t Performa	nce									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% E	Back Of	Prop.	Eff. I	Number	Aver.
ID		Class	Flows	Flows	Satn	Delay	Service	Qu	eue	Qued	Stop of	Cycles	Speed
			[Total HV]	[Total HV]				[Veh.	Dist]		Rate to	Depart	
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	: Airpo	ort Road											
1b	L3	All MCs	16 20.0	16 20.0	0.035	6.9	LOS A	0.1	1.1	0.21	0.54	0.21	51.5
3a	R1	All MCs	21 25.0	21 25.0	0.035	6.0	LOS A	0.1	1.1	0.21	0.54	0.21	51.5
Appro	bach		37 22.9	37 22.9	0.035	6.4	LOS A	0.1	1.1	0.21	0.54	0.21	51.5
North	East: I	Newell Hi	ghway										
24a	L1	All MCs	19 27.8	19 27.8	0.042	7.3	LOS A	0.0	0.0	0.00	0.19	0.00	62.7
25	T1	All MCs	47 35.6	47 35.6	0.042	0.0	LOS A	0.0	0.0	0.00	0.19	0.00	80.1
Appro	bach		66 33.3	66 33.3	0.042	2.1	NA	0.0	0.0	0.00	0.19	0.00	74.2
South	West:	Newell H	lighway										
31	T1	All MCs	60 29.8	60 29.8	0.037	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	11 30.0	11 30.0	0.007	6.7	LOS A	0.0	0.3	0.18	0.57	0.18	50.3
Appro	bach		71 29.9	71 29.9	0.037	1.0	NA	0.0	0.3	0.03	0.09	0.03	87.1
All Ve	hicles		174 29.7	174 29.7	0.042	2.6	NA	0.1	1.1	0.06	0.22	0.06	71.8

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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QUEUE ANALYSIS

V Site: [1] 101 Newell Hwy-Airport Rd AM Base (Base Case)

Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Lane Que	ues (Di	stance)												
Lane	Contin.	Deg.	Prog.	Overflow	Back of	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	((m)	Start	of Gap	Ave	rage	Sto	rage	Block. S	SL Ov. I	Lane
		(Queue)	(m)			1)	n)	Qu	eue	Ra	atio			No.
									(r	n)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.035	1.000	0.0	0.5	1.1	0.4	1.1	0.1	0.1	0.00	0.00	0.0	NA	NA
Approach		0.035			0.5	1.1	0.4	1.1	0.1	0.1	0.00	0.00			
NorthEast:	Newell H	lighway													
Lane 1	Y	0.042	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.042			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highway	/												
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.037			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection	า	0.042			0.5	1.1	0.4	1.1	0.1	0.1	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap. Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Lane Que	ues (Ve	hicles))												
Lane	Contin.	Deg.	Prog.	Overflow	Back o	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	(\	reh)	Start o	of Gap	Ave	rage	Stor	rage	Block. S	L Ov. I	_ane
		((Queue)	(veh)			(\4	eh)	Qu	eue	Ra	atio			No.
									(V	eh)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.035	1.000	0.0	0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.035			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			
NorthEast:	Newell F	lighway													
Lane 1	Y	0.042	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.042			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highwa	у												
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.037			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
Intersection	ı	0.042			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap. Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D). Short Lanes are not included in determining Queue Storage Ratios.

Continuous La	ane Per	forman	се										
Lane Number	Deg.	Unint.	Unint.	Hdwy	Spacing	Aver.	Occup.	Space	Space	Time	Den	isity	LOS
	Satn	Speed	Travel			Vehicle	Time	Time	Occup.	Occup.			(Density
			Delay			Length			Ratio	Ratio			Method)
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	
NorthEast: Newe	ell Highw	/ay											
Lane 1	0.042	84.0	0.0	54.29	1266.3	6.3	0.36	53.93	0.5	0.7	0.8	1.0	LOS A
SouthWest: New	ell High	way											
Lane 1	0.037	100.0	0.0	60.00	1666.4	6.1	0.29	59.71	0.4	0.5	0.6	0.8	LOS A
	0	,	0.0	60.00	1666.4	6.1	0.29	59.71	0.4	0.5	0.6	0.8	

Midblock Effective Detection Zone Length = 2 m

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MOVEMENT SUMMARY

abla Site: [1 (3)] 101 Newell Hwy-Airport Rd AM Project Case (Project Case) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Vehic	cle Mo	ovement	t Performa	nce									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% B	ack Of	Prop.	Eff. I	Number	Aver.
ID		Class	Flows	Flows	Satn	Delay	Service	Que	eue	Qued	Stop of	f Cycles	Speed
			[Total HV]	[Total HV]				[Veh.	Dist]		Rate to	Depart	
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	: Airpo	ort Road											
1b	L3	All MCs	16 20.0	16 20.0	0.038	6.9	LOS A	0.1	1.2	0.23	0.55	0.23	51.4
3a	R1	All MCs	21 25.0	21 25.0	0.038	6.6	LOS A	0.1	1.2	0.23	0.55	0.23	51.3
Appro	ach		37 22.9	37 22.9	0.038	6.7	LOS A	0.1	1.2	0.23	0.55	0.23	51.3
North	East: I	Newell Hi	ghway										
24a	L1	All MCs	82 29.5	82 29.5	0.082	6.5	LOS A	0.0	0.0	0.00	0.40	0.00	52.9
25	T1	All MCs	47 35.6	47 35.6	0.082	0.0	LOS A	0.0	0.0	0.00	0.40	0.00	65.3
Appro	ach		129 31.7	129 31.7	0.082	4.1	NA	0.0	0.0	0.00	0.40	0.00	56.9
South	West:	Newell H	lighway										
31	T1	All MCs	60 29.8	60 29.8	0.037	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	48 30.4	48 30.4	0.036	7.0	LOS A	0.2	1.4	0.28	0.59	0.28	50.1
Appro	ach		108 30.1	108 30.1	0.037	3.1	NA	0.2	1.4	0.12	0.26	0.12	69.2
All Ve	hicles		275 29.9	275 29.9	0.082	4.1	NA	0.2	1.4	0.08	0.37	0.08	60.2

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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QUEUE ANALYSIS

Site: [1 (3)] 101 Newell Hwy-Airport Rd AM Project Case (Project Case) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Lane Que	eues (Di	stance))												
Lane	Contin.	Deg.	Prog.	Overflow	Back of	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	((m)	Start	of Gap	Ave	rage	Sto	rage	Block. S	L Ov. I	Lane
		(Queue)	(m)			1)	n)	Qu	eue	Ra	atio			No.
									(r	n)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.038	1.000	0.0	0.5	1.2	0.5	1.2	0.1	0.2	0.00	0.00	0.0	NA	NA
Approach		0.038			0.5	1.2	0.5	1.2	0.1	0.2	0.00	0.00			
NorthEast:	Newell F	lighway													
Lane 1	Y	0.082	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.082			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highway	/												
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.036	1.000	0.0	0.6	1.4	0.6	1.4	0.1	0.1	0.01	0.01	NA	0.0	1
Approach		0.037			0.6	1.4	0.6	1.4	0.1	0.1	0.00	0.00			
Intersection	า	0.082			0.6	1.4	0.6	1.4	0.1	0.2	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Lane Que	ues (Ve	hicles)													
Lane	Contin.	Deg.	Prog.	Overflow	Back	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	()	/eh)	Start	of Gap	Ave	rage	Sto	rage	Block. S	SL Ov. I	ane
			Queue)	(veh)			(\v	eh)		eue	Ra	atio			No.
										eh)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.038	1.000	0.0	0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.038			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			
NorthEast:	Newell H	lighway													
Lane 1	Y	0.082	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.082			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highwa	y												
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.036	1.000	0.0	0.1	0.2	0.1	0.2	0.0	0.0	0.01	0.01	NA	0.0	1
Approach		0.037			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			
Intersection	ı	0.082			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Continuous L	ane Per	forman	се										
Lane Number	Deg.	Unint.	Unint.	Hdwy	Spacing	Aver.	Occup.	Space	Space	Time	Den	sity	LOS
	Satn	Speed	Travel			Vehicle	Time	Time	Occup.	Occup.			(Density
			Delay			Length			Ratio	Ratio			Method)
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	
NorthEast: New	ell Highw	ay											
Lane 1	0.082	70.2	0.0	27.80	542.5	6.2	0.42	27.38	1.2	1.5	1.8	2.3	LOS A
SouthWest: New	ell High	way											
Lane 1	0.037	100.0	0.0	60.00	1666.4	6.1	0.29	59.71	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

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MOVEMENT SUMMARY

Site: [1 (2)] 101 Newell Hwy-Airport Rd PM Base (Base Case) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Vehi	cle Mo	ovemen	t Performa	nce									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% E	ack Of	Prop.	Eff.	Number	Aver.
ID		Class	Flows	Flows	Satn	Delay	Service	Qu	eue	Qued	Stop o	f Cycles	Speed
			[Total HV]	[Total HV]				[Veh.	Dist]		Rate to	o Depart	
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	: Airpo	ort Road											
1b	L3	All MCs	16 20.0	16 20.0	0.036	7.0	LOS A	0.1	1.2	0.25	0.55	0.25	51.4
3a	R1	All MCs	21 25.0	21 25.0	0.036	6.2	LOS A	0.1	1.2	0.25	0.55	0.25	51.3
Appro	bach		37 22.9	37 22.9	0.036	6.5	LOS A	0.1	1.2	0.25	0.55	0.25	51.4
North	East: I	Newell Hi	ghway										
24a	L1	All MCs	19 27.8	19 27.8	0.055	7.5	LOS A	0.0	0.0	0.00	0.14	0.00	65.4
25	T1	All MCs	71 31.3	71 31.3	0.055	0.0	LOS A	0.0	0.0	0.00	0.14	0.00	84.5
Appro	bach		89 30.6	89 30.6	0.055	1.6	NA	0.0	0.0	0.00	0.14	0.00	79.6
South	West:	Newell H	lighway										
31	T1	All MCs	61 32.8	61 32.8	0.038	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	11 30.0	11 30.0	0.007	6.8	LOS A	0.0	0.3	0.22	0.57	0.22	50.2
Appro	bach		72 32.4	72 32.4	0.038	1.0	NA	0.0	0.3	0.03	0.08	0.03	87.3
All Ve	hicles		198 29.8	198 29.8	0.055	2.3	NA	0.1	1.2	0.06	0.20	0.06	74.3

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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QUEUE ANALYSIS

Site: [1 (2)] 101 Newell Hwy-Airport Rd PM Base (Base Case) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Lane Que	eues (Di	stance)												
Lane	Contin.	Deg.	Prog.	Overflow	Back	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue		(m)	Start	of Gap	Ave	rage	Sto	rage	Block. S	SL Ov. I	Lane
			Queue)	(m)			1)	n)	Qu	eue	Ra	atio			No.
									(r	n)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.036	1.000	0.0	0.5	1.2	0.5	1.1	0.1	0.1	0.00	0.00	0.0	NA	NA
Approach		0.036			0.5	1.2	0.5	1.1	0.1	0.1	0.00	0.00			
NorthEast:	Newell H	lighway													
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highway	/												
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection	า	0.055			0.5	1.2	0.5	1.1	0.1	0.1	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap. Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Lane Que	ues (Ve	hicles)													
Lane	Contin.	Deg.	Prog.	Overflow	Back c	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	(\	eh)	Start of	of Gap	Ave	rage	Sto	rage	Block. S	L Ov. I	_ane
		(Queue)	(veh)			(Ve	eh)	Qu	eue	Ra	atio			No.
									(V	eh)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.036	1.000	0.0	0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.036			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			
NorthEast:	Newell F	lighway													
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highwa	/												
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
Intersectior	ı	0.055			0.1	0.1	0.1	0.1	0.0	0.0	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap. Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D). Short Lanes are not included in determining Queue Storage Ratios.

Continuous L	ane Pei	rforman	се										
Lane Number	Deg.	Unint.	Unint.	Hdwy	Spacing	Aver.	Occup.	Space	Space	Time	Den	isity	LOS
	Satn	Speed	Travel			Vehicle	Time	Time	Occup.	Occup.			(Density
			Delay			Length			Ratio	Ratio			Method)
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	
NorthEast: New	ell Highv	vay											
Lane 1	0.055	87.6	0.0	40.24	979.0	6.2	0.34	39.90	0.6	0.8	1.0	1.3	LOS A
SouthWest: New	vell High	way											
Lane 1	0.038	100.0	0.0	58.97	1637.6	6.3	0.30	58.67	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

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MOVEMENT SUMMARY

abla Site: [1 (4)] 101 Newell Hwy-Airport Rd PM Project Case (Project Case) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Vehio	cle Mo	ovement	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver.	Level of Service	95% B Qu	ack Of	Prop. Qued		Number f Cycles	Aver. Speed
שו		Class	[Total HV]		Sam v/c	Delay sec	Service	[Veh. veh	Dist] m	Quea		Depart	speed km/h
South	: Airpo	ort Road											
1b	L3	All MCs	54 27.5	54 27.5	0.096	7.1	LOS A	0.4	3.4	0.25	0.56	0.25	51.0
3a	R1	All MCs	46 27.3	46 27.3	0.096	6.4	LOS A	0.4	3.4	0.25	0.56	0.25	51.1
Appro	ach		100 27.4	100 27.4	0.096	6.8	LOS A	0.4	3.4	0.25	0.56	0.25	51.1
North	East: I	Newell Hi	ghway										
24a	L1	All MCs	19 27.8	19 27.8	0.055	7.5	LOS A	0.0	0.0	0.00	0.14	0.00	65.4
25	T1	All MCs	71 31.3	71 31.3	0.055	0.0	LOS A	0.0	0.0	0.00	0.14	0.00	84.5
Appro	ach		89 30.6	89 30.6	0.055	1.6	NA	0.0	0.0	0.00	0.14	0.00	79.6
South	West:	Newell H	lighway										
31	T1	All MCs	61 32.8	61 32.8	0.038	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	11 30.0	11 30.0	0.007	6.8	LOS A	0.0	0.3	0.22	0.57	0.22	50.2
Appro	ach		72 32.4	72 32.4	0.038	1.0	NA	0.0	0.3	0.03	0.08	0.03	87.3
All Ve	hicles		261 29.8	261 29.8	0.096	3.4	NA	0.4	3.4	0.10	0.29	0.10	66.8

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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QUEUE ANALYSIS

Site: [1 (4)] 101 Newell Hwy-Airport Rd PM Project Case (Project Case) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Lane Que	ues (Di	stance))												
Lane	Contin.	Deg.	Prog.	Overflow	Back of	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	(m)	Start	of Gap	Ave	rage	Sto	rage	Block. S	SL Ov. I	_ane
			Queue)	(m)			(r	n)	Qu	eue	Ra	atio			No.
									(r	n)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.096	1.000	0.0	1.4	3.4	1.2	3.1	0.2	0.4	0.00	0.01	0.0	NA	NA
Approach		0.096			1.4	3.4	1.2	3.1	0.2	0.4	0.00	0.01			
NorthEast:	Newell F	lighway													
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highway	y												
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection	ו	0.096			1.4	3.4	1.2	3.1	0.2	0.4	0.00	0.01			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Lane Que	ues (Ve	hicles)													
Lane	Contin.	Deg.	Prog.	Overflow	Back of	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	()	/eh)	Start of	of Gap	Ave	rage	Sto	rage	Block. S	L Ov. I	ane
		(Queue)	(veh)			(\v	eh)		eue eh)	Ra	atio			No.
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.096	1.000	0.0	0.2	0.4	0.1	0.4	0.0	0.0	0.00	0.01	0.0	NA	NA
Approach		0.096			0.2	0.4	0.1	0.4	0.0	0.0	0.00	0.01			
NorthEast:	Newell H	lighway													
Lane 1	Y	0.055	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.055			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highwa	y												
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.007	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
Intersection	ı	0.096			0.2	0.4	0.1	0.4	0.0	0.0	0.00	0.01			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Continuous L													
Lane Number	Deg.	Unint.	Unint.	Hdwy	Spacing	Aver.	Occup.	Space	Space	Time	Den	sity	LOS
	Satn	Speed	Travel			Vehicle	Time	Time	Occup.	Occup.			(Density
			Delay			Length			Ratio	Ratio			Method)
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	
NorthEast: New	ell Highv	vay											
Lane 1	0.055	87.6	0.0	40.24	979.0	6.2	0.34	39.90	0.6	0.8	1.0	1.3	LOS A
SouthWest: Nev	well High	way											
Lane 1	0.038	100.0	0.0	58.97	1637.6	6.3	0.30	58.67	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

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SITE LAYOUT V Site: [1 (5)] 101 Newell Hwy-Airport Rd AM Project Case with Mitigation (Project Case wit Mitigation)

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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MOVEMENT SUMMARY

V Site: [1 (5)] 101 Newell Hwy-Airport Rd AM Project Case with Mitigation (Project Case wit Mitigation) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Vehic	cle Mo	ovemen	t Performa	nce									
Mov ID	Turn	Mov Class	Demand Flows	Arrival Flows	Deg. Satn	Aver. Delay	Level of Service	Qu	ack Of eue	Prop. Qued	Stop o	Number f Cycles	Aver. Speed
				[Total HV] veh/h %	v/c	sec		[Veh. veh	Dist] m		Rate to	o Depart	km/h
South	: Airpo	ort Road											
1b	L3	All MCs	16 20.0	16 20.0	0.044	6.9	LOS A	0.2	1.4	0.25	0.55	0.25	51.3
3a	R1	All MCs	21 25.0	21 25.0	0.044	6.9	LOS A	0.2	1.4	0.25	0.55	0.25	51.3
Appro	ach		37 22.9	37 22.9	0.044	6.9	LOS A	0.2	1.4	0.25	0.55	0.25	51.3
North	East: N	Newell Hi	ghway										
24a	L1	All MCs	82 29.5	82 29.5	0.052	5.7	LOS A	0.0	0.0	0.00	0.60	0.00	51.5
25	T1	All MCs	47 35.6	47 35.6	0.030	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
Appro	ach		129 31.7	129 31.7	0.052	3.6	NA	0.0	0.0	0.00	0.38	0.00	62.6
South	West:	Newell H	lighway										
31	T1	All MCs	60 29.8	60 29.8	0.037	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	48 30.4	48 30.4	0.046	7.2	LOS A	0.2	1.6	0.28	0.60	0.28	50.1
Appro	ach		108 30.1	108 30.1	0.046	3.2	NA	0.2	1.6	0.12	0.27	0.12	69.2
All Ve	hicles		275 29.9	275 29.9	0.052	3.9	NA	0.2	1.6	0.08	0.36	0.08	63.1

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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QUEUE ANALYSIS

Site: [1 (5)] 101 Newell Hwy-Airport Rd AM Project Case with Mitigation (Project Case wit Mitigation) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Lane Que	ues (Di	stance)												
Lane	Contin.	Deg.	Prog.	Overflow	Back of	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue		(m)	Start of	of Gap	Ave	rage	Sto	rage	Block. S	SL Ov. I	Lane
			Queue)	(m)			(r	n)	Qu	eue	Ra	atio			No.
									1)	n)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.044	1.000	0.0	0.5	1.4	0.5	1.3	0.1	0.2	0.00	0.00	0.0	NA	NA
Approach		0.044			0.5	1.4	0.5	1.3	0.1	0.2	0.00	0.00			
NorthEast:	Newell H	lighway													
Lane 1	Y	0.052	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.030	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.052			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highwa	y												
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.046	1.000	0.0	0.6	1.6	0.6	1.5	0.1	0.2	0.01	0.02	NA	0.0	1
Approach		0.046			0.6	1.6	0.6	1.5	0.1	0.2	0.00	0.00			
Intersection	۱	0.052			0.6	1.6	0.6	1.5	0.1	0.2	0.00	0.00			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Lane Que	eues (Ve	hicles)	1												
Lane	Contin.	Deg.	Prog.	Overflow	Back c	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	(v	eh)	Start	of Gap	Ave	rage	Sto	rage	Block. S	L Ov.	Lane
			Queue)	(veh)			(V	eh)		eue eh)	Ra	atio			No.
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.044	1.000	0.0	0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.044			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			
NorthEast:	Newell F	lighway													
Lane 1	Y	0.052	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.030	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.052			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highwa	y												
Lane 1	Y	0.037	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.046	1.000	0.0	0.1	0.2	0.1	0.2	0.0	0.0	0.01	0.02	NA	0.0	1
Approach		0.046			0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00			

Intersection	0.052	0.1	0.2	0.1	0.2	0.0	0.0	0.00	0.00
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Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Continuous La	ane Pei	rforman	се										
Lane Number	Deg.	Unint.	Unint.	Hdwy \$	Spacing	Aver.	Occup.	Space	Space	Time	Den	sity	LOS
	Satn	Speed	Travel			Vehicle	Time	Time	Occup.	Occup.			(Density
			Delay			Length			Ratio	Ratio			Method)
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	
NorthEast: New	ell Highv	vay											
Lane 1	0.052	60.0	0.0	43.85	730.4	6.1	0.49	43.36	0.8	1.1	1.4	1.7	LOS A
Lane 2	0.030	100.0	0.0	76.00	2110.8	6.5	0.30	75.70	0.3	0.4	0.5	0.6	LOS A
SouthWest: New	vell High	way											
Lane 1	0.037	100.0	0.0	60.00	1666.4	6.1	0.29	59.71	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

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MOVEMENT SUMMARY

V Site: [1 (6)] 101 Newell Hwy-Airport Rd PM Project Case with Mitigation (Project Case wit Mitigation) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Vehic	cle Mo	ovement	t Performa	nce									
Mov	Turn	Mov	Demand	Arrival	Deg.	Aver.	Level of	95% B	ack Of	Prop.	Eff.	Number	Aver.
ID		Class	Flows	Flows	Satn	Delay	Service	Que	eue	Qued	Stop o	f Cycles	Speed
			[Total HV]	[Total HV]				[Veh.	Dist]		Rate to	o Depart	
			veh/h %	veh/h %	v/c	sec		veh	m				km/h
South	: Airpo	ort Road											
1b	L3	All MCs	54 27.5	54 27.5	0.112	7.2	LOS A	0.4	3.8	0.26	0.57	0.26	51.0
3a	R1	All MCs	46 27.3	46 27.3	0.112	6.6	LOS A	0.4	3.8	0.26	0.57	0.26	51.3
Appro	ach		100 27.4	100 27.4	0.112	6.9	LOS A	0.4	3.8	0.26	0.57	0.26	51.1
North	East: I	Vewell Hi	ghway										
24a	L1	All MCs	19 27.8	19 27.8	0.012	5.7	LOS A	0.0	0.0	0.00	0.60	0.00	51.6
25	T1	All MCs	71 31.3	71 31.3	0.044	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
Appro	ach		89 30.6	89 30.6	0.044	1.2	NA	0.0	0.0	0.00	0.13	0.00	83.4
South	West:	Newell H	lighway										
31	T1	All MCs	61 32.8	61 32.8	0.038	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
32b	R3	All MCs	11 30.0	11 30.0	0.009	6.9	LOS A	0.0	0.3	0.22	0.58	0.22	50.2
Appro	ach		72 32.4	72 32.4	0.038	1.0	NA	0.0	0.3	0.03	0.09	0.03	87.3
All Ve	hicles		261 29.8	261 29.8	0.112	3.3	NA	0.4	3.8	0.11	0.28	0.11	67.8

Site Level of Service (LOS) Method: Delay (NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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QUEUE ANALYSIS

Site: [1 (6)] 101 Newell Hwy-Airport Rd PM Project Case with Mitigation (Project Case wit Mitigation) Output produced by SIDRA INTERSECTION Version: 10.0.5.217

New Site Site Category: (None) Give-Way (Two-Way) Site Scenario: 1 | Local Volumes

Lane Que	ues (Di	stance)												
Lane	Contin.	Deg.	Prog.	Overflow	Back o	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue		m)	Start o	of Gap	Ave	rage	Sto	rage	Block. S	SL Ov. I	Lane
			Queue)	(m)			(r	n)	Qu	eue	Ra	atio			No.
									(r	n)					
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.112	1.000	0.0	1.5	3.8	1.4	3.4	0.2	0.4	0.00	0.01	0.0	NA	NA
Approach		0.112			1.5	3.8	1.4	3.4	0.2	0.4	0.00	0.01			
NorthEast:	Newell F	lighway													
Lane 1	Y	0.012	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.044	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.044			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	: Newell	Highwa	y												
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.009	1.000	0.0	0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.1	0.3	0.1	0.3	0.0	0.0	0.00	0.00			
Intersection	۱	0.112			1.5	3.8	1.4	3.4	0.2	0.4	0.00	0.01			

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Lane Que	eues (Ve	hicles)													
Lane	Contin.	Deg.	Prog.	Overflow	Back c	of Queue	Que	ue at	Су	cle-	Qu	eue	Prob.	Prob.	Ov.
Number	Lane	Satn	Factor	Queue	(۷	eh)	Start of	of Gap	Ave	rage	Stor	rage	Block. S	L Ov.	Lane
			Queue)	(veh)			(\v	eh)		eue eh)	Ra	atio			No.
		v/c			Av.	95%	Av.	95%	Av.	95%	Av.	95%	%	%	
South: Airp	ort Road														
Lane 1		0.112	1.000	0.0	0.2	0.4	0.2	0.4	0.0	0.1	0.00	0.01	0.0	NA	NA
Approach		0.112			0.2	0.4	0.2	0.4	0.0	0.1	0.00	0.01			
NorthEast:	Newell F	lighway													
Lane 1	Y	0.012	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	2
Lane 2	Y	0.044	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Approach		0.044			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			
SouthWest	t: Newell	Highwa	y												
Lane 1	Y	0.038	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	0.0	NA	NA
Lane 2		0.009	1.000	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00	NA	0.0	1
Approach		0.038			0.0	0.0	0.0	0.0	0.0	0.0	0.00	0.00			

Intersection	0.112	0.2	0.4	0.2	0.4	0.0	0.1	0.00	0.01

Two-Way Sign Control Capacity Model: SIDRA Standard.

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

Short Lanes are not included in determining Queue Storage Ratios.

Continuous Lane Performance													
Lane Number	Deg.	Unint.	Unint.	Hdwy	Spacing	Aver.	Occup.	Space	Space	Time	Den	sity	LOS
	Satn	Speed	Travel			Vehicle	Time	Time	Occup.	Occup.			(Density
			Delay			Length			Ratio	Ratio			Method)
	v/c	km/h	sec	sec	m	m	sec	sec	%	%	veh/km	pc/km	
NorthEast: New	ell Highv	vay											
Lane 1	0.012	60.0	0.0	190.00	3166.4	6.0	0.48	189.52	0.2	0.3	0.3	0.4	LOS A
Lane 2	0.044	100.0	0.0	51.04	1417.6	6.2	0.30	50.75	0.4	0.6	0.7	0.9	LOS A
SouthWest: Newell Highway													
Lane 1	0.038	100.0	0.0	58.97	1637.6	6.3	0.30	58.67	0.4	0.5	0.6	0.8	LOS A

Midblock Effective Detection Zone Length = 2 m

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APPENDIX B

DEVELOPMENT PLANS

WESTERN WHEELERS MOTORSPORT PARK

REVISION D AMENDED FOR COUNCIL APPROVAL 25-08-2023

LEGEND

EXISTING FENCELINE

PROPOSED FENCE LINE

PROPOSED GUARD RAIL

PROPOSED BATTER

PROPOSED CULVERT

PROPOSED DRAIN INVERT

PROPOSED ROAD CENTRELINE PROPOSED EDGE OF SEAL



PROJECT LOCATION



REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404

	DRAWING SCHEDULE
SHEET	DRAWING TITLE
0	COVER SHEET
1	NOTES & BILL OF QUANTITIES
2	TYPICAL CROSS SECTIONS
3	GENERAL LAYOUT PLAN
4	DRAG STRIP PLAN & LONG SECTION (-20 - 340)
5	DRAG STRIP PLAN & LONG SECTION (340 - 620)
6	DRAG STRIP CROSS SECTIONS (-20 - 40)
7	DRAG STRIP CROSS SECTIONS (60 – 120)
8	DRAG STRIP CROSS SECTIONS (140 – 200)
9	DRAG STRIP CROSS SECTIONS (221.17 – 280)
10	DRAG STRIP CROSS SECTIONS (300 – 360)
11	DRAG STRIP CROSS SECTIONS (380 – 440)
12	DRAG STRIP CROSS SECTIONS (460 – 520)
13	DRAG STRIP CROSS SECTIONS (525.17 – 580)
14	DRAG STRIP CROSS SECTIONS (600 – 616.17)
15	RETURN ROAD PLAN & LONG SECTION (0 - 360)
16	RETURN ROAD PLAN & LONG SECTION (360 - 560)
17	ROAD 2 CROSS SECTIONS (24.14 - 42.73)
18	ROAD 2 CROSS SECTIONS (50 - 62.87)
19	ROAD 2 CROSS SECTIONS (66.2 – 120)
20	ROAD 2 CROSS SECTIONS (140 – 200)
21	ROAD 2 CROSS SECTIONS 220 - 280)
22	ROAD 2 CROSS SECTIONS (300 – 360)
23	ROAD 2 CROSS SECTIONS (380 – 440)
24	ROAD 2 CROSS SECTIONS (460 – 520)
25	ROAD 2 CROSS SECTIONS (540 – 560)
26	ROAD 3 PLAN & LONG SECTION
27	ROAD 3 CROSS SECTIONS (19.3 – 50)
28	ROAD 3 CROSS SECTIONS (60 - 106.26)
29	ROAD 3 CROSS SECTIONS (110 - 127.44)
30	CULVERTS 1, 2 & 4 PROFILES
31	CULVERTS 5 & 6 PROFILES
32	EROSION & SEDIMENT CONTROL PLAN (WEST)
33	EROSION & SEDIMENT CONTROL PLAN (EAST)
34	EROSION & SEDIMENT CONTROL NOTES
35	EROSION & SEDIMENT CONTROL DETAILS

BILL OF QUANT

N	\cap	ΓF	ς ·
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GENERAL

- 1. THIS PLAN DOES NOT DEFINE PROPERTY BOUNDARIES.
- 2. THESE DRAWINGS MUST NOT BE SCALED.
- 3. ALL LEVELS SHOWN ARE AUSTRALIAN HEIGHT DATUM.
- 4. ALL DIMENSIONS AND REDUCED LEVELS MUST BE VERIFIED ON SITE BEFORE THE COMMENCEMENT OF ANY WORK.

STORMWATER

1. ALL STORMWATER STRUCTURES AND PIPE WORK TO BE CONSTRUCTED IN ACCORDANCE WITH BLAND SHIRE COUNCIL'S STORMWATER CONSTRUCTION SPECIFICATION AND MANUFACTURER'S RECOMMENDATIONS..

ROADS

 UNLESS NOTED OTHERWISE, ALL VEGETATION SHALL BE STRIPP 100mm UNDER ALL PROPOSED PAVEMENT AREAS. PRIOR TO THE PLACEMENT OF ANY PAVEMENTS, THE EXPOSED TO A MINIMUM OF 100% STANDARD DENSITY FOR THE TOP 30 REMOVED AND REPLACED WITH GRANULAR FILL TO THE COUNT COMPACTED IN ACCORDANCE WITH THE COMPACTION REQUIF ALL FILL AND PAVEMENT MATERIALS SHALL BE COMPACTED IN THICKNESS LAYERS TO THE DENSITIES SPECIFIED BELOW: FILL UNDER ROAD PAVEMENTS 	SUBGRADE SHALL BE COMPACTED 0mm. ANY SOFT SPOTS SHALL BE CIL ENGINEER'S REQUIREMENTS AND REMENTS SET OUT BELOW.
	1000/ 570
TO WITHIN 500mm OF FINISH SUBGRADE LEVEL	
UP TO FINISH SUBGRADE LEVEL	100-105% STD
ROAD PAVEMENT MATERIALS SUB BASE (DGS 20)	95% MDD
BASE COURSE (DGB20)	98% MDD
4. PROVIDE TWO COAT BITUMEN FLUSH SEAL WITH 14mm AND 7 COATS OF BITUMEN REQUIRED TO BE PROPERLY HEATED AND T COMPACTED TO THE BLAND SHIRE COUNCIL'S REQUIREMENTS.	THE FINISHED SURFACE TO BE
5. ROADWORKS TO BE CARRIED OUT IN ACCORDANCE WITH BLAN	
6. CONTRACTOR MUST HAVE ALL SERVICES ACCURATELY LOCATED	D BY THE RELEVANT SERVICE
AUTHORITY PRIOR TO COMMENCEMENT OF CONSTRUCTION.	
7. CONTRACTOR MUST TAKE CARE NOT TO DISTURB TREES THAT A	ARE TO REMAIN.
8. ANY TREES TO BE REMOVED FROM THE PROPOSED ROAD FORM	ATION MUST HAVE ALL ROOTS
GRUBBED FOR A MINIMUM DEPTH OF 500mm WITH ALL MAJO	

- GRUBBED FOR A MINIMUM DEPTH OF 500mm WITH ALL MAJOR ROOTS COMPLETELY REMOVED. REMAINING HOLES SHALL BE FULLY BACKFILLED WITH SELECT FILL MATERIAL AND THOROUGHLY COMPACTED TO 95% STANDARD DENSITY.
- 9. TABLE DRAINS SHALL BE SHAPED TO DIRECT WATER INTO CULVERTS.

Description	Unit	Quantity
Earthworks		
Cut to fill	m3	21200
Cut to spoil	m3	200
Pipe Drainage		
Single cell 450mm Dia. RRJ CL4 RCP	m	68.32
Concrete Headwalls 450mm RCP	each	11
Rock Riprap (d50 = 200mm, min. depth = 300mm)	m3	8
Flexible Pavements		
Drag Strip		
Base Course (150mm)	m3	1,760
Subbase Course (150mm)	m3	1,980
Return Roads		
Base Course (150mm)	m3	622
Subbase Course (150mm)	m3	745
Pit Area		
Base Course (150mm)	m3	1,000
Subbase Course (150mm)	m3	1,100
Parking Area		
Base Course (150mm)	m3	553
Subbase Course (150mm)	m3	608
Two coat 14/7mm spray seal		
Drag Strip	m2	10,666
Return Roads	m2	2,206
Pit Area	m2	6,665
Parking Area	m2	3,684
Stopping Bed		
Sand (91m x 18.5m x 0.3m)	m3	505
Spectator Protection		
Guardrail (total length including 4x terminals)	m	560
Fencing	m	331
Erosion & Sediment Control		
Earth bund (1522m)	m3	380
Level spreader	ea.	2
Straw bales / coconut fibre logs (group)	locations	64

REV. DESCRIPTION DATE REGIONAL CLIENT: PROJECT: SCALE: D C AMENDED FOR COUNCIL APPROVAL 25-08-23 REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404 DATUM: AHD WESTERN WHEELERS MOT FOR COUNCIL APPROVAL BLAND SHIRE COUNCIL 22-06-23 PROJECTION: GDA2020 (ZONE 55) В FOR REVIEW 19-06-23 SURVEYED BY: А DRAFT - FOR COUNCIL COMMENT 17-04-23

ITIES	
-------	--

	SHEET No.	1	SHEETS	35				
TORSPORT PARK	SHEET TITLE:							
	NOTES &	BILL (of quant	ITIES				







	593				_
200		LU 505 X 0 505)))	340	
	/		/		
/	/				
				I	
	6.36	292		202	
			697		_
					=
		>>			
-1.287	-0.838	-0.359			
	0-	0-			
5	ر	 <u>ب</u>			
262.315	262.415	262.515			
263.602	263.253	262.874			
263	263	262.			
00	00				
300.00	320.00	340.00			
		SHEET No.	4	SHEETS	35
TORSPO	RT PARK	SHEET TITL	E:		
			G STF		& 340)
		LONG	32011	ON (-20 - 3	540)





H GEOM EXIST. CL								L=640 B=278°4		1									
CUT / FILL	-0.359	+0.102	+ 0.591	+0.759	006.0+	+1.022	+1.102	+1.182	+1.244	+1.263	+1.263	+1.501 +1.449	+1.507	+1.576	+1.791	+1.966	+2.193	+2.268	+2.377
DESIGN CENTRELINE	262.515	262.615	262.715	262.815	262.915	263.015	263.115	263.215	263.252 263.252	263.263	263.263	263.356	263.383	263.415	263.515	263.615	263.715	263.757	263.815
	262.874	262.513	262.124	262.056	262.015	261.993	262.013	262.033	262.008 262.008	262.000	262.000	261.907	261.876	261.839	261.724	261.649	261.522	261.489	261.438
CHAINAGE	340.00	360.00	380.00	4 00.00	4 20.00	440.00	460.00	480.00	487.44	4.01.40	489.53	508.28	513.67	520.00	540.00	560.00	580.00	588.41	600.00

SCALE:	1:1000H 1:100V	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:
JUALE.	1.100011 1.1000				engineering		
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		WESTERN WHEELERS MOTO
_		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL	
PROJECTION:	GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23	COOTAMUNDRA NSW 2590		
SURVEYED BY:		Α	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404		

	->
+2.3 <i>ff</i> +2.480	+2.572
263.872 263.872	263.915
611.41 261.392 263.872 61.872	261.343
600.00 611.41	620.00
TORSPOF	RT PARK SHEET NO. 5 SHEETS 35 SHEET TITLE: DRAG STRIP PLAN & LONG SECTION (340 - 620)

-0.4% 1 in 6.7 0.4% -0.4% -6.4% -6.6% 2.9% 7.8% -RL256.4m RL256.3m 0.000 -0.455 -0.468 0.285 0.288 -0.047 0.340 0.333 0.168 0.152 -0.539 0.098 0.263 .172 o o 260.65 260.23 260.23 260.97 260.98 260.77 260.78 260.85 260.86 260.28 260.28 260.86 260.39 260.81 2 2 260.1 260.1 260.654 260.681 260.694 260.689 260.690 260.366 260.275 260.684 260.704 260.816 260.820 260.905 260.288 260.434 260.445 260.552 316 331 260. 260. 40.00 18.39 16.25 15.25 10.25 9.25 40.00 26.82 20.76 19.76 10.25 9.25 0.00 9.25 19.25 20.25 40.00

CH 0.00



0.00 CH 40.00 -0.4% 0.4% RL256.1m 0.000 -0.521 -0.538 0.269 0.261 0.216 260.33 259.85 259.85 260.87 260.88 260.91 260.332 260.373 260.390 260.416 260.605 260.617 260.699

CH 20.00

0.00

10.25 9.25

C:\Use											
SCALE:	1:500H 1:250V	REV.	DESCRIPTION	DATE	REGIONAL engineering	CLIENT:		PROJECT:	SHEET No. 6	SHEETS	35
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23 22-06-23	REGIONAL ENGINEERING SERVICES		BLAND SHIRE COUNCIL	WESTERN WHEELERS MOTORSPORT PARK	SHEET TITLE:		TIONO
PROJECTION: SURVEYED BY:	GDA2020 (ZONE 55)	B A	FOR COUNCIL APPROVAL FOR REVIEW DRAFT - FOR COUNCIL COMMENT	19-06-23 17-04-23	PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404		BEAND SHINE COUNCIL		DRAG STRIP CI (-20	ROSS SEC - 40)	TIONS

4 0.00

27.59 25.19 24.19

	_	1 in 10.0		in 50 1.0%	
0.121	0.118	- 0.891	-0.902		-0.004
261.05	261.06	260.16	260.16		261.26
260.931	260.938	261.047	261.058		261.264
9.25	10.25	19.25	20.25		40.00

0.4%

0.151

261.01

864

260.



RL256.5m

CUT/FILL

Design

Strip V0.

SCALE:	1:500H	H 1:250V	REV.			ESCRIPT	ION CIL APPROVA	DA ⁻	E	REGIONAL engineering	CLIENT:			PROJECT	T:			
C:\Users\61435																		
VO neD rive \RES Common \Bla							CH 60.00										CH 100.	00
nd SC/Drag Strip	OFFSET	40.00	18.95	16.25 15.25	10.25	9.25	0.00	9.25 10.25	10 DE	20.25	40.00	40.00	33.43	16.25 15.25	10.25	9.25	0.00	9.25
\De sign	EXISTING	t	260.88	260.822 260.798	260.70	260.721	260.87	261.030 261.050	-cc + 36		261.478	260.960	261.668	260.991 260.960	260.808	260.778	261.012	261.20
VDrag Strip V		01	886	8 2	Ъ													-

			1 in 3.0 6.2%		in 7.1		-0.4%	0.4%		1 in 10.0	- 1 -	in 5.0 1.5%	
RL256.6m										10.0	Ŷ		RL256.
		0.029	- 0.334	-0.313	0.501	0.526	0.277	0.255	0.236	-0.966	-1.002	0.080	
		261.30	260.47	260.47	261.17	261.18	261.21	261.25	261.26	260.36	260.36	261.64	
	260.608	261.268	260.800	260.779	260.673	260.652	260.938	260.99 <i>7</i>	261.020	261.322	261.358	261.559	
	40.00	34.38	16.25	15.25	10.25	9.25	0.00	9.25	10.25	19.25	20.25	40.00	
	L						CH 8	30.00					

1 in 6.9

0.369 0.357

261.07 261.08

0.000 -0.476 -0.452

260.89 260.35 260.35

-0.4%

0.244

261.11

0.4%

0.122 0.106

261.15 261.16



1.1%

-0.014

261.46

1 in 5.0

-0.975 -0.987

260.26 260.26

<u>1 in 10.0</u>



		1 in 4		50 3.3%
		<u>1 in 10.0</u>	1	in 50 3.3%
0.122	0.108	-0.825	-0.853	- 0.051
261.45	261.46	260.56	260.56	262.03
261.330	261.348	261.381	261.409	262.078
9.25	10.25	19.25	20.25	40.00

	SHEET No.	7	SHEETS	35
TORSPORT PARK	SHEET TITLE	•		TIONS

			in 30 1 in 5.0		in 8.0		-0.4%	0.4%		-10.0%		1 in 5.0 1.8%		
RL257.1m														RL25
		0.002	- 0.950	-0.930	-0.132	-0.375	- 0.351	-0.429	-0.435	-1.205	-1.241		0.010	
		261.84	260.95	260.95	261.57	261.58	261.61	261.65	261.66	261.06	261.06		262.84	
	261.701	261.839	261.896	261.876	261.706	261.953	261.966	262.081	262.091	262.261	262.297		262.828	
	40.00	33.37	16.25	15.25	10.25	9.25	0.00	9.25	10.25	16.25	17.25		40.00	

										2.3%
		in 5.0 1 in 8	.5	-0.4%	0.4%		-10.0%		1 in 5.0	
.257.4m										
	0.000	-1.680 -1.695	-1.168 -1.177	-1.253	-1.339	-1.352	-2.053	-2.069		- 0.013
	262.74	261.19 261.19	261.77 261.78	261.81	261.85	261.86	261.26	261.26		263.91
	262.564 262.743	262.866 262.881	262.942 262.955	263.068	263.191	263.208	263.309	263.325		263.920
	40.00	16.25 15.25	10.25 9.25	0.00	9.25	10.25	16.25	17.25		40.00
		1 1								



CH 200.00

		1 in 30		- 1		-0.4%	0.4%		1;		in 5.0 2.2%	
		1 in 50	11	n 1.1					<u>1 in</u> 10.0			
RL257m												RL257.3m
CUT/FILL	0.004	-0.669	-0.626	0.248	0.199	0.116	-0.036	-0.051	-0.977	-1.042	-0.029	
Design	261.61	260.83	260.83	261.4 <i>7</i>	261.48	261.51	261.55	261.56	260.81	260.81	262.43	
EXISTING	261.448 261.601	261.495	261.452	261.226	261.279	261.399	261.588	261.607	261.783	261.848	262.458	262.101
OFFSET	40.00 33.92	16.25	15.25	10.25	9.25	0.00	9.25	10.25	17.75	18.75	40.00	70,00
	1		LI_			CH 1	40.00		1	1		



CH 180.00

SCALE:	1:500H 1:250V	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:	SHEET No. 8	SHEETS	35
JUALL.	1.30011 1.230 V								SHEETS	55
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		WESTERN WHEELERS MOTORSPORT PARK	SHEET TITLE:		
		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL		DRAG STRIP CF	ROSS SECT	IONS
PROJECTION:	GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23	COOTAMUNDRA NSW 2590			(140	- 200)	1
SURVEYED BY:		A	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404			(140	- 200)	

1.5% 1 in 5.0 1 in 5.0 1 in 9.1 -0.4% 0.4% -10.0% RL257.9m RL257.6m -1.576 -1.592 -2.529 0.00.0 -2.082 -2.089 -1.720 -1.826 -1.837 -0.010 262.05 262.06 261.46 261.46 263.42 261.43 261.43 262.01 264.35 261.97 261.98 263.508 263.515 263.550 263.570 263.878 263.893 263.279 263.416 263.735 263.985 264.001 264.359 40.00 10.25 9.25 9.25 16.25 17.25 40.00 26.20 16.25 15.25 0.00

CH 240.00





1 in 5.0 -0.4% 0.4% 1 in 9.5 -2.071 -2.091 -1.664 -1.680 0.000 -1.830 262.07 262.08 261.55 261.55 262.11 263.54 263.542 263.945 263.617 263.637 263.738 263.758 263.444 40.00 10.25 9.25 26.23 0.00 16.25 15.25

CH 260.00

SCALE:	1:500H 1:250V	REV.	DESCRIPTION	DATE	🔊 🔊 🔊 🔊 🔊	CLIENT:	PROJECT:	SHEET No. 9 SHEETS 35
DATUM: PROJECTION: SURVEYED BY:	AHD GDA2020 (ZONE 55)	D C B	AMENDED FOR COUNCIL APPROVAL FOR COUNCIL APPROVAL FOR REVIEW DRAFT - FOR COUNCIL COMMENT	25-08-23 22-06-23 19-06-23 17-04-23	REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590	BLAND SHIRE COUNCIL	WESTERN WHEELERS MOTORSPORT PARK	SHEET TITLE: DRAG STRIP CROSS SECTIONS (221.17 - 280)

SC\Drag Strip\Design\Drag Strip VC

				1.2%	
	-10.0%		1 in 5.0		
-1.657	-2.315	-2.327			0.000
262.25	261.65	261.65			264.25
9.25 263.909	263.967	263.979			4 0.00 264.253
9.25	15.25	16.25			4 0.00



1 in 5.0 -0.4% 0.4% in 5.0 -7.9% -10.0% 0.4% -0.4% -7.2% RL258.1m RL258.3m -1.176 -1.184 -0.776 -1.564 -1.575 0.003 0.000 838 -0.900 0.000 -0.228 -0.237 0.144 0.102 o' 261.91 261.91 263.63 261.85 261.85 263.03 41 262.45 262.38 262.37 262.15 262.15 262. 262.58 61 262. 263.416 263.427 263.630 262.869 263.032 263.082 263.090 263.253 263.352 263.154 262.365 262.374 262.383 262.168 262.513 262.434 40.00 40.00 21.88 0.00 15.25 16.25 16.25 15.25 9.25 9.25 40.00 0.00 17.34 16.25 15.25 9.25 CH 320.00 CH 360.00 0.8% 1 in 5.0 1 in 5.0 -0.4% 0.4% -0.4% 0.4% -8.2% -10.0% -7.5% RL258m RL258.2m -1.640 -1.651 0.000 -0.706 -0.714 0.000 -1.287 -2.044 -2.052 0.000 -0.312 -0.359 -1.224 -1.394 CUT/FILL 264.00 262.03 262.03 263.35 261.79 261.79 262.28 262.31 262.35 261.75 261.75 262.70 262.48 262.51 Design 263.796 263.804 263.999 263.350 263.746 262.527 262.732 262.740 263.602 262.790 262.874 .426 .437 263.502 262.704 201 EXISTING 263. 263.¹ 263.1 15.25 16.25 40.00 40.00 24.07 0.00 9.25 16.25 15.25 0.00 16.25 15.25 19.64 9.25 40.00 9.25 OFFSET CH 300.00 CH 340.00

0.8%

SCALE:	1:500H 1:250V	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:	SHEET NO. 10 SHEETS 35
JUALE.	1.30011 1.2307				engineering			
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		WESTERN WHEELERS MOTORSPORT PARK	SHEET TITLE:
		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL	WESTERN WHEELERS WUTURSPURT PARK	DRAG STRIP CROSS SECTIONS
PROJECTION:	GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23				(300 - 360)
SURVEYED BY:		Α	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404			(300 - 300)

	-10.0%		0.9%	
0.060	-0.594	-0.603		-0.001
262.65	262.05	262.05		262.86
262.592	262.646	262.655		262.863
9.25	15.25	16.25		40.00





CH 400.00





CH 420.00

C:\Us									
SCALE:	1:500H 1:250V	REV.	DESCRIPTION	DATE	REGIONAL engineering	CLIENT:	PROJECT:	SHEET No. 11	SHEETS 3
DATUM: PROJECTION: SURVEYED BY:	AHD GDA2020 (ZONE 55)	D C B	FOR REVIEW	19-06-23	REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590		BLAND SHIRE COUNCIL WESTERN WHEELERS MOTORSPORT PARK	SHEET TITLE: DRAG STRIP C (380	ROSS SECTIC - 440)
SURVETED BT.		A	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404			(***	,

_	-7.5%				1.1%
0.948		-0.313	-0.324	0.000	- 0.001
263.052		261.946	261.946	262.288	262.428
262.104		262.259	262.270	262.288	262.430
9.25		23.92	24.92	26.63	4 0.00

_	-8.2%			1.0%
0.868	-0.283	-0.293	0.000	0.000
262.952	261.932	261.932	262.242	262.407
9.25 262.084	262.215	262.226	262.242	262.407
9.25	21.75	22.75	24.30	40.00

1 in 10.0 -6.3% 1 in 10.0 1.1% RL257.6m RL257.6m -0.256 -0.267 0.000 0.000 1.261 1.182 1.12*7* 0.000 0.000 1.637 1.576 262.053 262.053 262.335 261.739 263.178 263.215 263.252 262.434 261.54 263.38 263.41 262.309 262.320 262.335 262.434 261.513 261.739 262.033 262.125 261.433 261.535 261.741 261.839 261.917 4 0.00 4 0.00 4 0.00 9.25 0.00 9.25 28.26 29.26 30.67 27.68 9.25 0.00 23.64 CH 480.00 CH 520.00 -0.4% 0.4% -0.4% 0.4% 1 in 10.0 -6.9% -10.0% 0.9% RL257.6m -0.351 -0.362 0.000 0.000 1.023 -0.002 0.000 1.361 1.180 1.102 1.421 CUT/FILL 261.986 261.986 262.368 263.152 263.278 261.732 263.078 263.115 262.464 261.681 263.315 DESIGN 262.338 262.348 262.368 262.466 261.732 262.013 262.129 261.483 261.681 261.898 261.857 261.954 261.519 EXISTING 40.00 40.00 22.71 0.00 26.09 27.09 29.00 0.00 9.25 25.22 9.25 9.25 40.00 OFFSET CH 460.00 CH 500.00

-0.4%

0.4%

SCALE:	1:500H 1:250V	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:	SHEET NO. 12 SHEETS 35
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES			SHEET TITLE:
PROJECTION:	GDA2020 (ZONE 55)	C	FOR COUNCIL APPROVAL FOR REVIEW	22-06-23 19-06-23	PO BOX 702 COOTAMUNDRA NSW 2590	BLAND SHIRE COUNCIL	WESTERN WHEELERS MOTORSPORT PARK	DRAG STRIP CROSS SECTIONS
SURVEYED BY:		A	DRAFT - FOR COUNCIL COMMENT	17-04-23				(460 - 520)

	-7.5%	8	9% 0.5%
1.515	-0.249	-0.256	0.000
263.45	261.87	261.87	262.16
9.25 261.937	262.118	262.125	262.164
9.25	30.49	31.49	4 0.00

-0.4%

0.4%

		_	\neg					П
]	
0.930	0.453	0.073	0.050	-0.207	-0.171	0.171	0.384	0.424
263.113	262.685	262.321	262.301	262.050	262.098	262.456	262.725	262.783
262.183	262.232	262.248	262.251	262.257	262.268	262.286	262.341	262.359
22.97	28.02	29.60	29.86	30.43	31.48	33.12	38.26	40.00
	262.183 263.113	262.183 263.113 262.232 262.685	262.183 263.113 262.232 262.685 262.248 262.321	262.183 263.113 262.232 262.685 262.248 262.321 262.248 262.321 262.251 262.301	262.183 263.113 262.183 263.113 262.232 262.685 262.248 262.321 262.251 262.301 262.257 262.301	262.183 263.113 262.183 263.113 262.232 262.685 262.248 262.321 262.251 262.301 262.257 262.050 262.258 262.058	262.183 263.113 262.232 262.685 262.232 262.685 262.248 262.321 262.251 262.301 262.257 262.050 262.268 262.098 262.286 262.098	262.183 263.113 262.232 263.113 262.232 262.685 262.248 262.685 262.251 262.321 262.257 262.321 262.258 262.098 262.286 262.098 262.286 262.456 262.286 262.038 262.286 262.038



FOR REVIEW

DRAFT - FOR COUNCIL COMMENT 17-04-23

А

SURVEYED BY:

19-06-23

	-9.9%			
2.151	- 0.216	-0.192	0.000	0.000
261.601 263.752	261.623	261.661	261.881	261.938
	261.839	261.853	261.881	261.938
9.25	30.98	32.26	34.84	40.00

	-9.2%		1.1%
1.948	-0.171	0.000	0.000
263.652	261.677	261.890	261.948
9.25 261.704 263.652	261.848	261.890	261.948
9.25	30.86	34.73	40.00

	SHEET No.	13	SHEETS	35
TORSPORT PARK	SHEET TITLI DRAG ST	RIP CR	OSS SEC 7 - 580)	TIONS





REGIONAL engineering s e r v i c e s REV. DESCRIPTION DATE CLIENT: PROJECT: SCALE: 1:500H 1:250V D C B AMENDED FOR COUNCIL APPROVAL FOR COUNCIL APPROVAL 25-08-23 REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590 PHONE 0407 351 404 DATUM: AHD WESTERN WHEELERS MO BLAND SHIRE COUNCIL 22-06-23 PROJECTION: GDA2020 (ZONE 55) FOR REVIEW 19-06-23 SURVEYED BY: Α DRAFT - FOR COUNCIL COMMENT 17-04-23

	SHEET No.	14	SHEETS	35					
TORSPORT PARK		SHEET TITLE:							
IUKSFURI FARK	DRAG STRIP CROSS SECTIONS								
			616.17)						



		A				
300	/		340	/	360 7	
+ +			765			
- 244 - 342 		110	101			
			-0-0-		8	
EL2.265.213	00					
		-2.000%			>>> 	
+0.305 +0.301	+0.301 +0.264		+0.206	+0.187	+0.156	
264.742 264.741	264.643		264.393	264.213	264.012	
264.437 264.440	264.379		264.187	264.026	263.856	
297.00 299.96	320.00		340.00	349.96	360.00	
)TORSPORT F	PARK			PLAN (35 IG



CULVERT No.4

450Ø RCP CL4 RRJ LENGTH 7.32m, GRADE 1% (FALL WEST) INLET INV. 261.35, OUTLET INV. 261.275 NEW CULVERT



	SHEET No.	16	SHEETS	35
TORSPORT PARK		D 2 PL	AN & LON (360 - 560	

RL260.5m RL260.6m 263.08 263.11 16 263.11 263.08 262.95 262.98 262.98 262.95 263.02 263. 0.921 0.978 0.923 0.886 0.934 0.944 .601 0.709 0.775 .213 0.724 0.691 0.736 -0.243 0 -0. 262.177 262.143 159 262.166 262.188 209 195 262.217 262.240 262.252 262.237 262.246 262.255 263 262. 262. 262. 262. 50 0.00 1.50 50 1.50 2.50 50 4.50 2.50 1.50 0.00 1.50 2.50 50 N. 7 CH 27.10 CH 34.70 -3.0% -3.0% -1.0% -3.0% -3.0% 1 in 9,1 -3.0% -3.0% -3.0% -3.0%





-3.0%

-3.0%

-3.0%

-3.0%





CH 24.14

CH 30.00

-3.0% -3.0%

-3.0% -3.0%

SCALE:	1:100	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:
SCALE.	1.100				engineering		
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		WESTERN WHEELERS MOT
		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL	
PROJECTION:	GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23	COOTAMUNDRA NSW 2590		
SURVEYED BY:		A	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404		

	SHEET No.	17	SHEETS	35
TORSPORT PARK		2 CROS	SS SECTIO 42.73)	ONS

CH 40.00

-3.0%	-3.0%	-3.0%		
			1 in 4.0	
262.93	262.88	262.85	262.31	
52.	52.	52.	62	
5	5	2(2	
0.627	0.577	0.546	0.000	0.000
0.6	0.0	0.5	0.0	0.0
			~	
262.301	262.306	262.306	262.308	262.310
52.	5	2.5	2	52.
26	26	26	26	2(
			~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	~
0.00	1.50	2.50	4.68	6.68
0		.7	4	9

## CH 42.73

1:-	-3.0%	-3.0%	-3.0%
1 in 4.0			
262.33	262.81	262.84	262.88
0.0000000000000000000000000000000000000	0.473	0.502	0.556
262.332 262.331 262.331	262.333	262.334	262.325
4.40 6.40	2.50	1.50	0.00

23 12.0151 PM	-	8.08 262.431	6.08 262.433	5.00 262.434 4.50 262.435		2.50 262.437	1.50 262.439	0.00 262.440	1.50 262.438	2.50 262.432	50 262	5.00 262.416	5.99 262.410	707 66		8.98 262.587	6.98 262.579	5.00 262.569 4.50 262.564	.50 262.54	50 262	0.00 262.519	1.50 262.504	2.50 262.494	4.50 262.475 r oo 2707 70	5.00 262.470 6.49 262.455	8.49 262.435	
Plotted 25/08/20	L				1			CH 53.40	)	L		J I			L		<b>I</b>			1 1	CH 62	.87					1
					1 in 4.0	-3	.0% -3	3.0%	3.0% -	3.0%	1 in 4.0		-0.5%					1-110 4-10	1 in 4:0	-2.9320%	-3.0%	-3.0%	-3.0%	1 in 4.0	1 in 4.0	-0.9%	
	RL260.7	m													RL260.6m												
V0.1.dwg	Design		262.38	262.20 262.20		262.70	262.73	262.78	262.73	262.70	262.20	262.20	262.39				262.53	262.10 262.10	262.60	262.63	262.68	262.63	262.60	262.10	262.44		
lesign\Drag Strip VI	CUT/FILI	0.194	0.239	0.255		0.299	0.335	0.375	0.326	0.299	- 0.192	-0.190	0.000				-0.093	-0.434	0.081	0.117	0.176	0.14.4	0.123	-0.359	0.000	0.000	
and SC\Drag Strip\C	EXISTING	262.377	262.383	262.386 262.387		262.393	262.397	262.401	262.406	262.403	262.393	262.391	262.388 262.378			262.540	262.535	262.530 262.529	262.524	262.515	262.502	262.488	262.479	262.461	dC.4.202 262.444	262.426	
Jrive\RES Common\BI	OFFSET	7.73	5.73	5.00		2.50	1.50	0.00	1.50	2.50	4.50	5.00	7.74			8.73	6.73	5.00	2.50		0.00	1.50	2.50	4.50	00.c 6.37	8.37	
C:\Users\61435\0nel				I				CH 50.00	)	1		ı I		I		L		I	1	ı I	CH 60	.00			<u> </u>		
PRO	UM:	1:100 AHD GDA20	020 (ZONE	55)	C B	F	DED FOR OR COUL FOF	COUNCIL A NCIL APPRO REVIEW COUNCIL C	APPROV/ OVAL	AL 25	2-06-23	IONAL E COOTA	REGIONAL Engineering E R V I C E S NGINEERING SERVI 20 BOX 702 NUNDRA NSW 2590 NE 0407 351 404	ICES	CLIENT: BLA	ND SHIF	E COUN		PROJECT: WESTE	RN WH	EELERS	MOTOR	RSPOF	RT PARK	SHEET TITLE ROAD 2	18 SHEE CROSS SE (50 - 62.87)	ECTIONS

RL260.6m 262.41 262.43 262.16 262.16 262.66 262.69 262.69 262.66 262.16 262.16 262.74 -0.257 -0.254 0.134 0.165 0.178 0.229 0.000 0.253 0.297 0.254 0.230 -0.287 262.419 262.416 262.431 262.433 262.434 262.435 262.437 262.439 262.438 262.432 262.410 262.440

-3.0%

--3208%

1 in 4.0

-2.9%

1 in 4.0

-3.0%

-3.0%

1 in 4.0 -0.6%

1 in 4.0 RL260.5m 262.08 262.08 262.58

0.000

0.000

262.61	262.58	262.08	262.08	262.45		
0.109	0.089	- 0.391	-0.386	0.000	0.000	
262.504	262.494	262.475	262.470	262.455	262.435	
1.50	2.50	4.50	5.00	6.49	8.49	

-3.0%	-3.0%	1 in 4.0		. 0		
		- 11 4.0		1 in 4.0		
						_
262.61	262.58	262.08	262.08	262.45		
262	262	262	262	262		
60	39	91	36	00	U	2
0.109	0.089	- 0.391	-0.386	0.000		5.0
	Ŭ	1	Ĭ	0		-
262.504	262.494	262.475	262.470	262.455	35.7 C9C	י ר +
52.5	52.4	52.4	52.1	52.4		
26	26	56	26	2(	2	7
1.50	20	00	0	6+	σ	
<u> </u>	2.50	4.50	5.00	6.49	07 X	г 5

-3.0%

262.66

0.139

-3.0%

262.61

0.079

1 in 4.0

-0.485 -0.480

262.58

0.040

		262.6	262.6	262.6 262.5	262.5	262.5	262.5	262.54	262.5	262.5 262.5	262.5			262.60	262.58	262.5	262.56	262.54	262.53	262.5	262.50	262.49	262.4	262.45		262.43
023 12:01:51 PM		9.24	7.24	5.00	2.50	1.50	0.00	1.50	2.50	4.50	6.78	878	5	8.64	6.64	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50 5.00	6.10		8.10
Plotted 25/08/2							CH 80	).00												CH 12	20.00					
	RL260.5m			1 in 4.0	1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0	1 in 4.9	-1.1%	RL260.6m			1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0	1 in 4.0	-1.0	%
0.1.dwg	Design		262.61	262.07 262.07	262.57	262.60	262.64	262.60	262.57	262.07 262.07	262.46				262.61		262.12	262.62	262.65	262.69	262.65	262.62	262.12 262.12		06.202	
Design/Drag Strip V	CUT/FILL		0.000	-0.521	0.006	0.046	0.107	0.078	0.059	-0.420 -0.414	0.000	0.000			0.000	-0.474	-0.470	0.047	0.085	0.14.2	0.110	0.088	-0.396		0.00	-0.004
Iland SC\Drag Strip\	EXISTING	262.632	262.611	262.588 262.582	262.561	262.550	262.534	262.518	262.508	262.486 262.481	262.464	262.443		262.626	262.609	262.593	262.589	262.572	262.564	262.552	262.539	262.531	262.515 262.510		202.4.90	262.481
Drive\RES Common\E		9.18	7.18	5.00 4.50	2.50	1.50	0.00	1.50	2.50	4.50	6.59	8.59		8.96	6.96	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50 5.00		70.0	8.52
C:\Users\614.35\One							CH 66	5.20											I	CH 10	)0.00					
SCALE	Ξ:	1:100		REV	. I	DESCRIP	TION		DATE	REC S E F	GIONAL ineering	CLIENT:				PROJE	CT:						SHEET No.	-	SHEETS	35
PROJI	DATUM:     AHD     D     AMENDED FOR COUNCIL APPROVA       PROJECTION:     GDA2020 (ZONE 55)     C     FOR COUNCIL APPROVAL       SURVEYED BY:     B     FOR REVIEW				22 19	5-08-23 2-06-23 0-06-23 7-04-23	REGIONAL ENGINI PO BC COOTAMUND PHONE 04	CES	BLAND SHIRE COUNCIL WESTERN WHEELERS MOTORSPORT PARK					SHEET TITLE: ROAD 2 CROSS SECTIONS (66.2 - 120)												

		1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0		1 in 4.0	-1.1%
RL260.5m												
	262.62	262.06	262.06	262.56	262.59	262.63	262.59	262.56	262.06	262.06	262.50	
	0.00.0	-0.54.2	-0.537	-0.021	0.017	0.075	0.042	0.020	-0.463	-0.459	0.000	-0.005
	262.636 262.619	262.601	262.596	262.580	262.572	262.559	262.547	262.539	262.522	262.518	262.503	262.487
	9.24 7.24	5.00	4.50	2.50	1.50	00.0	1.50	2.50	4.50	5.00	6.78	8.78



_	-3.0%	-3.0%	-3.0%	-3.0%	1 in (		. 0	-1.0%	
					1 in 4.0		1 in 4.0	-1.0%	
	262.71	262.75	262.71	262.68	262.18	262.18	262.45		
	0.175	0.235	0.206	0.187	-0.292	-0.287	0.000		0.001
	262.534	262.519	262.503	262.492	262.471	262.466	262.455		262.434
	1.50	0.00	1.50	2.50	4.50	5.00	6.10		8.10

				1 in 4.0	1 1	1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0		-1.1%	Γ		1 in	4.0	1 in 4.0	-3.0%	-3.0%
	RL260.8	m												RL261.3m						
				262.60 262.35	262.35	262.85	262.88	262.92	262.88	262.85	262.35	262.47 262.47		-		263.20	262.86 262.86	263.36	263.39	263.44
				0.000 -0.240	-0.235	0.288	0.329	0.391	0.363	0.344	-0.133	-0.121	0.001	-		0.000	-0.328	0.195	0.234	0.292
		262.623		262.600 262.589	262.583	262.560	262.549	262.532	262.515	262.504	262.481	262.470 262.470	262.447		263.220	263.202	263.190 263.185	263.167	263.158	263.145
23 12:01:52 PM		8.01	Č	6.01 5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50	5.49	7.49		8.36	6.36	5.00	2.50	1.50	00.0
Plotted 25/08/2023 12:01:52 PM					11		11	CH 16	50.00					L				1	11	CH 2
	Γ			1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0	1 in	<b>.0</b> -1.0%	٦		1	n 4.0	1 in 4.0	-3.0%	-3.0%
	RL260.7m													RL261	m					
0.1.dwg	Design		262.59	262.24	262.24	262.74	262.77	262.81	262.77	262.74	262.24	ZDZ.Z4	262.46			262.79	262.56 262.56	263.06	263.09	263.13
Jesign/Drag Strip V	CUT/FILL		0.000	-0.338	-0.333	0.190	0.231	0.293	0.265	0.246	-0.232	-0.22b	0.000	<u>,</u>		0.000	-0.229	0.292	0.331	0.389
Vusers/61435/OneDrive/RES Common/Bland SC/Drag Strip/UDesign/Drag Strip V0.1.dwg	EXISTING	262.616	262.593	262.577	262.572	262.549	262.538	262.521	262.504	262.492	262.471	C04.202	262.456 361.620	nn t. 707	262.812	262.794	262.785 262.781	262.764	262.755	262.742
Drive\RES Common\B	OFFSET	8.4.2	6.4.2	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50	00.c	18.C 5.87 5.05	50.	7.95	5.95	5.00	2.50	1.50	0.00
s ers \61435\0 nel			I		<u> </u>		ıl	CH 14	40.00				1		L	1	1		II	CH 1

eDrive\R																				
C.\Users\614.35\0n					CH 1	40.00												CH 1	180.00	
SCALE:	1:200H 1:100	V	REV.	D	ESCRIPTIO	N	D	ATE	$\sim$	engi	neering	CLIEN	T:			PROJECT:				
DATUM: PROJECTION: SURVEYED BY:	AHD GDA2020 (ZO	NE 55)	 C B	FOR CC	OR COUNCI OUNCIL APP OR REVIEV R COUNCIL	PROVAL V	22- 19-	08-23 06-23 06-23 04-23	REGIONAL	ISER ENGINE POBO AMUNDI	VICES ERING SERVIC	ES	BLANI	O SHIRE COUN	CIL	WEST	ERN \	NHEELE	ERS MO	т

TORSPORT PARK

SHEET No. 20 SHEETS 35 SHEET TITLE: ROAD 2 CROSS

SECTIONS (140 - 200)

263.09	263.06	262.56	262.56	262.69	
0.357	0.335	-0.147	-0.143	0.000	0.000
262.729	262.720	262.703	262.699	262.694	262.677
1.50	2.50	4.50	5.00	5.55	7.55

-0.9%

263.39	263.36	262.86	262.86	263.09	
0.261	0.240	-0.242	-0.237	0.000	0000
263.131	263.122	263.104	263.099	263.091	263.073
1.50	2.50	4.50	5.00	5.92	79. F

-3.0%	-3.0%	1 in 4.0		. 0	-0.9%
		7.0		1 in 4.0	0.770
263.39	263.36	262.86	262.86	263.09	
0.261	0.240	-0.242	-0.237	0.000	0.000
263.131	263.122	263.104	263.099	263.091	263.073
1.50	2.50	4.50	5.00	5.92	7.92

CH 200.00

-3.0%

-3.0%

1 in 4.0

| RL262m     |                                            |                                                                                                                                                                                                                                                                                                                                                           |                                          |                                                                                                                                                                                                                                                                          |                                                                                                                                                                                                                                                                                            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                                                                |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                     |
| F          | RL261.7m<br>Design<br>CUT/FILL<br>EXISTING | RL261.7m         65:92           RL261.7m         65:92           CUT/FILL         65:92           EXISTING         65:92           00:00         14:00           CUT/FILL         00:00           EXISTING         15:8           0FFSET         15:8           CALE:         1:100           ATUM:         AHD           ROJECTION:         GDA2020 (ZO | S6 E S S S S S S S S S S S S S S S S S S | RL261.7m         1/m 4.0         S6323 0000 0         S63233         CALE:         1:100         ATUM:         AHD         ROJECTION:         GBA2020 (ZONE 55) | S6:69:00         S6:59         S6:59 | Set         Set <td>RL261.7m           Image: Second colspan="2"&gt;Second co</td> <td>RL261.7m         95         95         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         &lt;</td> <td>RL261.7m         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         00000         0000         0000</td> <td>RL261.7m         S3         &lt;</td> <td>RL261.7m         Sign 22<br/>(200000         Sign 22<br/>(2000000         Sign 22<br/>(2000000         Sign 22<br/>(20000000         Sign 22<br/>(20000000         Sign 22<br/>(20000000         Sign 22<br/>(20000000         Sign 22<br/>(20000000         Sign 22<br/>(20000000         Sign 22<br/>(200000000         Sign 22<br/>(20000000000000000000000000000000000</td> <td>RL2617m         S23         S23</td> <td>RL261.7m           RL261.7m           RL261.7m           RL261.7m           RL261.7m           CH 2000           <th co<="" td=""><td>Image: second second</td><td>Image: Section of the sectio</td><td>Image: second second</td><td>RL261.7m         RL261.7m         RL262.7m         RL262.7m</td><td>Image: state state</td><td>Image: state state</td><td>NUM         NUM         NUM</td></th></td> | RL261.7m           Image: Second colspan="2">Second co | RL261.7m         95         95         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         97         < | RL261.7m         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         0000         00000         0000         0000 | RL261.7m         S3         < | RL261.7m         Sign 22<br>(200000         Sign 22<br>(2000000         Sign 22<br>(2000000         Sign 22<br>(20000000         Sign 22<br>(20000000         Sign 22<br>(20000000         Sign 22<br>(20000000         Sign 22<br>(20000000         Sign 22<br>(20000000         Sign 22<br>(200000000         Sign 22<br>(20000000000000000000000000000000000 | RL2617m         S23         S23 | RL261.7m           RL261.7m           RL261.7m           RL261.7m           RL261.7m           CH 2000           CH 2000 <th co<="" td=""><td>Image: second second</td><td>Image: Section of the sectio</td><td>Image: second second</td><td>RL261.7m         RL261.7m         RL262.7m         RL262.7m</td><td>Image: state state</td><td>Image: state state</td><td>NUM         NUM         NUM</td></th> | <td>Image: second second</td> <td>Image: Section of the sectio</td> <td>Image: second second</td> <td>RL261.7m         RL261.7m         RL262.7m         RL262.7m</td> <td>Image: state state</td> <td>Image: state state</td> <td>NUM         NUM         NUM</td> | Image: second | Image: Section of the sectio | Image: second | RL261.7m         RL261.7m         RL262.7m         RL262.7m | Image: state | Image: state | NUM         NUM |

1 in 4.0

1 in 4.0

-0.8%

1 in 4.0

1 in 4.0

-3.0%

-3.0%

-3.0%

-3.0%

## WESTERN WHEELERS MOTORSPORT PARK

SHEET No. 21 SHEETS 35 SHEET TITLE: ROAD 2 CROSS

SECTIONS 220 - 280)

264.44	264.41	263.91	263.91	264.21		
0.195	0.173	- 0.310	-0.306	0.000	-0.009	
264.244	264.236	264.219	264.215	264.205	264.189	
1.50	2.50	05.4	5.00	6.19	8.19	

ſ	2		4	5	S		7	
0.00								
-3.0%	-3.0%	1 in 4.0			1 in 4.0	-1.3%		
					1			
264.44	264.41		263.91	263.91	761. 21	17:407		
								-

0%	-3.0%	1 in 4.0			•	-1.2%	
			/		1 in 4.0	- 1.270	
4	5		1	-	4		
264.64	264.61		264.11	264.11	264.34		
26	26		26	26	26		
0.250	0.231		-0.246	40	0.000		0.000
0.2	0.2		0.2	-0.240	0.0		0.0
			I	1			
е	32		58	m	+2		19
m.	m.		Ľ.	٣.	.34		m.
264.393	264.382		264.358	264.353	264.342		264.319
1.50	2.50		4.50	5.00	5.92		7.92
-	5		4	പ	0		f

-3.0% -3.0%

1 in 4.0

1 in 4.0

-3.0%

RL262.5m														RL261.9m					
		264.49	264.07	264.07	264.57	264.60	264.64	264.60	264.57	264.07	264.07	264.28				263.89	263.44	263.44	
-		0.000	-0.393	-0.385	0.147	0.194	0.263	0.243	0.229	-0.238	-0.230	0.000	0.000			0.000	-0.442	-0.440	
-	264.521	264.489	264.461	264.453	264.420	264.404	264.379	264.355	264.338	264.306	264.297	264.283	264.251		263.896	263.887	263.879	263.877	
-	8.68	6.68	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50	5.00	5.86	7.86		8.80	6.80	5.00	4.50	
							CH 32	20.00											
						-3.0%	-3.0%	-3.0%	-3.0%										
			1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0		<u>1 in 4.0</u>	-1.1% -1.3%				1 in 4.0		
RL262.6 Design	m	264.49	1 in 4.0 564.11	264.17	1 in 40 564.67	264.70	-3.0% 564.744	-3.0%	-3.0%	1 in 4.0 264.17		264.40	-1.1% -1.3%	RL262.3n	1	264.22		263.82	
		0.000 264.49		-0.308 264.17							264.17		-1.1% -1.3% 600.0 -	RL262.3n		0.000 264.22	263.82		
Design	202 L		264.17		264.67	264.70	264.74	264.70	264.67	264.17	-0.237 264.17	564.40		RL262.3n	264.233		-0.398 263.82	-0.396	

CH 300.00

-3.0%

-3.0%

in 4.0

1 in 4.0

-3.0% -3.0%

1 in 4.0 -1.6% -1.6%

CH 340.00

c:\Us								
SCALE:	1:100	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:		PROJECT:
SCALE.	1.100				engineering			
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	- REGIONAL ENGINEERING SERVICES			WESTERN WHEELERS MO
-		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702		BLAND SHIRE COUNCIL	
PROJECTION:	GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23				
SURVEYED BY:		Α	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404			

# IOTORSPORT PARK

SHEET No. 22 SHEETS 35 SHEET TITLE: ROAD 2 CROSS SECTIONS (300 - 360)

-3.0%	-3.0%	1 in .			4.504	0.000
		1 in 4.0		1 in 4.0	-1.5%	-2.0%
264.35	264.32	263.82	263.82	264.06		
264	264	263	263	264		
0.194	0.186	-0.270	-0.259	0.000		0.010
0.	0.	0 1	0. 1	o.		0
264.154	264.132	264.089	264.078	264.057		264.013
264.	264	.64.	64.	.64.		264
1.50	2.50	4.50	5.00	5.95		7.95

## CH 360.00

-3.0% -3.0%

263.97

0.104

263.863

1.50

-3.0%

264.35

0.148

264.201

1.50

263.94

0.069

263.868

2.50

264.32

0.113

264.205

2.50

1 in 4.0

264.01

0.156

263.856

0.00

-3.0%

264.39

0.207

264.187

0.00

1 in 4.0

1 in 4.0

-3.0%	-3.0%	1 in 4.0		1 in 4.0	-2.8%
		7.0		1 m 3	
263.97	263.94	263.44	263.44	263.79	
0.118	0.092	-0.398	-0.386	0.000	-0.010
263.850	263.845	263.835	263.823	263.790	263.744
1.50	2.50	4.50	5.00	6.41	8.41

			1 in 4.0	1 in	4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0		1.007				1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%
			4.0									-1.8%										
	RL261.1m	ו	262.96 262.64	40.207	263.14	263.17	263.21	263.17	263.14	262.64 252.64	262.70		RL260.3m		262.24	261.89	261.89	262.39	262.42	262.47	262.42	262.39
				C87.U-	0.263	0.317	0.397	0.388	0.382	-0.079		-0.007			0.000	-0.315	-0.305	0.237	0.286	0.360	0.344	0.333
		263.013	262.965 262.934 262.934	776.207	262.874	262.850	262.815	262.779	262.755	262.716 262.709	262.705	262.675		262.276	262.237	262.208	262.198	262.156	262.137	262.108	262.079	262.060
12:01:53 PM		8.31		, 0c.4	2.50	1.50	0.00	1.50	2.50	4.50 7.00		7.27		8.38	6.38		4.50	2.50	1.50	0.00	1.50	2.50
Plotted 25/08/2023 12:01:53							CH 4(	00.00												CH 44	40.00	
PI			1 in 4.0	1 in	4.0	-3.0%	-3.0%	-3.0%	-3.0%	<u>1 in 4.0</u>		-1.9%				1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%
	210/15		4.0									- 1. 7/6				4.0						
бмр	RL261.5m Design		263.04	203.04	263.54	263.57	263.61	263.57	263.54	263.04 253.04	263.15		RL260.7m		262.56	262.24	262.24	262.74	262.77	262.81	262.77	262.74
\Design\Drag Strip V0.1.dwg	CUT/FILL		0.000 -0.322	- 0.512	0.229	0.280	0.355	0.341	0.331	- 0.128 - 0.18	0.000	0.004			0.000	-0.299	-0.289	0.252	0.303	0.379	0.357	0.342
on/Bland SC/Drag Strip/De	EXISTING	263.434	263.359 263.359	203.349	263.308	263.287	263.257	263.226	263.206	263.165 263.155	263.146	263.105		262.604	262.563	262.536	262.526	262.485	262.464	262.433	262.410	262.395
eDrive\RES Common\Bla	OFFSET			4 0 0	2.50	1.50	00.0	1.50	2.50	4.50				8.30	6.30	00	4.50	2.50	1.50	00.0	1.50	2.50
Js ers \61435\0 neD							CH 38	30.00												CH 42	20.00	
3 S(	CALE:	1:100		REV.		DE	SCRIPTIO	N	D	ATE	RE	EGIONAL	CLIENT:					PROJECT	:			
D/ Pi	ATUM: ROJECTION: JRVEYED BY:	AHD GDA2020 (2	ONE 55)	D C B A	F	OR CO	UNCIL APP	L APPROVA ROVAL / COMMENT	22-0 19-0	06-23 REGIONAL 06-23 06-23 COOT	ENGI PO I AMUN	Gineering R V I C E S INEERING SERVICES BOX 702 NDRA NSW 2590 0407 351 404	E	BLAND SHI	RE CC	OUNCIL		WE	STERN	I WHEEI	LERS MC	DTORSP

-3.0%	1 in 4.0				7 497		
					-3.1%		
262.39		261.89					
0.333		-0.139	-0.132	0.000		-0.035	
2.50 262.060		262.032	5.00 262.025	262.018		7.50 261.991	
2.50		4.50	5.00	5.50		7.50	
-3.0%	1 in 4.0				-2.2%		
					L.L/0		
.74		.24	.24	35			

	1 in 4.0				-2.2%
262.74		262.24	262.24	262.35	
0.342		-0.124	-0.115	0.000	- 0.012
262.395		262.361	262.352	262.345	262.311
2.50		4.50	5.00	5.43	7.43

	SHEET No.	23	SHEETS	35
TORSPORT PARK	SHEET TITLE ROAD 2		SS SECTI	ONS
		(000 -	- ++0)	

			1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0			-1.3%						1 in 4.0	-3.0%	-3.0%	-3.0%
			+.0																			
RL259.9m														RL259.6m								
		261.70	261.48	261.48	261.98	262.01	262.06	262.01	261.98	261.48	261.48	261.60				261.28	261.12	261.12	261.62	26165	26170	
		0.000	-0.206	-0.201	0.314	0.353	0.411	0.379	0.358	-0.125	-0.120	0.000	- 0.005			000.0	-0.150	-0.147	0.367	707 0	0 793	
	261.721	261.699	261.690	261.686	261.671	261.662	261.649	261.635	261.627	261.609	261.605	261.601	261.580		261.292	261.278	261.274	261.271	261.257	261250	261237	
	7.86	5.86	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50	5.00	5.46	7.46		7.62	5.62	5.00	4.50	2.50	150	00 0	
							CH 4	80.00													CH S	520.00

				1 in 4.0		1 in 4.0	-3.0%	-3.0%	-3.0%	-3.0%	1 in 4.0			-2.1%	
	RL260.1m			+.0										2.00	RL259.8m
	RL200.IIII		~	<u>,</u> ,		<u>``</u>	-	. +	6	<u>,</u> ,	\\	<u>,</u>			
-	Design		261.92	261.66	261.66	262.16	262.19	262.24	262.19	262.16	261.66	261.66	261.79		
	GADE TANK		0.000	-0.249	-0.242	0.281	0.322	0.383	0.354	0.335	-0.144	-0.138	0.000	-0.021	-
		261.949	261.923	261.910	261.903	261.880	261.869	261.853	261.837	261.826	261.805	261.799	261.794	261.772	
		8.05	6.05	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50	5.00	5.53	7.53	(     
	nuvee-blockers.unv		<b>I</b>					CH 41	50.00						L
	SCALE:	1:100				REV.		DESCRIPT	ION		DATE		R R	REGIONAL engineering ervices	CLIENT:
⊢											25-08-23		i s	ERVICES	

olotted 25/08/2023 12:01:53

					0	-3.0%	-3.0%	-3.0%	-3.0%	1			
					1 in 4.0					1 in 4.0		_	-1.1%
L259.8m													
		261.49	261.31	261.31	261.81	261.84	261.88	261.84	261.81	261.31	261.31	261.41	
		0.000	-0.176	-0.173	0.340	0.377	0.431	0.396	0.373	- 0.113	- 0.110	0.000	-0.004
	261.502	261.489	261.484	261.481	261.468	261.4.61	261.452	261.442	261.435	261.421	261.418	261.415	261.397
	7.72	5.72	5.00	4.50	2.50	1.50	00.0	1.50	2.50	4.50	5.00	5.43	7.43

CH 500.00

C:\Use											
SCALE:	1:100	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:		PROJECT:	SHEET No. 24	SHEETS 3	35
OUALL.	1.100										55
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL		REGIONAL ENGINEERING SERVICES			WESTERN WHEELERS MOTORSPORT PARK	SHEET TITLE:		
		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702		BLAND SHIRE COUNCIL		ROAD 2 CRO	SS SECTION	١S
PROJECTION:	· · · ·	В	FOR REVIEW	19-06-23	COOTAMUNDRA NSW 2590					- 520)	
SURVEYED BY:		А	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404				(100	- 320)	

-3.0%	-3.0%	1 in 4.0				-0.7%
						-0.1%
261.65	261.62		261.12	261.12	261.19	
0.430	0.409		-0.074	-0.070	0.000	0.002
261.223	261.215		261.198	261.194	261.192	261.175
1.50	2.50		4.50	5.00	5.27	7.27

					1.0%	-2.3%	-2.3%	-2.6%	-2.6%	-1.0%		1 in 4.0		
RL259.2m														
		260.95	261.13	261.13	261.11	261.13	261.17	261.13	261.10	261.08	261.08	260.80		
	0.213	0.000	0.194	0.200	0.206	0.241	0.295	0.274	0.261	0.266	0.272	0.000		0.310
	260.973	260.947	260.938	260.931	260.906	260.893	260.874	260.855	260.842	260.817	260.810	260.795		260.770
	7.74	5.74	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50	5.00	6.15		8.15
							CH 50	50.00			I			
						2.004	-3.0%	-3.0%	2.007					
٢			1 in 4.0		1 in 4.0	-3.0%	- 3.0%	-5.0%	-3.0%	1 in 4.0		-(	).5%	
RL259.3m		-	6	6	6	2	6	7	6	6	6	0		
Design		261.11	260.89	260.89	261.39	261.42	261.46	261.42	261.39	260.89	260.89	261.00		
CUT/FILL		0.000	-0.215	-0.209	0.313	0.354	0.414	0.381	0.359	- 0.125	-0.121			0.006
EXISTING	261.132	261.110	261.100	261.095	261.0 <i>7</i> 3	261.062	261.047	261.035	261.027	261.011	261.007	261.003		260.987
OFFSET	7.90	5.90	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50				7.4.7
							CH 54	40.00			1			1

)							
SCALE:	1:100	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:
JUALE.	1.100				engineering		
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		
-	=	С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL	WESTERN WHEELERS M
PROJECTION:	GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23			
SURVEYED BY:		Α	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404		

itted 25/08/2023 12:01

	SHEET No.	25	SHEETS	35
MOTORSPORT PARK	SHEET TITLE ROAD 2		SS SECTIO	SNC
		(540 -	560)	

							_//		//	- ^	-/ /	TP 62.86	0,	80	06		100	TP 106.26
							¢	0			+	+ +						
						TP 31.4	5				450¢ LENG INLET	VERT No.6 RCP CL4 RRJ TH 7.32m, GRA INV. 261.426, CULVERT	DE 1% (FALI					
							20					4 RRJ .m, GRADE 0.5% 1.223, OUTLET						
023 12.0154 PM			~		LLass 4 RK 4200													
Plotted 25/08/7		0.398%	263.892 20.000	>			-2.15	57%	<			000.09 000.09		1.2119	6	->	«	000'01 P. 262.630
R.L. 2	57.4																	
H GEOM EXIST. C	iL	L=3 B=8°	31.45 59'44"			L=31.42 R=20.00			· · · · ·	В	L=43.40 =98°59'44''			L R:	=34.02 =200.00			B=10
CUT / F	∏ +2.377	+2.329 +2.329 +2.323 +2.300	+2.178 +2.178 +2.176 +2.176 +2.161	+1.892 +1.866	+1.816 +1.523	+1.217	+0.954	+0.887	+0.730	+0.540	+0.417	+0.350	+0.303 +0.284 +0.271	+0.223	+0.217	+0.222 ±0.235	+0.236	+0.257 +0.257 +0.259
DESIGN CENTREL	AU 263.815	263.852 263.852 263.852 263.854 263.858	263.828 263.828 263.828 263.827 263.821	263.676 263.660	263.444	263.228	263.013	262.951	262.797	262.609	262.478	262.403	262.384 262.383 262.383 262.383	262.420	262.484	262.513 262.513	262.569	262.643 262.643 262.643
		261.523 261.523 261.523 261.531 261.558	261.650 261.650 261.651 261.651 261.660	261.784 261.794	261.813 261.921	262.011	262.059	262.064	262.067	262.069	262.061	262.053	<b>262.081</b> 262.099 262.112	262.197	262.267	262.291 267 328	262.333 262.333	262.382 262.384 262.384
	iE ⁰⁰	9.25 9.25 9.25 10.00	19.20 19.25 19.30 20.00	29.25 30.00	64.15 40.00	50.00	60.00	62.86	70.00	80.00	00.06	100.00	106.26 108.43 110.00	120.00	127.44	130.00 134.08	134.61 13051	140.28 140.28 140.28
SCALE: DATUM: PROJECTION SURVEYED B	AHD : GDA2	H 1:100V 2020 (ZONE 55)	C B	MENDED   FOR (	DESCRIPTIO FOR COUNCI COUNCIL APP FOR REVIEV OR COUNCIL	L APPROVAL PROVAL	DATE 25-08-23 22-06-23 19-06-23 17-04-23	GIONAL COOT	REGIO engine s e R V ENGINEERI PO BOX 70 TAMUNDRA 1 HONE 0407 3	REFINE ICES NGSERVICES 02 NSW 2590	CLIENT: BL	AND SHIRE COU	NCIL	PROJECT: WEST	TERN V	VHEE	ELEF	S MO1





	SHEET No.	27	SHEETS	35	
TORSPORT PARK	SHEET TITLE ROAD 3		SS SECTIC - 50)	ONS	





PHONE 0407 351 404

Α

DRAFT - FOR COUNCIL COMMENT 17-04-23

4.50 5.00 5.43	6.43				
		SHEET No.	28	SHEETS	35
DTORSPORT P	ARK	SHEET TITL ROAD 3	3 CRO	SS SECTI 06.26)	ONS

4		0%		
	261.903	261.903	262.009	
	262.018	262.013	262.009	262.000
	4.50	5.00	5.43	6.43

4		0%	1 in 4	
		076		
	261.828	261.828	262.003	
	262.010	262.007	262.003	261.999
	4.50	5.00	5.70	6.70

261.809	261.809	262.053	
262.060	262.058	262.053	262.049
4.50	5.00	5.98	6.98

	0.97	1 in 4	
	0%		
261.809	261.809	262.053	
262.060	262.058	262.053	262.049
4.50	5.00	5.98	6.98

		1 in 4	0%	1 in 4	-3%	-3%	-3%	-3%	1 in 4	0%	1 in 4	
RL260.4m												
	262.314	261.909	261.909	262.409	262.439	262.484	262.439	262.409	261.909	261.909	262.223	
	262.321 262.314	262.302	262.299	262.285	262.278	262.267	262.257	262.249	262.235	262.232	262.223	262.216
	7.62 6.62	5.00	4.50	2.50	1.50	0.00	1.50	2.50	4.50	5.00	6.26	7.26

CH 127.44





SCALE:	1:100	REV.	DESCRIPTION	DATE		CLIENT:	PROJECT:	SHEET No. 29 SHEETS 35
		D	AMENDED FOR COUNCIL APPROVAL	25-08-23	SERVICES			SHEET TITLE:
DATUM: PROJECTION:	AHD GDA2020 (ZONE 55)	C	FOR COUNCIL APPROVAL FOR REVIEW	22-06-23 19-06-23	REGIONAL ENGINEERING SERVICES PO BOX 702 COOTAMUNDRA NSW 2590	BLAND SHIRE COUNCIL	WESTERN WHEELERS MOTORSPORT PARK	ROAD 3 CROSS SECTIONS
SURVEYED BY:		A	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404			(110 - 127.44)

			-3.0%	-3.0%	-3.0% .	-3.0%						-0.4% 0.4%			
_									4.6%	-6.6%				-9.6%	
RL260.6m				450Ø Class 0.509					 RL258.4m			450Ø Class 4 RRJ 0.50%			
CUT / FILL		262.95	262.98	l mil	262.98	262.95			259.99 259.99	260.81	260.82	260.86 260.89	260.90	260.03 260.03	
DESIGN SURFACE	-0.243 261.977	0.709	0.736	I I I I I I I I I I I I I I I I I I I	0.724	0.691	262.050	INVERT -0.213	-0.305 -0.319	259.850 INVERT 0.323	0.312	0.152	0.142	259.972 INVERT -0.812 -0.816	
EXISTING SURFACE	262.217	262.237	262.240	.24	262.252	262.255		262.263	260.293 260.307	260.491	260.506	260.615	260.754	260.842 260.846	
CHAINAGE	4.50 3.66	2.50	1.50	0.00	1.50	2.50	3.66	4.50	23.79	12.20	9.25	0.00	10.25	12.20 19.25 20.25	

CULVERT No.1 (ROAD 2 - CH34.7)

CULVERT No.2 (ROAD 1 - CH 8.06)

SCALES: HORIZONTAL 1:200 VERTICAL 1:100

		1 :		4	4.0		-2.7%	-2.7%	-2.8%	-2.8%		60			
		1 in 4.0	2									+.0		_	
								450Ø CL	ss 4 RRJ		E				
RL258.3m									0%						
CUT/FILL		-0.211	-0.206			0.316	0.353	0.4.05	0.375	0.354		-0.130	-0.126	0.000	
DESIGN		260.876	260.876	260.450	INVERT	261.376	261.403	261.442	261.400	261.372	260.377	INVERT 260.872	260.872	260.993	
EXISTING	261.119	261.088	261.082			261.060	261.049	261.037	261.025	261.017		261.001	260.997	260.993	
OFFSET	7.88	5.00	4.50	3.66		2.50	1.50	0.00	1.50	2.50	3.66	4.50	5.00	5.49	

# EXTENSION OF EXISTING 4500 RCP (GRADE UNKNOWN) EXTEND WITH 450¢ RCP (CL4 RRJ) BY 7.32m TO NORTH AT 1% GRADE.

CULVERT No.3

CULVERT No.4 (ROAD 2 - CH 541.76)

~			REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:	
SCALE:	1:100 OR AS SHOWN				engineering				
	ATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		WESTERN WHEELERS MOT	
	PROJECTION: SURVEYED BY:		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL		
		GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23	COOTAMUNDRA NSW 2590			
SU			Α	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404			



CULVERTS 1, 2 & 4 PROFILES

					-3%	-3%	-3%	-3%			
			1 in 4						1 in 4		
											$\overline{}$
			-			450Ø Cla 0.5					
RL259.2m											
DESIGN	261.776	261.150	INVERT	263.554	263.584	263.629	263.584	263.554	261.223	INVERT	261.832
EXISTING	261.768 261.776			261.806	261.809	261.813	261.816	261.818			261.832 261.834
OFFSET	10.61 9.61	7.63		2.50	1.50	0.00	1.50	2.50	7.01		9.39

CULVERT No.5 (ROAD 3 - CH 31.45)

								-3%	-3%		-3%	-3%	1:-			
			1 in 4	0%	T								1 in		0%	1 in 4
				<u>``</u>	·											
													L			
									450Ø	Cla	iss 4 RRJ					
										1.0	0%					
RL259.3m																
		42	08	98	26	L L	38	338		383	338	308	353	ERT 808	98	
DESIGN		262.142	261.808	261.808	261.426	INVERT	262.308	2.3		2.3				INVER 261.808	261.808	
		26	26	26	26	Z	26	262.		262.	262	262	261.	26 26	26	
	t 7	42	36	33			24	19		12	74	00		90	88	83
EXISTING	62.147	262.142	262.136	262.133			262.124	262.119		262.112	262.104	262.100		262.090	262.088	262.083
	26	26	26	26			26	2(		26	26	26		26	26	26
	34	34	00	0	66		50	1.50		0	50	50	90	0	00	0
OFFSET	<b>₽</b> .	6.9	5.00	4.50	m.		2.5	1.0		0.00	1.50	2.5	3.66	4.50	5.00	6.00

CULVERT No.6 (ROAD 3 - CH 110.00)

SCALE: 1:1	1:100	REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:	PROJECT:
SCALE. 1:100							
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		WESTERN WHEELERS MOTO
		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL	
PROJECTION: GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23				
SURVEYED BY:		Α	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404		

	SHEET No.	35									
TORSPORT PARK	SHEET TITLE:										
IURSPURTPARK	CULVERTS 5 & 6										
	PROFILES										





SCALE:		REV.	DESCRIPTION DAT		REGIONAL	CLIENT:	PROJECT:
					s engineering		
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES		
PROJECTION:		С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702	BLAND SHIRE COUNCIL	WESTERN WHEEL
	GDA2020 (ZONE 55)	В	FOR REVIEW	19-06-23	COOTAMUNDRA NSW 2590		
SURVEYED BY:		Α	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404		

- AS REQUIRED. (v) NOTE THAT A SEDIMENT BASIN IS NOT REQUIRED AT THIS SITE.
- (iv) SAFE STORAGE AREAS WILL BE PROVIDED FOR FUELS, OILS, FERTILISERS, CHEMICALS AND OTHER HAZARDOUS MATERIALS

(iv) INSTALL CLEAN WATER DIVERSION DRAINS (SD 5-6). NOTE THESE MAY BE CONSTRUCTED TO LATER ACT AS THE FORMAL

(iii) INSTALL SEDIMENT AND BARRIER FENCES (SD 6-8) IN THE LOCATION SHOWN.

ROADSIDE DRAINAGE (SUBJECT TO THE ENGINEERING PLANS).

- (iii) RECEPTACLES WILL BE PROVIDED FOR WASTE MATERIALS AND LITTER.
- CONSEQUENT POLLUTION TO DOWNSLOPE LANDS AND WATERWAYS WILL NOT OCCUR.
- (i) SEDIMENT FENCING (SD 6-8) WILL BE INSTALLED AS SHOWN OR ELSEWHERE AT THE DISCRETION OF THE SITE MANAGER. (ii) SEDIMENT REMOVED FROM ANY TRAPPING DEVICE WILL BE DISPOSED IN LOCATIONS WHERE FURTHER EROSION AND
- 1.3 SEDIMENT CONTROL

2.1 STAGE 1 - SECURING THE SITE

(i) INSTALL THE STABILISED ACCESS (SD 6-14).

(v) DELINEATE PARKING AND STOCKPILE LOCATIONS.

(ii) INSTALL A SITE OFFICE (IF REQUIRED).

2. WORKS PROGRAM

- (iv) LIMIT DISTURBANCE TO THAT ESSENTIAL FOR WORKS BEING UNDERTAKEN AT ANY GIVEN TIME.
- (iii) THE SOIL EROSION HAZARD ON THE SITE WILL BE KEPT AS LOW AS PRACTICABLE BY MINIMISING LAND DISTURBANCE.
- CONSTRUCTION.
- (i) SITE DISTURBANCE WILL BE LIMITED IN EXTENT AND NATURE TO THAT DESCRIBED IN THIS SWMP (ii) VEHICULAR ACCESS TO THE SITE WILL BE RESTRICTED TO THOSE VEHICLES THAT ARE ESSENTIAL FOR EFFICIENT
- 1.2 EROSION CONTROL
- (ii) ALL SUBCONTRACTORS WILL BE INFORMED OF THEIR RESPONSIBILITIES IN MINIMISING THE POTENTIAL FOR SOIL EROSION AND POLLUTION TO DOWNSLOPE AREAS.
- (j) CONTRACTORS WILL ENSURE THAT ALL SOIL AND WATER MANAGEMENT WORKS ARE UNDERTAKEN AS INSTRUCTED IN THIS SPECIFICATION AND CONSTRUCTED FOLLOWING THE GUIDELINES STATED IN LANDCOM, 2004 (THE BLUE BOOK).
- NOTE THAT SD REFERS TO STANDARD DRAWINGS.
- 1.1 LEGAL RESPONSIBILITIES
- 1. GENERAL INSTRUCTIONS

CONSTRUCTED AS PER SD 4-1. (ii) CONDUCT EARTHWORKS AND ROAD CONSTRUCTION ACTIVITIES AND CONSTRUCT THE LONG-TERM ROAD DRAINAGE.

(i) AS WORKS ARE COMPLETED. PROGRESSIVELY STABILISE EXPOSED SOILS

(i) STRIP AND STOCKPILE TOPSOIL FROM THE ROAD CORRIDOR. STOCKPILES ARE TO BE LOCATED AS SHOWN AND ARE TO BE

(ii) REMOVE TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES ONLY AFTER PERMANENT VEGETATIVE STABILISATION IS

(iii) UNDERTAKE ONGOING MONITORING AND MAINTENANCE OF REVEGETATION AND STABILISATION WORKS AS DETAILED BELOW.

(i) AREAS OF CONCENTRATED FLOW (DRAINAGE DEPRESSIONS AND DIVERSION DRAINS) WILL BE STABILISED BEFORE THEY CARRY ANY FLOW.

(ii) DIVERSION DRAINS ARE TO BE LINED WITH JUTE OR COCONUT FIBRE MATTING IMMEDIATELY FOLLOWING CONSTRUCTION.

(iv) REPLACE TOPSOIL OVER EXPOSED SOIL AREAS TO A DEPTH OF AT LEAST 75 MM (NB REDUCE THIS TO 50 MM WHERE

(iii) FOLLOWING THE COMPLETION OF EARTHWORKS AND ROAD CONSTRUCTION, AREAS OF EXPOSED SOILS ARE TO BE LEFT IN A

(iii) UNDERTAKE ONGOING MONITORING AND MAINTENANCE OF EROSION AND SEDIMENT CONTROL WORKS AS DETAILED.

2.2 STAGE 2 - CONSTRUCTION / INSTALLATION

- COMPLETED AND THE SITE IS REHABILITATED.

- THE SEVERITY OF ANY RAIN EVENTS. RESPRAYING AS NECESSARY.

- INCLUDE:
- PLANTING ADDITIONAL STABILISING VEGETATION.
- STABILISING SOILS WITH MULCHES OR ALTERNATIVE SOIL BINDERS.

  - INSTALLING FORMALISED DRAINAGE CHANNELS OR PIPES.

  - IMPROVING SOIL PERMEABILITY BY CULTIVATING THE SOIL SURFACE.

- (i) SITE MONITORING WILL BE UNDERTAKEN DURING AND AFTER CONSTRUCTION TO ENSURE THAT:
- THIS PLAN IS BEING ADHERED TO; AND
- REVEGETATION ACTIVITIES ARE SUCCESSFUL

- (ii) WASTE RECEPTACLES WILL BE EMPTIED AS NECESSARY.
- (iii) A SELF-AUDITING PROGRAM WILL BE INITIATED FOR THE SITE. THIS SITE SUPERINTENDENT WILL INSPECT THE SITE AT LEAST WEEKLY AND FOLLOWING ANY RAIN EVENTS EXCEEDING 2 MM. AN INSPECTION LOG WILL BE MAINTAINED. INSPECTIONS WILL PAY PARTICULAR ATTENTION TO:
- REMOVAL OF SPILLED SAND OR OTHER MATERIALS FROM NEAR HAZARD AREAS.
- ENSURING BARRIER FENCING IS MAINTAINED AND EXCLUSION ZONES ARE BEING OBSERVED BY ALL SITE WORKERS AND CONTRACTORS.
- ENSURING PROGRESSIVE AND PROMPT REHABILITATION OF LANDS, THAT REHABILITATION HAS EFFECTIVELY REDUCED THE EROSION HAZARD AND UPGRADING OR REPAIR AS APPROPRIATE.

BATTERS EXCEED 1(V) TO 2(H) (SD 4-2). (v) MONITOR VEGETATION AREAS AND WATER THEM AS REQUIRED. (w) 4.

2.3 STAGE 3 - SITE REHABILITATION

COMPLETE.

3. STABILISATION

ROUGH CONDITION TO FACILITATE KEYING-IN OF TOP SOIL.

- SITE MONITORING AND MAINTENANCE

CONSTRUCTING ADDITIONAL EROSION AND / OR SEDIMENT CONTROL WORKS AS MIGHT BECOME NECESSARY TO ENSURE THE DESIRED WATER CONTROL IS ACHIEVED, I.E. MAKE ONGOING CHANGES TO THE SWMP. MAINTAINING EROSION AND SEDIMENT CONTROL MEASURES IN A FUNCTIONING CONDITION UNTIL ALL EARTHWORK ACTIVITIES ARE

REMOVAL OF TRAPPED SEDIMENT AND DISPOSAL TO SAFE AREAS.

REMOVAL OF TEMPORARY SOIL CONSERVATION STRUCTURES AS THE LAST ACTIVITY IN THE REHABILITATION PROGRAM. (iv) A RAIN GAUGE WILL BE INSTALLED AT THE SITE AND WILL BE MONITORED DAILY BY THE SITE SUPERINTENDENT TO DETERMINE

(v) REVEGETATION AREAS WILL BE INSPECTED REGULARLY TO INVESTIGATE FAILURES AND PROGRAM NECESSARY REPLANTING OR

(vi) AN ADEQUATE WATERING AND FERTILISING SYSTEM WILL BE MAINTAINED IN REVEGETATION AREAS. (vii) AREAS OF LOCALISED SOIL EROSION WILL BE IDENTIFIED AND APPROPRIATE PREVENTIVE MEASURES IMPLEMENTED. THESE MIGHT

- TAKING STEPS TO MINIMISE ANY CONCENTRATED STORMWATER FLOWS.

(viii) ANY AREAS OF LOCALISED POOR DRAINAGE WILL BE IDENTIFIED AND APPROPRIATE REMEDIAL ACTION TAKEN. THIS MIGHT INCLUDE:

IMPROVING SOIL PERMEABILITY BY INSTALLING INFILTRATION TRENCHES.

- PLANTING MOISTURE TOLERANT VEGETATION IN PROBLEM AREAS.

SHEETS 35 SHEET No. 34 SHEET TITLE: ERS MOTORSPORT PARK **EROSION & SEDIMENT** CONTROL NOTES



-				-								
		REV.	DESCRIPTION	DATE	REGIONAL	CLIENT:		PROJECT:		35	OUFFTO	25
SCALE:	SCALE.				engineering				SHEET No.	30	SHEETS	35
DATUM:	AHD	D	AMENDED FOR COUNCIL APPROVAL	25-08-23	REGIONAL ENGINEERING SERVICES			WESTERN WHEELERS MOTORSPORT PARK	SHEET TITLE:			
PROJECTION: GDA2020 (ZONE 55)	=	С	FOR COUNCIL APPROVAL	22-06-23	PO BOX 702		BLAND SHIRE COUNCIL		EROSION & SEDIMENT			NT
	В	FOR REVIEW	19-06-23					CONTROL DETAILS				
SURVEYED BY:		A	DRAFT - FOR COUNCIL COMMENT	17-04-23	PHONE 0407 351 404					CONTROL DETAILS		